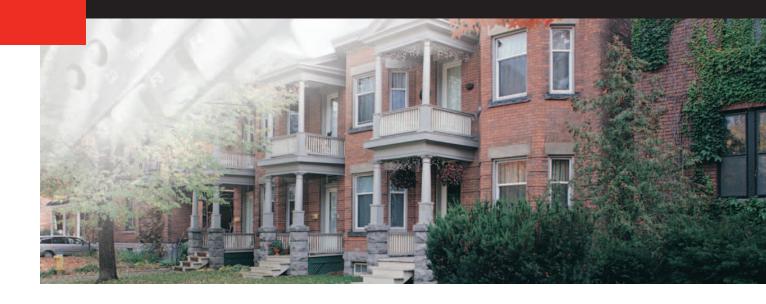
# RESEARCH REPORT



Impact of Water Softeners on Septic Tanks: Field Evaluation Study





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# Impact of Water Softeners on Septic Tanks Field Evaluation Study

# **Final Report**

Submitted to: Canada Mortgage and Housing Corporation

Submitted by:

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January 6<sup>th</sup>, 2006



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## **Executive Summary**

A field study of septic tank performance was conducted in order to determine whether water softener backwash addition to the septic tank had a significant effect upon tank performance. The sample group consisted of septic tanks receiving water softener backwash (n=27) and tanks not receiving water softener backwash (n=48). This study does not address impacts upon the performance of leaching fields.

Significant differences (P<0.05) in the sodium and chloride concentrations in tank sludges were found between the two groups with mean chloride concentrations increasing from 146 to 1515 mg/L and mean sodium concentrations increasing from 239 to 548 mg/L in tanks receiving water softener backwash. No significant differences (P>0.05) were found for indicators of tank performance including: septic tank effluent COD, CBOD<sub>5</sub>, TSS, and *E.coli*, sludge VSS and the sludge and scum accumulation rate. The results from this study indicate that water softener backwash discharged to septic tanks has no significant effect upon the biological or physical functioning of the septic tank; however, elevated chloride concentrations from water softener backwash may accelerate the corrosion of reinforced concrete tanks.

#### Résumé

Une étude a été effectuée sur le terrain afin d'établir si l'ajout de l'eau résiduelle d'un adoucisseur d'eau à la fosse septique avait une incidence importante sur la performance de la fosse. Le groupe échantillon comportait des fosses septiques qui recueillaient l'eau résiduelle d'un adoucisseur d'eau (n=27) et des fosses qui ne recueillaient aucune eau résiduelle provenant d'un adoucisseur d'eau (n=48). L'étude n'examine pas les incidences sur la performance des champs d'épuration.

Des différences importantes (P<0,05) ont été constatées entre les deux groupes au chapitre des concentrations de sodium et de chlorure dans les boues des fosses, les concentrations moyennes de chlorure augmentant de 146 à 1 515 mg/L et les concentrations moyennes de sodium passant de 239 à 548 mg/L dans les fosses qui recueillaient l'eau résiduelle d'un adoucisseur d'eau. Aucune différence marquée (P>0,05) n'a été observée pour les indicateurs de performance des fosses, notamment : la demande chimique en oxygène (DCO) de l'effluent de la fosse septique, la demande biochimique en oxygène des matières carbonées (DBOMC<sub>5</sub>), le TSS et l'*E.coli*, la MVS de la boue et le taux d'accumulation de la boue et de l'écume. Les résultats de cette étude indiquent que l'eau résiduelle des adoucisseurs d'eau évacuée dans les fosses septiques n'a pas d'incidence importante sur le fonctionnement biologique ou physique de la fosse septique; cependant, les surconcentrations de chlorure provenant de l'eau résiduelle de l'adoucisseur d'eau peuvent accélérer la corrosion des fosses en béton armé.



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We gratefully acknowledge the financial support of the Canada Mortgage and Housing Corporation for sponsoring this research project.

## **Introduction and Study Objectives**

This study involves a field evaluation of the impact of water softener backwash on the functioning of septic tanks treating domestic wastewater. The primary objective of the study is to evaluate the impact of sodium chloride addition from water softener backwash on the physical and biological treatment occurring in septic tanks under field conditions.

Systems with and without water softener backwash discharged to the septic system are compared using several indicators of system performance: COD, CBOD<sub>5</sub>, TSS and *E.coli* outlet concentrations, bacterial populations in the tank, sludge and scum accumulation rates, and signs of bed failure. The significance of each indicator is tested using an ANOVA at a 5 percent level of significance.

## **Background and Literature Review**

There have been several studies conducted over the past 30 years which have attempted to address the issue of water softener discharge effects on onsite systems. Study results and field observations have provided contradictory evidence as to whether water softener discharge is detrimental to onsite systems. The potential impacts addressed include: hydraulic loading to the septic system, septic tank microbiology, tank mixing and settleability of suspended solids, and leaching field soil permeability (CWRS, 2001). Another potential impact which has not been addressed in previous studies is the potential for chloride induced corrosion of concrete tanks.

#### How a Water Softener Works

Water softeners remove hardness (dissolved calcium and magnesium) through an ion exchange process. Incoming hard water passes through a tank containing ion exchange resin beads which are super saturated with sodium. As the water passes by the beads, the calcium and magnesium ions replace the sodium ions on the resin and sodium is released into the water. When the resin becomes saturated with calcium and magnesium, a backwash regeneration cycle is instigated. A concentrated salt brine solution (NaCl) is bachwashed through the resin, replacing the calcium and magnesium ions on the resin with sodium ions. The regenerate water, containing calcium, magnesium, sodium and chloride flows into the septic tank and eventually into the leaching bed. The amount of sodium added to the water and salts added to the septic system will depend upon the hardness of the water, household water use and the type and operation of the water softener. Potassium chloride (KCl) can be used instead of sodium chloride to regenerate the ion exchange resin. Potassium chloride, which is roughly twice the cost of sodium chloride, is typically used when a resident is on a sodium reduced diet or when the treated wastewater is reused for irrigation.

#### Septic Tank Hydraulics

It is generally agreed that the hydraulic load from water softener backwash regeneration should not have a significant impact upon the detention time in the septic tank (CWRS, 2001; Moore, 2001). Regeneration rates can create an additional discharge of up to 190L per cycle, which is comparable to the volume discharged from a typical washing machine (CWRS, 2001). Given that water softeners typically recharge 1 to 2 times per week, the additional volume is equivalent to one or two extra loads of laundry per week. In a study on home water use, Siegrist *et al.* (1976) found that water softener discharge accounted for only 6.2% of the total flow to the septic tank. Water softener discharge should in most circumstances have no significant impact on the hydraulics of the septic tank as the volume is relatively small, the wastewater is discharged quite slowly to the tank, and in most cases the regeneration backwash cycle occurs at night, when household water use is at a minimum.

It has been suggested by CWRS (2001) that the regeneration brine could cause density stratification within the septic tank and that this could lead to wastewater short circuiting through the tank. To our knowledge no studies have been conducted to test this hypothesis.

## Impact of Salt on Septic Tank Microbiology

Septic tanks provide primary wastewater treatment through sedimentation and anaerobic digestion. The organic matter in the sludge layer undergoes facultative and anaerobic decomposition and is converted to more stable compounds and gases.

The biological conversion of organic matter under anaerobic conditions occurs in three steps: hydrolysis, acidogenesis and methanogenesis. In the hydrolysis step, a group of nonmethanogenic microorganisms break down high molecular weight organic compounds including proteins, starches and cellulose into simpler compounds such as monosaccharides and amino acids. In the acidogenesis step, a second group of nonmethanogenic microorganisms consisting of facultative and obligate anaerobic bacteria, referred to as *acidogens*, ferment the products to simple organic acids, the most common of which is acetic acid. Nonmethanogenic bacteria that have been isolated from

anaerobic digesters include: Clostridium spp., Peptococcus anaerobic, Bifidobacterium spp., Desulphovibrio spp., Corynebacterium spp., Lactobacillus, Actinomyces, Staphylococcus, and Escherichia coli. In the methanogenesis step, methanogenic bacteria, referred to as methanogens, convert hydrogen and acetic acid formed by the acidogens into methane gas and carbon dioxide. Common methanogens include: Methanobacterium, Methanobaciullus, Methanococcus, and Methanosarcina. (Crites and Tchobanoglous, 1998)

Sodium is moderately inhibitory to anaerobic bacteria at 3.5 to 5.5 g/L and is highly inhibitory at 8 g/L (Robert Alley, 2000). In a study of sodium toxicity in mesophilic completely mixed anaerobic digesters it was found that methane production was reduced when sodium concentrations reached 6 to 9 g/L sodium addition; however, the addition of 200 mg/L calcium and 325 mg/L magnesium antagonized the sodium inhibition effect (Bashir and Matin, 2001). In a similar study on three different sludges, 50% inhibition was observed over a range of 3 to 16 g/L sodium with a strong antagonizing influence from the presence of other salts (Feijoo *et al.*, 1995). In another study utilising an anaerobic granular biomass, sodium concentrations of 5, 10, and 14 g/L caused 10, 50 and 100% inhibition of methanogens, respectively, at neutral pH (Rinzema *et al.*, 1988).

Kargi and Dincer (1999) found COD removal was inhibited in an rotating biological contactor (RBC) unit at NaCl concentrations greater than 20 g/L (2%), while Uygur and Kargi (2004) found decreasing COD, NH<sub>4</sub>-N and PO<sub>4</sub>-P removal with increasing NaCl concentration from 0 to 6 g/L using a lab scale anaerobic/aerobic sequencing batch reactor (SBR) system with a synthetic feed. In a study of a high NaCl wastewater treated by an anaerobic/anoxic/aerobic process, it found that COD removal declined from 97% to 60% and to 71% in non acclimatized and acclimatized brine solutions, respectively, as NaCl concentrations increased from 0 to 30 g/L (Panswad and Anan, 1999).

A study by the National Sanitation Foundation (NSF) (1978) on the impact of water softener brine on aerobic treatment units found no negative effects on the bacterial population. The literature review conducted by the Centre for Water Resources Studies

(Dalhousie University) reflects the same opinion, stating that salt addition to the septic tank slightly reduces the osmotic potential in the tank toward the optimum range for bacterial growth (CWRS, 2001). However, these findings were based upon NaCl concentrations measured at the septic tank outlet, as opposed to within the sludge itself where much of the digestion is occurring. Contradictory opinions were expressed in the Pipeline article (Moore, 2001) from two onsite wastewater experts who have observed trends of inadequate treatment from septic systems receiving water softener discharge including the non-digestion and carry-through of cellulose waste, as well as reduced scum layer development and carryover of solids and grease. These observations imply that the water softener discharge impacts the anaerobic bacterial metabolism as well as the settleability of solids in the tank, possibly due to density stratification and short circuiting through the tank.

Salt concentrations in septic tank effluent typically range from 40 to 100 mg/L chloride and 60 to 100 mg/L sodium excluding the addition from water softeners (Crites and Tchobanoglous, 1998). Sodium concentration in softened well water was 278±186 mg/L compared to 110±98 mg/L in municipal non-softened water in a Michigan study (Yarows *et al*, 1997). Backwash brine will increase chloride levels in septic tank effluent from 70 to 100 mg/L to 1500-2000 mg/L (CWRS, 2001).

In a study by Tyler *et al.* (1977), septic tank effluents (including systems with and without water softeners) were found to have salt concentrations from 7.3 to 21.8 meq/L (427 to 1644 mg/L NaCl) and sodium absorption ratios from 2.5 to 24.7. Sodium concentrations from septic tank effluent from households with a water softener (n=7) were 275 ±149 mg/L Na compared with 142±52 mg/L Na from households without a water softener. The osmotic potentials of septic tank effluents were determined to be between -0.21 and -0.77 bars, compared with reported optimal potential of -14 bars (~17,550 mg/L NaCl) for bacterial cell growth, suggesting that increasing salt content could actually improve the osmotic potential within a septic tank for bacterial life.

#### Hydraulic Conductivity of the Leaching Bed

Sodium can cause clay to swell, thereby reducing the hydraulic conductivity in the leaching bed. A study at the University of Wisconsin-Madison examined the effect of water softener discharge on the percolation rate of water in the leaching bed and found that there was no impact upon soil hydraulic conductivity (Corey *et al.*, 1977). The researchers concluded that the calcium and magnesium in the regenerate waters counteracted the impact of the sodium, as divalent cations reduce swelling in clay soils. Soils with a clay content of 15% or more can experience swelling and a deterioration of hydraulic conductivity if the sodium adsorption ratio (SAR) is greater than 10, while the SAR value should be less than 20 for soils with lower clay content (Crites and Tchobanoglous, 1998). SAR is the ratio of sodium to calcium and magnesium ions in solution.

#### Corrosion of Concrete Tanks

Hydrogen sulphide gas (H<sub>2</sub>S) is considered to be the primary cause of corrosion of concrete septic tanks. Sulphate in wastewater is biologically reduced under anaerobic conditions to sulphide which can combine with hydrogen to form hydrogen sulphide gas (H<sub>2</sub>S) (Metcalfe and Eddy, 1991). Hydrogen sulphide gas accumulates in the void space above the liquid layer in the septic tank, where it can be oxidized biologically to sulphuric acid. The sulphuric acid leaches calcium from the concrete, reducing the tank's structural integrity and can lead to structural failure. As well, hydrogen sulphide can directly corrode exposed concrete reinforcement by reacting with iron to form iron sulphide (Perry and Green, 1997).

Chloride in known to act as a strong catalyst of corrosion of the iron bars in reinforced concrete (Litvan, 1984). Therefore, elevated chloride levels in septic tanks could accelerate concrete tank corrosion. However, we are not aware of any studies which have evaluated the relative role of elevated chloride concentrations from water softener backwash on the corrosion of concrete tanks.

## Methodology

#### Field Data Collection

The study consists of the evaluation of 75 different residential septic tanks - 27 tanks with water softener backwash discharged to the tank and 48 without.

The field data was collected by René Goulet of Goulet Septic Tank Pumping. Mr. Goulet operates a septic pumping truck in Eastern Ontario, generally within the United Counties of Stormont, Dundas and Glengarry and the United Counties of Prescott and Russell (East of Ottawa between the Quebec and US borders). Ontario Rural Wastewater Centre (ORWC) researchers accompanied Mr. Goulet for the first several sample events in order to develop and document a standardised sampling methodology.

Each homeowner was asked to participate in the study as Mr. Goulet arrived to pump out the septic tank. Therefore, there was no possibility of bias from homeowners changing their practices on account of the study. Participating homeowners and individual data will remain confidential. A survey form was filled out by Mr. Goulet and each homeowner to gather the following information on each system: water softener type and amount of salt used, tank age, date of last pump-out, number of residents and bedrooms, type of septic system, soil type, and any history of bed failure or water quality problems. The survey form is presented in Appendix A.

The size, material and condition of each tank as well as any signs of leaching bed failure were documented by Mr. Goulet. The sludge and scum depths were measured using a "Sludge Judge"; a 2.5cm dia. clear plastic tube with a ball valve in the orifice. The tube is lowered into the tank and fills with a column of the tank liquid. When the tube is raised the ball closes the orifice and the depth of the sludge and scum layers can be measured. A photograph was taken of the outlet baffle when corrosion was evident.

A 2-L sludge sample was collected from the top 10 cm of sludge in the first compartment of each tank. The sludge sample was collected by taking a series of water column

samples using the "Sludge Judge" and transferring the sludge component of the sample into a 2-L sample bottle. A 1-L sample was also collected from the outlet T of each tank. The "Sludge Judge" was used to collect this sample as well. Any scum was pushed aside prior to taking the sample and only sample collected from the level of the outlet T was transferred to the sample bottle. Samples were stored in a dedicated refrigerator in Mr. Goulet's garage prior to pick-up by ORWC staff and transfer to the Collège d'Alfred laboratory for analysis.

#### Laboratory Analyses

All samples were stored at 4°C and all analytical methods follow Standard Methods for the Examination of Water and Wastewater (APHA/AWWA/WEF, 1998).

Each sludge sample was analysed for: Cl, Na, Ca, Mg, TSS, VSS, pH and total coliform. Each septic tank effluent sample was analysed for: Cl, Na, Ca, Mg, TSS, VSS, CBOD<sub>5</sub>, COD, pH, total coliform, *E.coli* and heterotrophic plate count (HPC).

The Cl, TSS, VSS, CBOD<sub>5</sub>, pH, total coliform, *E.coli* and HPC analyses were conducted in the ORWC Water Quality Laboratory at Collège d'Alfred, while the Ca, Mg, and Na analyses were conducted at Accutest Laboratories in Ottawa.

## Statistical Analysis

The analytical results were divided into two groups: samples from tanks receiving water softener discharge and samples from tanks not receiving water softener discharge. Outliers were defined as being  $\pm$  3 standard deviations from the mean and were removed from the dataset. Data from the 2 groups were compared using a single factor ANOVA test for significance (P=0.05).

#### **Results and Discussion**

Raw data is presented in Appendix B.

#### Septic Tank Sample Group

The study sample consists of 75 septic tanks divided into two subgroups: 27 tanks receiving water softener backwash discharge (WS) and 48 tanks not receiving water softener backwash discharge (NWS). Table 1 compares the two experimental subgroups in terms of tank characteristics (volume, material, age) and use (number of inhabitants, years since the tank was last pumped out). As can be seen from Table 1, tank characteristics and use are similar between the two subgroups, suggesting that the impact of salt on tank performance can be compared between the two groups without an evident bias in the sample populations used.

**Table 1. Septic Tank Sample Group** 

Parameter	Unit	Tanks Receiving Water Softener Backwash	Tanks Not Receiving Water Softener Backwash
		Median (Range)	Median (Range)
Number of Tanks	Number	27	48
Tank Volume	Litres	3600 (2700-5400)	3600 (1800-5400)
Tank Material		27 concrete	45 concrete - 2 steel –
			1 plastic
Tank Age	Years	20 (5-40)	20 (2-40)
Number of	Persons	3 (1-5)	3 (1-6)
Inhabitants			
Years Since Last	Years	5 (2-19)	4 (0.5-20)
Pump-out			

Salt use to regenerate water softeners typically varied between 20-40 kg/month.

#### Effect of Water Softener Backwash on Tank Performance

Table 2 compares tanks receiving water softener backwash to tanks not receiving water softener backwash in terms of sodium and chloride concentrations and indicators of tank performance: Septic Tank Effluent (STE) COD, CBOD<sub>5</sub> and TSS concentrations and

*E.Coli* counts, solids accumulation within the tank, and bacteria populations within the tank. The sodium adsorption ratio (SAR) for the two groups is also compared, as this parameter could impact soil permeability in leaching beds with high clay content.

Table 2. Effect of Water Softener Backwash Discharge on Tank Performance

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Parameter	Unit	<b>Tanks Receiving Water Softener</b>	n	<b>Tanks Not Receiving Water</b>	n	AVOVA
		Backwash		Softener Backwash		P=0.05
		(Mean $\pm 1$ Standard Deviation)		(Mean ± 1 Standard Deviation)		
Cl <sup>-</sup> (STE)	mg/L	686±773	21	90±69	35	0.00
Cl <sup>-</sup> (sludge)	mg/L	1515±1329	15	146±67	21	0.00
Na (STE)	mg/L	604±801	19	121±76	36	0.00
Na (sludge)	mg/L	548±386	12	239±87	20	0.00
SAR (STE)		9.2±8.6	20	4.4±4.7	34	0.01
COD (STE)	mg/L	$1004\pm1328$	13	1611±2636	27	0.44
CBOD <sub>5 (STE)</sub>	mg/L	340±203	18	396±281	33	0.46
TSS (STE)	mg/L	703±715	18	400±571	32	0.11
VSS (sludge)	g/L	33.5±20.7	16	30.3±13.3	21	0.57
TC (sludge)	cts/100 mL	1.87 x 10 <sup>6</sup> (geometric mean)	16	4.46 x 10 <sup>6</sup> (geometric mean)	18	0.44
HPC (STE)	cts/100 mL	2.83 x 10 <sup>6</sup> (geometric mean)	11	3.86 x 10 <sup>6</sup> (geometric mean)	25	0.54
E.coli (STE)	cts/100 mL	$3.24 \times 10^5$ (geometric mean)	16	2.29 x 10 <sup>5</sup> (geometric mean)	35	0.63
Sludge and Scum	L/person/year	118±78	23	117±57	39	0.95
Accumulation Rate						
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NOTE: P<0.05 is considered to be a significant difference between means.

There were significant differences in both sodium and chloride concentrations (**P<0.05**) between tanks receiving water softener backwash and tanks not receiving water softener backwash (**P=0.00**). The chloride concentrations (Cl <sub>(STE)</sub> = 686±773 vs 90±69 mg/L) are similar to values reported in the literature: 1500 to 2000 mg/L in the STE of systems receiving water softener backwash and 70 to 100 mg/L in systems not receiving water softener backwash (CWRS, 2001). The two subgroups have significantly different sodium chloride concentrations; therefore, the impact of salt can be compared using indicators of septic tank performance.

Septic tank effluent quality was compared between the two subgroups in terms of COD, CBOD<sub>5</sub> and TSS; three common indicators of onsite wastewater system performance. As well, *E.coli* and HPC counts were compared to test whether salt impacts two common bacterial indicators. There were no significant differences (**P>0.05**) between COD (STE) (**P=0.44**), CBOD<sub>5</sub> (STE) (**P=0.46**), TSS (STE) (**P=0.11**), *E.coli* (STE) (**P=0.63**) and HPC (STE) (**P=0.54**) comparing tanks receiving water softener backwash to tanks not receiving water softener backwash.

Typical STE contains 150-250 mg/L BOD<sub>5</sub> and 40-140 mg/L TSS (Crites and Tchobanoglous, 1998). The average CBOD<sub>5</sub> and TSS values measured in this experiment (CBOD<sub>5 (STE)</sub> =  $377\pm255$  mg/L; TSS (STE) =  $509\pm636$  mg/L) were higher than values reported in the literature. This suggests that high solids carryover into the leaching field may be a more significant problem than is suggested by the literature. The data from this study reinforces the importance of using septic tank effluent filters to prevent solids carryover into the leaching field and the importance of implementing management programs to have septic tanks periodically inspected and/or pumped out.

Bacterial degradation within the tank was measured indirectly using three indicators: volatile suspended solids (VSS), which is a common measure of bacteria biomass in aerobic and anaerobic digesters, total coliform, which is a common indicator of facultative bacteria, and the sludge and scum accumulation rate (Equation 1).

Sludge and Scum Accumulation Rate = Depth of Sludge & Scum x Tank Volume (Equation 1)

Liquid Depth x Persons x Years since last pump-out

There were no significant differences comparing tanks receiving water softener backwash to those not receiving water softener backwash for sludge VSS concentration (**P=57**), sludge total coliform counts (**P=0.44**) and sludge and scum accumulation rate (**P=0.95**). The lack of any observed impact from sodium concentrations on biological activity in the tank is consistent with the literature, which reports that sodium is only moderately inhibitory to anaerobic bacteria at concentrations of 3500-5500 mg/L and strongly inhibitory at 8000 mg/L (Roberts Alley, 2000); compared with an average sodium concentration observed in this study of only 550 mg/L. Only one sodium measurement was greater than the 3500 mg/L threshold.

There was a significant difference in Sodium Adsorption Ration (SAR) (**P=0.01**) comparing STE from tanks receiving water softener backwash to those not receiving water softener backwash. The tanks receiving water softener backwash had a median SAR of 7.9 and a range of 0.5-35.0, while the tanks not receiving water softener backwash has a median SAR of 1.6 and a range of 0.5-15.9. Thirteen of fifty eight STE samples had SAR values greater than 10; the limit at which swelling could occur in clay soils of greater than 15% clay content. Three of the thirteen systems with SAR>10 were in clay soils and none of the thirteen systems were showing signs of hydraulic failure. However, this study did not investigate the condition or permeability of the leaching field soils.

#### Tank Corrosion

The primary agent of concrete tank corrosion is sulphuric acid derived from hydrogen sulphide gas. However, high chloride concentrations from water softener backwash could play a role in accelerating the corrosion of reinforced concrete tanks by contributing to the corrosion of the reinforcing bars.

The condition of each tank in the study was recorded on the survey form and pictures were taken of systems which had experienced obvious corrosion. Table 3 describes the condition of the concrete tanks, while Figure 1 exhibits corroded outlet baffles from two of the tanks evaluated. As can be seen from Table 3, 38% of tanks receiving water softener backwash exhibited obvious corrosion of the outlet baffle, compared with 23% of tanks not receiving water softener discharge. It would appear that concrete tanks receiving water softener discharge are more likely to experience corrosion of the outlet baffle than tanks which are not receiving water softener discharge; however, the subjective and descriptive nature of the evaluation makes drawing a firm conclusion difficult. The impact of chloride from water softener backwash on corrosion of reinforced concrete tanks beyond that caused by hydrogen sulphide gas has not been evaluated in this study.

Table 3. Effect of Water Softener Brine on Tank Corrosion

Measure	Units	Tanks Receiving Water Softener Backwash	Tanks not Receiving Water Softener Backwash
Median Age (Range)	Years	20 (5-40)	20 (2-40)
Number of Tanks	Number	26	31
Number of Corroding	Number	10	7
Outlet Baffles			
Portion with	%	38	23
Corroding Baffles			



Figure 1. Corroded Outlet Baffles of two Tanks Receiving Water Softener Backwash – Does chloride accelerate the corrosion caused by H<sub>2</sub>S gas?

## Condition of the Leaching Bed

Twelve of seventy-five systems evaluated were experiencing hydraulic failure; where failure is defined as surface breakout (2 systems) or water level in the tank higher than the outlet (10 systems). Of the twelve leaching beds experiencing hydraulic failure, none

were receiving water softener backwash; however, one home had a water softener which was not discharging the backwash to the septic system.

Importantly, 9 of the 12 systems were installed in clay soils, representing 41% of the systems installed in clay soils compared with just 3% failure of systems installed in other soil types. This data suggests that clay soils are a strong determinant of system failure.

The failed systems ranged in age from 10 to 40 years, with a median age of 27 years compared with a median age of 20 years for the rest of the systems, suggesting that system age is also a determinant of failure.

#### Solids Accumulation in the Tank

The solids (sludge and scum) accumulation rate was calculated to be 117±65 L/person/year (n=62). The literature and Ontario regulations typically suggest pumping out the septic tank when it has become 1/3 full of solids. Using this volume as the pumpout threshold, Table 4 provides a suggested tank pump-out frequency based upon the mean accumulation rate measured from 62 septic tanks. As well, the accumulated sludge data is presented as a function of time in Figure 2. As can be seen from Figure 2, few tanks required pumping before 3 years, while most required pumping after 5 years.

**Table 4. Suggested Tank Pump-out Frequency (Years)** 

Tank			Persons in	the Home		
Volume	1 Person	2 Persons	3 Persons	4 Persons	5 Persons	6 Persons
1800L	5	2	1	1	1	
2700L	7	3	2	1	1	1
3600L	10	5	3	2	2	1
4500L		6	4	3	2	2
5400L		7	5	3	3	2

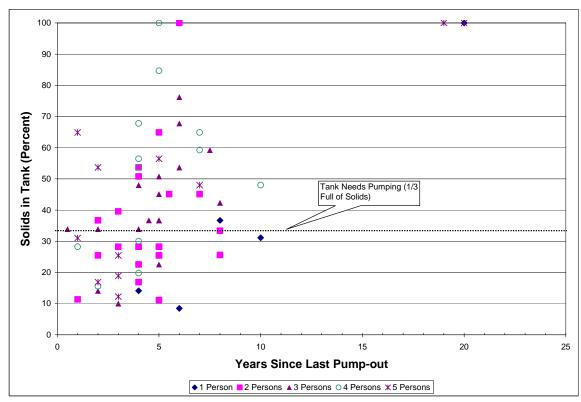


Figure 2. Sludge and Scum Accumulation with Time

## **Conclusions and Further Study**

A number of septic systems receiving water softener backwash (n=27) and not receiving water softener backwash (n=48) were compared to determine whether water softener backwash impacts the functioning of the septic tank.

There were significant differences (P<0.05) in the sodium and chloride concentrations between tanks receiving and not receiving water softener backwash. Mean sludge chloride concentrations increased from 146 mg/L in tanks not receiving water softener backwash to 1515 mg/L in tanks receiving water softener backwash. Mean sludge sodium concentrations increased from 239 mg/L in tanks not receiving water softener backwash to 548 mg/L in tanks receiving water softener backwash. While the data shows an increase in salt concentration with the use of water softeners, sodium concentrations do not reach levels required to inhibit biological activity within the septic tanks.

There were no significant differences (P>0.05) between tanks receiving water softener backwash to tanks not receiving water softener backwash in terms of series of indicators of tank performance: COD <sub>(STE)</sub> (P=0.44), CBOD<sub>5</sub> <sub>(STE)</sub> (P=0.46), TSS <sub>(STE)</sub> (P=0.11), *E.coli* <sub>(STE)</sub> (P=0.63), HPC <sub>(STE)</sub> (P=0.54), TC <sub>(sludge)</sub> (P=0.44), VSS <sub>(sludge)</sub> (P=0.57) and sludge and scum accumulation rate (P=0.95).

Tanks receiving water softener backwash were more likely to exhibit obvious corrosion of the outlet baffle (38% versus 23%); however, the evaluation was subjective in nature. The potential impact of chloride on the corrosion of reinforced concrete tanks beyond that of H<sub>2</sub>S gas has not been evaluated and bears further investigation.

Twelve of the seventy five systems evaluated were experiencing hydraulic failure. It appears that clay soils (9 out of 12 systems) and system age (median of 27 years) were the determinant factors of failure. None of the failed systems were receiving water softener backwash.

The results from this study indicate that water softener backwash discharged to septic tanks has no significant effect upon the biological or physical functioning of the septic tank with no significant differences observed in indicators of tank performance including the rate of solids accumulation and septic tank effluent quality.

#### **Further Study**

This field evaluation study considered the impact of water softener backwash on septic tanks. Further study is required to evaluate the impact of water softener backwash upon leaching field soils (particularly clay soils) and upon aerobic treatment units. A related issue which should be studied is the impact of calcium carbonate clogging of treatment unit orifices and media surfaces from hard water and from water softener backwash.

## **Technology Transfer**

The results of the study were presented at a Special Symposium on the Impacts of Water Softeners on Onsite Wastewater Systems October 13<sup>th</sup>, 2005 in Cleveland, Ohio cosponsored by the National Onsite Wastewater Recycling Association and the Water Quality Association. The paper presented at the Symposium will contribute to a "White Paper" being prepared on the topic.

The study results will be presented at the Annual Ontario Onsite Wastewater Association Conference in March 2006 in Kitchener, Ontario.

Study findings were published in an article in the fall 2005 edition of the Ontario Onsite Wastewater Association's "Onsite Wastewater News".

A summary of the study findings and the Final Report will be placed on the ORWC website in PDF format (<a href="www.orwc.uoguelph.ca">www.orwc.uoguelph.ca</a>).

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# **Appendix A – Homeowner Survey Form**

#### Effect of Water Softeners on Septic Systems - Survey Form

Date				
Location Information				
Name				
Address				
Tank Information				
Tank Type	Concrete	P	lastic	
Tank Size				
Tank Age				
Condition of tank				
* take photo *				
Date of last tank				
pump-out				
Sludge + scum depth (cm)				
Conductivity in first	Bottom 1/4	Middle	3/4	ton
chamber (µS/cm)	Bottom 4	Middle	<del>-7</del> 4	top
Outlet Sample (1L):	Temp (°C):			
•	1 . ,			
	pH:			
Clade a seconda (2L) (ten	T (0C)			
Sludge sample (2L) (top 10cm of sludge in first	Temp (°C):			
chamber):	***			
chamber).	pH:			
Water Softener Information				
Is a water softener being	Yes	No		
used				
Is water softener being	Yes	No		
discharged to septic tank	N. Cl	IZ CI		
Type of salt	NaCl	KCl		
Amount of salt used				
(kg/month)  Backwash Cycle (L/cycle,				
cycles/day)				
Water Use Information				
# of people in house				
# of bedrooms				
Drainage Field Information				
Type of system	Conventional	Treatment System	1:	
		ž		
	Raised Mound			
Age of system				
Signs of problems	mushy ground	effluent breakout	t	odours
	toilets backing up	water level in tan	k higher tha	an outlet
	water rushing bac	k into tank after pump	out	
Type of soil				
Well Information	I			
Well type (depth, m)	dug well		drilled w	vell
History of well water quality				
(Ecoli, fecal coliform, total				
coliform, nitrate): number of samples, dates, results				
sampies, dates, results	l			

# Appendix B – Raw Data

J				Tank Information		
	Tank Type	Tank Size	Tank Age	Condition of Tank	Last Pump- out	Sludge + Scum Depth (cm)
1	Concrete	800 gal	23	1 compartment-Good. Replaced outlet baffle	Oct. 2004	10.
2	Concrete	600 gal	37	1 compartment-Good. Baffle on.	2 yrs	22.
3	Concrete	1000 gal	23	Rotting cover, around outlet pipe always at outlet baffle rotting	2 yrs	15.
4	Concrete	1000 gal		1 compartment-Rotted outlet, baffle photos 1-2	1986	91.
5	Concrete	1000 gal		2 compartments-good	2004	25.
6	Concrete	1000 gal		Photos 3-4	1998	40.
7	Concrete	1000 gal	5	good	never pur	25.
8	Concrete	1000 gal	30	2 compartments-good at inlet could not see outlet	4 yrs	45.
9	Concrete	800 gal		Photo 20	4 yrs	
10	Concrete	800 gal		Photo 21 - outlet end of tank starting to break down		45.
11	Steel	400 gal		Poor but cannot really see as I had to pump through a pipe 2 years ago		68
12	Concrete	800 gal	15	good	1998	53.
13	Steel - rotting	400 gal	40	rotting	never pur	91
14	Concrete	600 gal	23	Pumped through a pipe	2 yrs	48
15	Concrete	800 gal	30	Seems good, manhole over	4 yrs	48
16	Concrete	800 gal	10	Photo 20 - rotten cover	4 yrs	6
17	Concrete	800 gal	9	Photo 18-19	6 yrs	91.
18	Concrete	800 gal	13	Tank is rotting - No.15	4 yrs	48
	Concrete	800 gal		No. 15	4 yrs	50.
20	Concrete	600 gal	35		6 months	30
21	Concrete	600 gal	35	good - 1 compartment	6 yrs	6
22	Concrete	800 gal		good - No. 13	6 yrs	68
23	Concrete	800 gal	25	1000 C	5 yrs	25.
24	Concrete	1200 gal	25	good	3 yrs	25
	Concrete	800 gal		good	5 yrs	45
	Concrete	800 gal		No. 12	3 yrs	35
	Concrete	800 gal		No. 12		v new own
28	Concrete	800 gal	30	good	10 yrs ago	2
29	Concrete	800 gal	20		8 yrs ago	3
30	Concrete	800 gal	20	No.11	8 yrs ago	2
31	Concrete	600 gal	15	good	5 yrs ago	1
	Concrete	1200 gal	9	good	4 yrs ago	2
	Concrete	800 gal		good	3 yrs	
34	Concrete	800 gal	28		2 yrs ago	1
35	Concrete	1000 gal	2	good (new)	never	3
36	Concrete	1200 gal	25	picture 5	3 yrs ago	1
37	Concrete	800 gal	25	good conditions	2 yrs ago	
38	Concrete	800 gal	25	No 8-9	3 yrs ago	1
39				P YOU COULT		
40	Concrete	800	20	good	8 yrs ago	38.
	Concrete	800		good	4.5 yrs ag	
	Concrete	800		good	4 yrs ago	25
	Concrete	1000			5 years	50.
		7775,5377		good		
	Concrete	800		good	5.5 years	40.
	Concrete	1000		good (replaced outlet baffle)	7.5 years	53
46	Concrete	1000	25	Good	7 years	43.
47	Concrete	800	20	Outlet end starting to break		20.
48	Concrete	800	20	good	5 years	22
49	Concrete	800		good	4 years	12
	Concrete	1200		good	5	40.
	Concrete	800		good	10	43
		800			4	
	Concrete	7,000		good	-	20
	Concrete	600		good	5	22
	Concrete	800		Outlet end starting to break	5	58.
55	Concrete	800		good (replaced outlet baffle)	4	43
56	Concrete	800	15	good	1	27
57	Plastic	850		good	2	3
	Concrete	800		good	5	76
	Concrete	800	30	good	4	30
	Concrete	1200		deteriorating at outlet end	3	22
61	Concrete	1000		rotting outlet baffle		48
62	Concrete	800	17	good	1	58
63	Concrete	1000	18	good	2	12
34	Concrete	800	15	good	2	30
65	Concrete	800	. 7	Outlet end starting to break	7	58
66	Concrete	1000		good	5	full
	Concrete	800		good	8	3
68	Concrete	1000	30	good	>15	91.4 (full)
69	Concrete	800	13	deteriorating at outlet end	4	17
	Concrete	800		good	5	20
71						
	Concrete	800	12	good	4	15
	Concrete	800		cover rotting	5	3
	Concrete	600		deteriorating at outlet end	5	3
	Concrete	800		good		68.
	Concrete	800		good	6	7
	Concrete	800		deteriorating at outlet end	6	48

Water So	ftener Informa	ntion			Water Us	e Informatio
Water Softener Use	System	Salt	Salt Amount (kg/mont	Backwash	# of People	# of Bedrooms
No	NO	N/A	N/A	N/A	2	
No	No	N/A	N/A	N/A	2	
Yes	Yes	NaCl	40 kg/month		5	
Yes	Yes	KCI	Unknown	3 days	5	
No	NO	N/A	N/A	N/A	4	
Yes	No	NaCl	20 kg/month		2	
Yes	Yes	NaCl	25 kg per 4		2	
Yes	Yes	NaCl NaCl	40 kg/month		2	
Yes Yes	No - just since	1	40kg/2 mon 40kg/2 mon		2	
No	No - just since	N/A	N/A	N/A	4	
No	No	N/A	N/A	N/A	4	
No	No	N/A	N/A	N/A	1	
No	No	N/A	N/A	N/A	5	
Yes	Yes	NaCl	140 kg/mon		2	
No	No	N/A	N/A	N/A	4	
Yes	Yes	KCI	40 kg/month	3 days	2	
Yes	Yes		2 x 20 kg/m	Automatic	4	
Yes	Yes	KCI.	30 kg/month	3-4 days	4	
No	No	N/A	N/A	N/A	3	
No	No	N/A	N/A	N/A	3	
Yes	Yes	NaCl		automatic evi	3	
No	No	N/A	N/A	N/A	2	
Yes	No	NaCl	20 kg/month		2	
Yes	Yes	NaCl	30 kg	4 days	3	
Yes	Yes	NaCl	30 kg/month	4 days	2	
No	No		1		3	
Yes	Yes	KCI	20 kg/month		1	
Yes	Yes	KCI	30 kg/month	4 days	. 2	
No	No	1401			2	
Yes	Yes	KCI	20 kg/month	1	2	
No No	No	-	-		4	
No	No No	+	-		3	
no No	No				6	
No	No		+		5	
Yes	Yes		-		4	
Yes	Yes	NaCl	40 kg/month	3.4 Nove	5	
100	No	14001	40 kg/moriti	o-4 days		
No	No				3	
1000	No	1	-		3	
No	1000					
No	No				2.5	
No	No	-	-		- 5	
No	No				2	
No	No	1			3.5	
Yes	Yes	NaCl	40	3-4 days	5	
Yes	Yes				3	
No	No				2	
No	No				1	
No	No				3	
No	No		+		4	
No	No				2	
No	No		1		2	
	_	-	+			
No	No		-		2	
No	No		-		3	
No	No				5	
No	No				2	
No	No				4	
No	No				3	
Yes	Yes	KC!	2 h	2 do:	5	
Yes	Yes	KCI	3 bags/mor		5	
Yes	No	KCI		3 days	5	
No Vec	No No	KCI		3 days	3	
Yes Yes	Yes	NaCl	40kg/month	3 days	4	
No	Yes No	IAGCI	-okg/month	Juays	4	
No	No	1	+		1	
No	No	-	+	-	5	
No	No				4	
No	No		+		3	
140	140				3	
No	No				2	
Yes	Yes	KCI	40 kg/month	automatic	3	
Yes	Yes	NaCl	40 kg/2mon		3	
Yes	Yes	NaCl	40 kg/zmonth		3	
	Yes	NaCl	40 kg/3 mo		1	
Yes						

	Drainage Field In	formation	1.	
	Type of System	Age of System	Problems	Soil Type
_1	Conventional	1982	Water level in tank higher than outlet	Clay loam
2	Conventional		None	Grenville loam
	Raised mound		No. Mantle of gravel at end	Sand/stone fill on clay
	Conventional		None	Earners loam
	Conventional	40 yrs	Water level in tank higher than outlet & water ru	
	Conventional	F	None	Clay loam
	Raised mound Conventional	5 yrs 30 yrs	None None	Sandy loam Clay loam
	Conventional	30 yrs	None	Clay loam
	Conventional	17 yrs	None	Sandy
			Water level in tank higher than outlet	Stony with clay fill, stone fen
12	Conventional	27 yrs	None	Clay
13	Conventional	40 yrs	Water level in tank higher than outlet	Clay
	Conventional (not			Clay loam
	Conventional	30 yrs	None	Earners loam
	Raised mound	0	Very sludgy	Earners loam
	Conventional Conventional	9 yrs	No - past due for being pumped	Sandy
	Raised mound	15 yrs	No	Sandy
	Conventional		Toilets backing up; water level in tank higher tha	The state of the s
	Conventional		Toilets backing up; blocked inlet pipe	Earners loam
1536	Raised mound	13 yrs	No No	Sandy
23	Conventional	25 yrs	No	Eamer's loam
24	Conventional		No	Sandy with gravel
	Raised mound	12 yrs	No	Sandy
	Conventional	40 yrs	No	Earners loam
-	Raised mound		No problem except Outlo towd ot tank decompo	
	Conventional	30 yrs 20 yrs	No problem No	Eamers loam
	Conventional Conventional	20 yrs	water level in tank higher than outlet	Sandy Earners loam
	Conventional	15 yrs	No problem	Sandy Gravelly
	Conventional	100,200	No problem	Eamers loam
33	Conventional	25 yrs	No problem	Stoney Hard pan
34	Conventional	1 1800	No problem	Earners loam
35	Raised mound		No problem	Sandy
	Conventional	25 yrs	No problem	Stony Hard Ground
	Conventional	25 yrs	No problem	Stony Hard Ground
	Conventional		No problem	Hard Stoney
39	Service Control of the Control of th		NI	FOR CONTRACTOR
-	Raised mound	200	No problem	Eamers loam
75.83	Conventional	5775	No problem	Earners loam
	Conventional	3.00	No problem	Clayish soil
- 177.11	Conventional	5	No problem	Clay
	Raised mound		No problem	Sandy
_	Conventional	32	No problem	Clay
1000	Conventional		No problem	Clay
100	Raised mound		No	Earners loam
	Raised mound	1.2	No	Sandy
	Conventional	15	Water level in tank higher than outlet	clay
	Raised mound		No	sandy
- 200	Raised mound		No	Sandy
	Conventional		Water level in tank higher than outlet	Clayish soil
	Raised mound		No	Sandy
	Conventional	2233	No	clay
-33	Raised mound	1 100	No	Sandy
	Raised mound	15	No	Sandy
	Raised mound		No No	Sandy
	Conventional	1000	No No	Clayish soil
	Conventional Raised mound	1000	No No	Clay Sand
	Raised mound	1	No	Sandy
	Conventional		Water level in tank higher than outlet	Clay
	Conventional	1000	No	Sandy-clay
64	Raised mound		No	Sandy
	Raised mound		No	Sandy
	Conventional	-	No	Eamers loam
	Conventional		No No	Stoney Hard pan
	Conventional	1-000	No No	Earners loam
	Raised mound	-	No No	Sandy
71	Raised mound	12	INO.	Sandy
	Raised mound	12	No	Sandy
	Raised mound		No	Sandy
	Raised mound		No	Sandy
	Conventional		No	Clayish
76	Raised mound		No	Sandy
	Raised mound		No	Sand

ļ			V	Vastewater I	Parameters			5
ı	Effluent cB0D5 mg/L	Sludge COD mg/L	Effluent COD mg/L	Effluent pH mg/L	Sludge pH mg/L	Effluent TSS mg/L	Sludge TSS mg/L	Sludge TS mg/L
	ing.c	mga.	myrc	6.9	myre	nigat	1300	4292
		1830			6.63		3000	
	488	3080	830	7.15	0.03	130	3400	838
	400	3000	030	7.13	7.12	130	12300	6992
	325			7.44	5.99	250	10150	682
	271			6.93	6.48	170	22600	1458
	271			0.33	6.72	170	5850	1787
3	406			6.9	5.88	520	2000	2834
	332		-	7.12	6.42	80	11850	5645
	283			7.09	6.19	160	14800	308
	203			7.03	5.72	100	3900	300
					6.04		6600	802
3				6.1	0.04		16250	002
ì				0.1	5.86		5300	
					6.06		13400	1524
	344			6.75	6.21	100	47400	5128
	324			6.71	6.1	770	8550	7187
	204		130	7.01	6.58	1000	14000	1025
i	201		100	7.09	6.2	2000	14000	5620
ı				7.00	6.65	2,000	72000	398
					0.03		54000	530
	188		150	6.88	6.7	2000	26000	5568
1	100		100	0.00	6.48	2000	88000	6288
					6.72		40000	1263
			1	6.74	6.59		20000	1298
	412			0.14	0.00	33000	26000	2023
	1005			6.745	6.66	2000	42000	750
3	1005			0.143	6.766	2,000	42000	1433
,					6.6		12000	2382
1	222.8			6.815	6.42	1000	12000	2893
	269			0.013	0.42	1000	12000	2033
	355					1000	33000	1199
3	333		-		-	1000	5000	1133
,	4610					103000	78000	7293
	4010					103000	14000	921
	2930					11333	4000	1056
,	380					1000	12667	979
1	178					2000	109000	8393
3	183				1	2000	4000	417
1	271		127		-	46	4000	711
-+			1,000					
	340		135			20		
2	130		115			34		
1	977		645			292		
1	745		717			460		
9	473		512			176		
	1357		4273			13100		
	452		357			120		
3	443		327			62		
4	604	1	630			108		
,			200		-			
1	223		407			160		
	177		5248			8040		
?	42		125			108		
3	>8000		7573			30000		
1	1503		7148			1880		
5	1019	-	2573			780		
6	352		217			124		
,	1157		3973			5480		
3	172		667			520		
9	266		362		-	320	-	
)	235		205		7	70		
	189		90			56		
2	562		562			164		
3	375		457			48	E 1	
1	245		320			104		
5	242		362			76		
3	2202		9423			6080		
	248		152			60		
3	272		375			360		
)	236		240			98		
ı	314		172		1	80		
	84		262		11 1	44	i i	
2	119		295			44		
3	862		2898			1260		
į	69		660			512		
5	185		182		-	172	*	
3	751		2423		-	880		
•	294		492			70		
	234		732		L	70		

#### Canada Mortgage and Housing Corporation Impact of Water Softeners on Septic Tanks - Final Report

					Salts						
	Effluent CI mg/L	Sludge Cl mg/L	Effluent Ca mg/L	Sludge Ca mg/L	Effluent Mg mg/L	Sludge Mg mg/L	Effluent K	Sludge K mg/L	Effluent Na mg/L	Sludge Na mg/L	SAR
1		66		911		75				163	
2	252	81 1145	52	562 469	41	73 92	17		129	152 619	
4	252	188		2840	41	219	17	268		019	
5	161	128		428	9		47		156	351	
6	150	150	13	768	4	38	12		228	360	1
7		1091		2050		16				673	
8	71 59	188 203		581	8 5		17 15		137	234	- 8
10	86	116		1570 351	10		26	-	201 162	351 310	- 3
11		86		1450	,,,	87			102	213	-
12		100		349		135				341	
13		150		1220	1	139				280	
4		77		677		139				405	
5  6	183	989 289	119	1730	32	192	29		120	207	
17	6140	11694	877	1770	82		26	44	2820	207	1
18	1203	2525		2310	74		36		716	1040	- 1
19	775	1884	68	918	13		9	70			
20		210		1780		86				274	
21		1	1	2570		158	1	1		189	
22	1535	1263		2940	25		18		707	847	
23		284 100		4060 2600		115 132				206 191	
24 25		5051		976	_	78				1290	
26	2652	2652	353	509	43		1830		49	121	- 8
27	44	147	104	1500	34		26	55			
28		775		1990		218				598	
29		814								1)	
30	90	236	149	1700	35		26		84	181	
31	41	450	505	58	C4	34			4070	242	
32	142	150 72		3400 167	64	181 47		22	1870	190	1:
34	95	171	1400	2340	315			22	197	128	- 3
35		95		568		22				158	
36	292	188	807	90	60				147	129	- 3
37	2181	3070		21	56				1110	129	1:
38	525	897	226	3440	29				353	654	
39	74	111	72	165	102	141			146	120	
10	262		19						247	-	
11	27		115		10		-		41	4	
12	67		117		11				42		
13	89		128		18				91		
14	70		99		19				34		- 3
15	52		101	-	20			-	35		
16 17	772		624		214 6				5090		3
	214		19						426		1
18 19	27 49		100 121		16 17			-	31 29		
50 50	162		121	- 1	7		1		29	1	
51	118		198		27				80		- 8
52	51		34		11				122		
53	846		263		60				130		
54	262		169		40				104		
55	222		165		39				104		
56 57	32 45		6 13		3 4				189 216		1
58	11		15		2				174		1
59	40		83		29				74		
30	543		78		27				551		1
31	209		91		30		18		162		
32	22		9		1		19		143		
33	18		6						154		1
34 35	70 191		18 14		4		246		57 302		1
95 36	23		82		25				38		
37	27		79		27				48		
88	37		101		31				50		
69	100		36		11				56		
70	180		12		3				312		1
71	33		87		31				48		
72	567		57		21				131		
73	1920		469		121		65	1	2660		2
74	663 245		141 53		59 23				397 339		
76	39		51	-	21		-		51		
77	44		45		21		20		50		

	STE <i>E.coli</i>	Bacteria STE Total Coliform	Sludge Total Coliform	STE HPC	VSS of Sludge
	cts/100mL	cts/mL	cts/mL	cts/100 mL	g/L
1			12900		30.725
2			12000		24.28
3		8500	1000		26.965
4			59000		53.315
5	700	3300			10.165
6	110	31	5600		35.71
7					27.59
8	73000	8200	9000		6.86
9		9000	22000		39.17
10	5200	21100	1200		11.66
11					22.215
12			37000		35.135
13			2380		31.34
14					21.865
15			1520		18,905
16	11000	119	116000		34.935
17	700	7	520		43.17
18	190000		12000		30.805
19			2000		43.835
20			100		37.04
21			3800	1	
22		5500	120000		30.61
23					50.48
24			6000		35.95
25			1200		16.085
26			1000		52.825
27			28000		43.345
28			48000		87.615
29			1900	- 1	34.355
30	40000	290	31000		38.215
31			23110		
32	2870000	1890000	11700000		26.41
33			3535000		11.16
34	500000	97000	2095000		56.95
35			28000000		33.605
36	1360000	45000	4400000		8.005
37	41000		9930000		11.3
38	72000	13300	1700000		42.97
39	231500	94381	430000		5.06
40	1890000			8000000	7327
41	3300000	*	-	4000000	
12	200000			62000000	
43	320000			17000000	
14	640000	-	7	2000000	
45	100000		3	11300000	
16	6200000			12000000	
47	6800000			19000000	
48	400000			5700000	
49	600000			6100000	
50	200000				7
51	1500000				
52	100000		7	300000	-
53	900000			3900000	1
54	200000			700000	
55	600000			1800000	
56	200000			4000000	
57	14900000			5000000	
58	200000			15000000	
59	6400000			3000000	
60	1200000			1500000	7
61	1300000			7100000	
62	27600000			76000000	
63	700000			30000000	
64	300000			900000	
65	100000			1800000	
66	100000			600000	
67	200000			1100000	
68	40000	-	7	700000	-
69	100000		- 1	900000	
70	100000			12000000	
71	200000			1300000	
72	200000			1300000	
73	100000			600000	
74	300000		-	1800000	
75	900000			4200000	-
76	2100000			4000000	

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