ENVIRONMENT CANADA CONSERVATION AND PROTECTION

A QUADRATIC PROGRAMMING MODEL

OF THE SOUTH SASKATCHEWAN RIVER BASIN

USER'S MANUAL

PREPARED BY
Roger McNeill

INLAND WATERS AND LANDS
PACIFIC AND YUKON REGION

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ABSTRACT

A prototype water-use optimization model for the South Saskatchewan River Basin was developed by staff of Pacific and Yukon Region. Quadratic programming was used and solved by the non-linear programming software called MINOS at the University of British Columbia. Access to the model is through the commercial account of Water Planning and Mangement Branch at the university. The model can be changed and solved from remote locations using dial-up ports or DATAPAC lines. The model allocates water use in the Basin with the objective of maximizing economic returns over a given The Basin is broken down into five sub-basins, with five final use year. activities over a time period of 12 individual months. The detailed equational structure of the model and methods of accessing it and editing the relevant files are presented. Many of the relationships are based on preliminary data or rough estimates. However, the basic structure of the model will not have to be significantly changed once better data have been obtained.

RESUME

Un modèle prototype d'optimization de l'utilisation d'eau pour le bassin de la rivière Saskatchewan Sud fut développé par le personnel des Eaux Intérieures et Terres de la région du Pacifique et du Yukon. Le modèle emploie une méthode de programmation quadratique qui est résoulue par le logiciel de programmation non-linéaire MINOS de l'université de la Ce modèle est accessible à travers un compte de la Columbie-Britannique. direction de planification et gestion des eaux à l'université. Le modèle peut être modifié et résoulu à partir d'un endroit eloigné en utilisant des lignes téléphoniques ou par DATAPAC. Le modèle alloue l'utilisation d'eaux dans le bassin afin d'atteindre le maximum de débit économique annuel. Le bassin est divisé en cinq sous-bassins et chacun comprend cinq catégories d'utilisation d'eau pour douze mois. Les équations du modèle sont présentées en détails ainsi que les façons d'aborder it de rédiger un Plusieurs rapports du modèle sont basés sur des données fichier. préliminaires ou des estimations brutes. Cependant, quand de meilleures données seront obtenues il ne sera pas nécessaire de changer de manière significative la base du modèle.

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I. INTRODUCTION

Multiple use of the water resource is an important issue in the South Saskatchewan River Basin. Managing the available water and allocating it among various users will continue to be a challenge for the agencies involved, especially as demands for water increase. The operation of Lake Diefenbaker is critical since it is the major reservoir in the system providing storage for consumptive uses while maintaining lake levels for power generation and recreation. In this study an optimization model is developed with the object of providing a tool for managers of the system to aid in determining the allocation of water and storage management in the Basin.

The model developed is a prototype model and relies heavily on secondary data and in some cases rough estimates of economic and physical parameters. Considerable effort would be required to improve the data and estimates which are used in the model. However, the structure of the model itself has proved to be logically consistent and is flexible in terms of incorporating new data or changing its geographical configuration or resolution. It is not anticipated that any major structural changes to the model would have to be made in order for it to be a useful tool for water management and planning in the Basin.

A. The Optimization Approach

The approach used in the development of the model is to attempt to maximize economic benefits from the allocation and management of the water in the Basin. Although it is not possible to quantify all the benefits from water use in dollar terms, reasonable estimates for the major water users (both consumptive and non-consumptive) can be obtained. Constraints can be imposed on the model to ensure that the solution meets environmental and social criteria that cannot be quantified in dollar values. It is important to note that the model should only be used as an aid to management and planning of the system. The final allocation of water in the system should depend not only on the quantifiable economic benefits, but also on the non-quantifiable environmental and social factors.

Economic benefits are defined as in traditional benefit-cost analysis. This implies that water is valued based on the wilingness-to-pay for it by the people or industries who use it. Depending on the situation, there are many ways to obtain an estimate of the willingness-to-pay and this study uses whatever method is most appropriate or feasible for the particular situation.

B. Quadratic Programming

In this study a quadratic programming model is constructed which represents the allocation of water within the Basin. A quadratic programming model consists of a quadratic objective function to be maximized subject to a number of linear constraint equations. For example, the following equations form a quadratic program.

Maximize $Z = 2A + 3A^2 + 2AB - 4B + B^2$

subject to: A < 10

2A + 3B > 20

B > 5

where Z = the function to be maximized

A&B = non-negative variables

This model is solved when we find the values of the A and B variables which result in the maximum value of function Z while satisfying the three constraints. There are several computerized algorithms which can solve a quadratic program quite efficiently. It is therefore feasible to solve a very large model (i.e. a model with a thousand variables and a thousand constraints) in a few seconds of mainframe computer time.

When applying quadratic programming to modeling the allocation of water in a river basin, the objective function would be used to represent the economic benefits associated with the use of water by various activities. For example the variable A in the above problem might represent the amount of water used by the domestic sector while the variable B could represent the water use for hydro-power generation. The objective function is simply a mathematical expression of the benefits from using the water in these sectors. It so happens that quadratic functions are usually quite appropriate forms for representing economic benefits associated with use of a commodity.

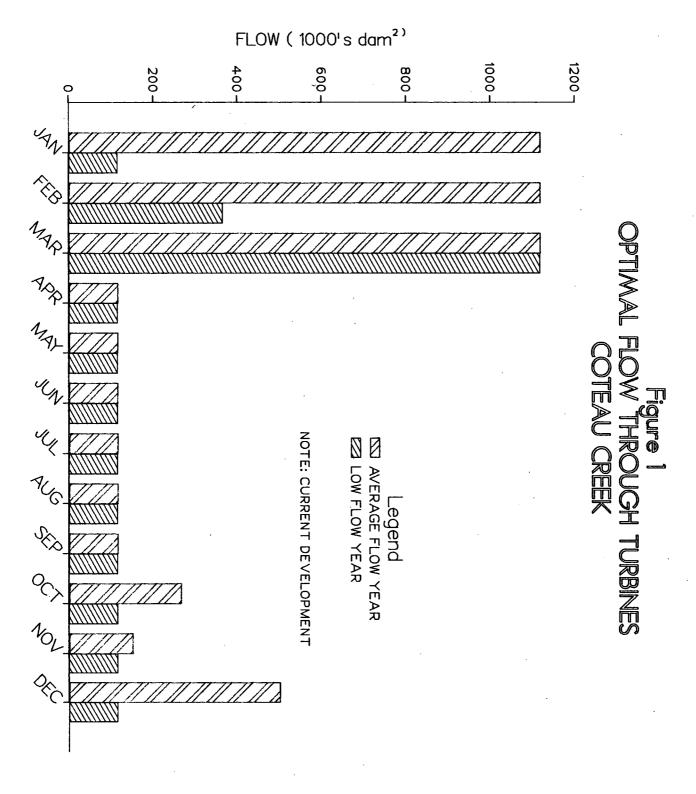
The linear constraints can be used to model the physical supplies and distribution of water in the system. Factors such as storage, maximum/minimum flows, evaporation, return flows, and diversions can be readily modeled as linear equations. Logical constraints such as storage balance equations and supply-demand relationships can also be modeled as linear equation in a straightforward manner.

C. Applications of the Model

This section gives some examples of the capabilities and possible applications of the model based on some initial solutions. In general the model is intended as a tool to aid in long term planning in the Basin and mid-to-long term operations of the system. It is particularly useful for assessing the sensitivity of economic benefits to parameters such as prices, flows and storage capabilities. It can be run over a number of run-off and growth scenarios. Various constraints on the model can be removed or lessened in order to analyze the effect on economic benefits from water use.

1. Monthly Allocation of Water

For each water using activity, the model solution gives the optimal amount of water during each time period and the optimal activity level. For example, the model solution will give the optimum area of irrigated land and the associated water used for irrigation in each month. Likewise it will give the optimum power production and flow through turbines in each month. Figure 1 shows the optimal monthly flows of water through the



turbines for dry and average years based on an assumed price structure for power.

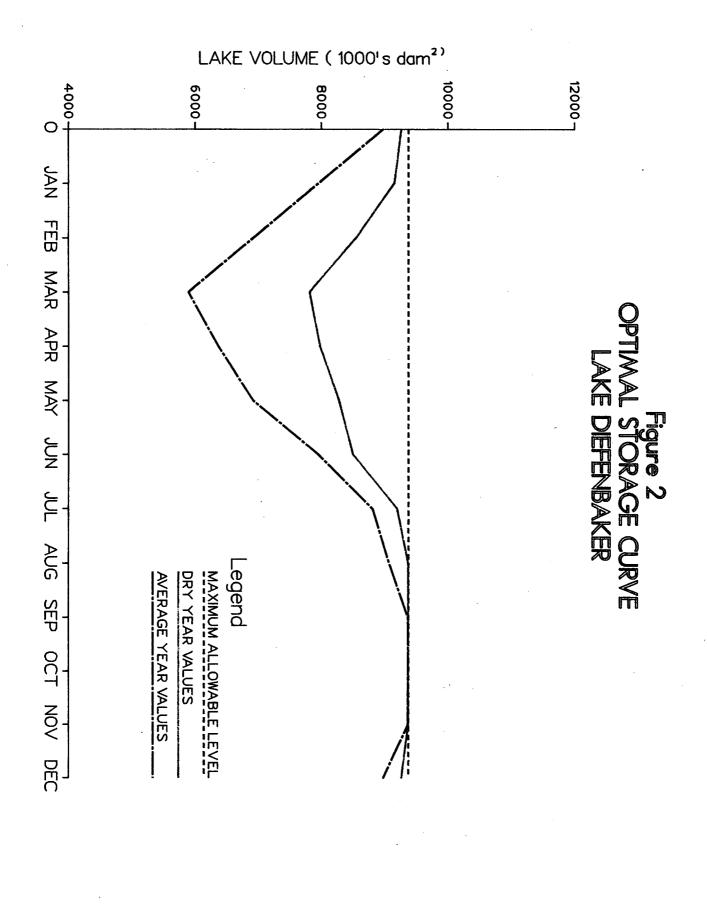
2. Operation of Lake Diefenbaker

The optimal lake level regime is a function of hydropower generation, recreational needs, storage for consumptive uses and maximum and minimum elevation restrictions. The model determines the optimum lake level and release from storage in each time period. Figure 2 shows the optimal lake level in each month of dry and average run-off years.

3. Assessing Physical Changes

The model can be used as a benefit calculator to assess the economic benefits of changes in physical parameters such as storage capacity or irrigation efficiency. For example, to determine the benefits of increasing the storage capacity on Lake Diefenbaker, the model could be run with and without the increased storage, and the benefits could be determined by comparing the objective function values for the two solutions.

A test run was carried out which increased the storage capacity of Lake Diefenbaker by 10%. This resulted in an increase in the objective function of \$9.9 million. Most of the increase (98%) resulted from increased power generation while a small proportion (2%) resulted from increased recreational values. Similar tests could be carried out for increasing storage on the tributaries or downstream of Lake Diefenbaker.



D. Accessing the Model

The model is currently installed on the University of British Columbia mainframe computer and is accessed via the commercial account of Water Planning and Management in Vancouver. The account can be accessed through a datapac line or through a modem connection from remote sites. Full details on how to run the model are given in chapter three.

The model is solved using a software package called MINOS which is a FORTRAN based set of routines for solving linear and non-linear programming programs. This software and the data files for the model can be transferred to other systems which have FORTRAN compilers. Because of the large size of the model, personal computers may not be suitable for running the model. However, the model is easily solved on a mainframe and solutions would probably be feasible on a super-micro or mini-computer.

II. SPECIFICATION OF THE EQUATIONS

This section presents the constraint equations and objective function coefficients for the model. The model is composed of five sub-basins shown in Table 1. The sub-basins are represented by distinct modules which are linked to form the model for the entire Basin. The modules are fairly similar in structure except for the Lake Diefenbaker sub-basin which includes storage, recreation and hydro-power activities not found in the other reaches. Figure 3 is a schematic representation of the Basin showing the five sub-basins and the water supply and demand points. A brief explanation of the logic behind the equations and the source and method of deriving the coefficients are also given.

A. Variable Names

Some conventions are followed so that each variable and constraint can be identified by its sub-basin and month. Variable names are usually similar between different sub-basins except for the first and last characters. The first character of any variable or constraint name represents its sub-basin module. The final one or two characters of the name represent the period from 1 to 12, as shown in Table 1. The same variable names and conventions are used in the computer files which contain the MINOS code for the model (see appendix B).

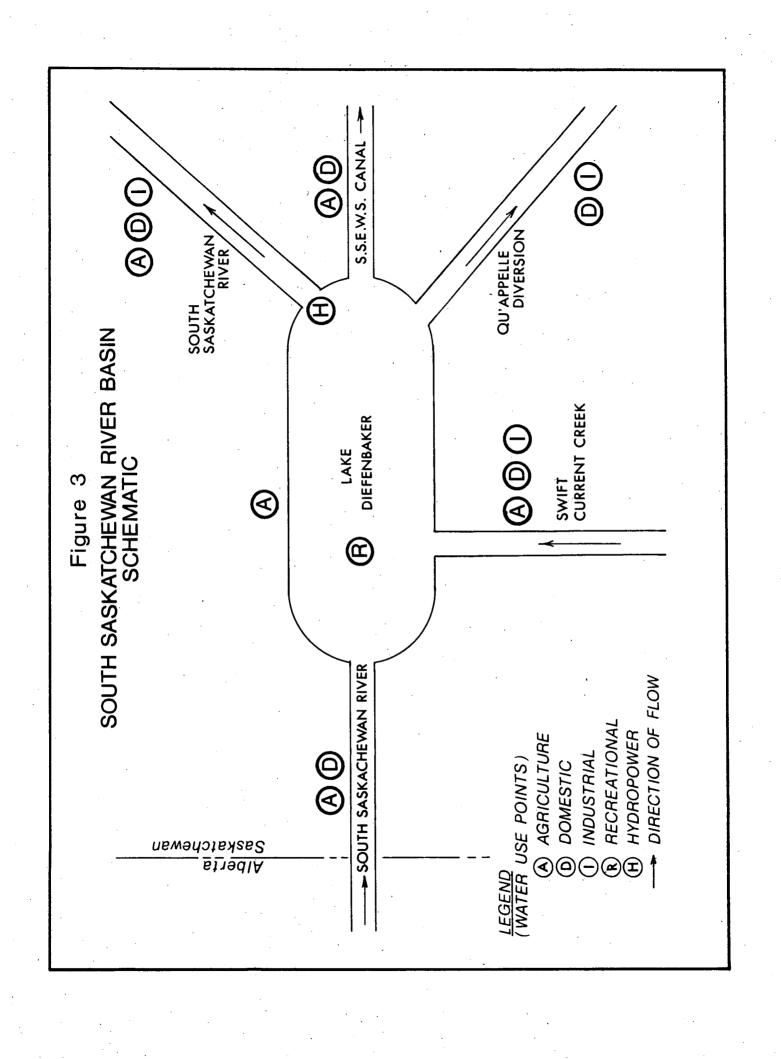


TABLE 1
Conventions Used in Variable and Equation Names

UBBASINS	CODE (first character)		
South Sask. River upstream of Lake Diefenbaker	R		
Swift Current Creek	S		
Lake Diefenbaker	L		
Lake Diefenbaker (hydropower generation)	Н		
SSEWS Canal	C		
South Sask. River downstream of Lake Diefenbaker	D		
PERIOD	CODE (last character)		
January February March April May June July August September October November December	1 2 3 4 5 6 7 8 9 10 11		

Each constraint equation in the model is required to have a name in the MINOS computer package. The same conventions to identify sub-basins and periods are used for these equations. However, the equation names are appended with the letter 'C' in order to distinguish them from variable names.

B. Equations

In this section the constraint equations and the objective function coefficients are presented for each sub-basin. Where required a brief explanation of the logic behind the equations and the data sources are given.

1. South Saskatchewan River Upstream Sub-basin

This portion of the mainstem is fairly simple to model since there is no storage and few water use activities. The most important feature of this sub-basin is the transboundary flows from Alberta into the Basin. These flows are the single most important source of water in the Basin.

a. Water Supply Activities

The first set of equations define the total amount of water available in the sub-basin for each period. Total supply consists of the transboundary flow from Alberta, the natural run-off in the sub-basin and any return flows from water use. Equations RSUP1C to RSUP12C define the monthly water supplies.

```
+ RROA1
                          + RRET1
                                   - RGSUP1
(RSUP1C)
          AREL1
                                              = 0
                 + RROA2
                          + RRET2
                                   - RGSUP2
(RSUP2C)
          AREL2
                          + RRET3 - RGSUP3
(RSUP3C)
          AREL3
                 + RROA3
                                              = 0
(RSUP4C)
          AREL4
                 + RROA4
                          + RRET4
                                    - RGSUP4
(RSUP5C)
          AREL5
                 + RROA5
                          + RRET5
                                    - RGSUP5
                                              = 0
          AREL6 + RROA6
                          + RRET6
                                    - RGSUP6
                                              = 0
(RUSP6C)
                          + RRET7
                                    - RGSUP7
(RSUP7C)
          AREL7
                 + RROA7
                 + RROA8
                          + RRET8
                                    - RGSUP8
(RSUP8C)
          AREL8
(RSUP9C) AREL9 + RROA9
                          + RRET9
                                   - RGSUP9 = 0
(RSUP10C) AREL10 + RROA10 + RRET10 - RGSUP10 = 0
(RSUP11C) AREL11 + RROA11 + RRET11 - RGSUP11 = 0
(RSUP12C) AREL12 + RROA12 + RRET12 - RGSUP12 = 0
```

where:

AREL1 to AREL12 = transboundary flow from Alberta

RROA1 to RROA12 = natural run-off within the sub-basin

RRET1 to RRET12 = return flows within the sub-basin

RGSUP1 to RGSUP12 = gross water supply available

Not all of the gross monthly water supply will be available for use when reuqired the month. During peak flow periods, some of the water will be wasted unless the peaks can be captured by off-stream storage. A figure of 20% wastage was used to define net supply available. This figure was based on a similar optimization study of the Okanagan River Basin (McNeill, 1987). However, at this stage it is not known how

appropriate it is for the South Saskatchewan River Basin. Equations (RWAS1C) to (RWAS12C) define the available water supply after wastage.

```
(RWAS1C) RNSUP1 - .8 RGSUP1 = 0

(RWAS2C) RNSUP2 - .8 RGSUP2 = 0

(RWAS3C) RNSUP3 - .8 RGSUP3 = 0

(RWAS4C) RNSUP4 - .8 RGSUP4 = 0

(RWAS5C) RNSUP5 - .8 RGSUP5 = 0

(RWAS6C) RNSUP6 - .8 RGSUP6 = 0

(RWAS7C) RNSUP7 - .8 RGSUP7 = 0

(RWAS7C) RNSUP7 - .8 RGSUP7 = 0

(RWAS9C) RNSUP8 - .8 RGSUP8 = 0

(RWAS9C) RNSUP9 - .8 RGSUP9 = 0

(RWAS10C) RNSUP10 - .8 RGSUP10 = 0

(RWAS11C) RNSUP11 - .8 RGSUP11 = 0

(RWAS12C) RNSUP12 - .8 RGSUP12 = 0
```

where:

RNSUP1 to RNSUP12 = monthly supply available after wastage

The next set of equations are straightforward definitions of the amount of water crossing the Alberta-Saskatchewan border. Because of apportionment agreements between the provinces, Saskatchewan is only guaranteed 50% of the natural flow from Alberta. For this reason the amounts designated in the model represent 50% of the longterm (56 year) average transborder flow. The data used to obtain this figure are from

Acres International (1986) which developed a water use forecasting model for the Saskatchewan Nelson Basin. The units are cubic decameters (dam^3) .

(AREL1C) AREL1 = $77 \cdot 127$ dam³

(AREL2C) AREL2 = 68 562

(AREL3C) AREL3 = 168 882

 $(AREL4C) \quad AREL4 = \quad 404 \quad 581$

 $(AREL5C) \quad AREL5 = 724 \quad 193$

(AREL6C) AREL6 = 1 244 057

 $(AREL7C) \quad AREL7 = 848 \quad 429$

(AREL8C) AREL8 = 436 419

(AREL9C) AREL9 = 300 169

(AREL10C) AREL10 = 249 041

(AREL11C) AREL11 = 145 993

(AREL12C) AREL12 = 98 047

The fifty-six-year average natural run-off in the sub-basin is defined in a similar manner in equations (RNAT1C) to (RNAT12C). The run-off data was again obtained from Acres International (1986). However, the basin configuration used in their study was somewhat more aggregated than the configuration used in this project. In the Acres model, the South Saskatchewan River and Lake Diefenbaker were considered as a single node (sub-basin) ending at Saskatoon. In the current study it was assumed that 30% of the naturalized flow for this node occurred in

the upstream river sub-basin. Although the 30% figure is only a first guess, the solution will probably not change if the relative distribution of local natural run-off is different.

(RNAT1C) RROA1 = 2.683 dam³

(RNAT2C) RROA2 = 720

(RNAT3C) RROA3 = 154

(RNAT4C) RROA4 = 64 607

(RNAT5C) RROA5 = $5 \cdot 162$

(RNAT6C) RROA6 = 1669

(RNAT7C) RROA7 = 63814

(RNAT8C) RROA8 = 25 310

(RNAT9C) RROA9 = 23 694

(RNAT10C) RROA10 = 11 597

(RNAT11C) RROA11 = 8 346

(RNAT12C) RROA12 = 2456

Return flows are defined in equations (RRET1C) to (RRET12C). These are a function of the amount and kinds of water using activities in the sub-basin. The only two significant users and sources of return flows are irrigation and domestic users. The return flows from irrigation occur in the heavy irrigation periods from May to September, while the return flows from domestic use are more evenly distributed throughout the year.

```
(RRET1C) RRET1 -.0625RDODO = 0
(RRET2C)
        RRET2
               -.0625RD0D0 = 0
(RRET3C) RRET3 -.0625RDOD0 = 0
(RRET4C) RRET4 -.0625RD0D0 = 0
(RRET5C) RRET5 -.0625RDODO - .229RHEC = 0
(RRET6C) RRET6 - .0625RD0D0 - .438RHEC = 0
        RRET7 - .0625RDODO - .789RHEC = 0
(RRET7C)
(RRET8C) RRET8 - .0625RD0D0 -
                               .546RHEC = 0
(RRET9C) RRET9 - .0625RD0D0 - .492RHEC = 0
(RRET10C) RRET10 - .0625RD0D0 = 0
(RRET11C) RRET11 - .0625RD0D0 = 0
(RRET12C) RRET12 - .0625RD0D0 = 0
```

where:

RDODO = gross annual water withdrawn for domestic use
RHEC = number of hectares irrigated

The coefficients in the return flow equations are based on estimates made in Acres International (1986). The model developed in this study estimated a 36 percent return flow from irrigation in the Lake Diefenbaker sub-basin. The distribution of return flows throughout the months of the year was in direct proportion to the amount of water applied in each month. The coefficients on RHEC in the above equations are obtained by multiplying:

36% x (annual water applied per hectare) x (percent of annual total applied in each month)

The return flows from domestic use are based on 75% return. The return flow is considered to be a constant percentage of domestic use on a month by month basis. The coefficients in the above equations were obtained simply by dividing 75% by twelve.

b. Water Demand Activities

Total water demands (gross withdrawal) are defined by equations (RTOD1C) to (RTOD12C). These equations are fairly straightforward since irrigation and domestic use are the only water users in the sub-basin.

```
(RTOD1C) RAGD1 + RDOD1 - RTOD1 = 0
```

(RTOD2C) RAGD2 + RDOD2 - RTOD2 = 0

(RTOD3C) RAGD3 + RDOD3 - RTOD3 = 0

(RTOD4C) RAGD4 + RDOD4 - RTOD4 = 0

(RTOD5C) RAGD5 + RDOD5 - RTOD5 = 0

(RTOD6C) RAGD6 + RDOD6 - RTOD6 = 0

(RTOD7C) RAGD7 + RDOD7 - RTOD7 = 0

(RTOD8C) RAGD8 + RDOD8 - RTOD8 = 0

(RTOD9C) RAGD9 + RDOD9 - RTOD9 = 0

(RTOD10C) RAGD10 + RDOD10 - RTOD10 = 0

(RTOD11C) RAGD11 + RDOD11 - RTOD11 = 0

(RTOD12C) RAGD12 + RDOD12 - RTOD12 = 0

where:

RAGD1 to RAGD12 = water withdrawn for irrigation in each period

RDOD1 to RDOD12 = water withdrawn for domestic use in each period

RTOD1 to RTOD12 = total water withdrawn in each period

The annual amount of water withdrawn for irrigation is expressed as a function of the number of irrigated hectares in the sub-basin as shown in equation (RAGDOC). The per hectare water application is based on the average year irrigation requirement from Acres International (1986).

$$(RAGDOC)$$
 6.741RHEC - RAGDO = 0

where:

RHEC = number of irrigated hectares

RAGDO = annual amount withdrawn for irrigation

The monthly withdrawals for irrigation are then expressed as percentages of the annual withdrawal as shown in equations (RAGD5C) to RAGD9C). These monthly percentages are the same as used in Acres International (1986).

(RAGD5C) RAGD5 - .091 RAGD0 = 0

(RAGD6C) RAGD6 - .177 RAGD0 = 0

(RAGD7C) RAGD7 - .317 RAGD0 = 0

(RAGD8C) RAGD8 - .218 RAGD0 = 0

(RAGD9C) RAGD9 - .187 RAGD0 = 0

where:

RAGD5 to RAGD9 = amount withdrawn for irrigation in each period.

The next equation sets out an upper limit on the amount of land This limit is set at the current level of available for irrigation. irrigated hectares in the sub-basin, but could be increased when of development. Some problems were levels evaluating future encountered in determining the amount of presently irrigated land in As mentioned, Acres International (1986) used a each sub-basin. different sub-basin configuration so their data could not be used The approach taken was to examine the without some modification. irrigated area for each irrigation district and attempt to place it in the appropriate sub-basin. It is recommended that these estimates be re-calculated based on the latest field estimates.

(RHECC) RHEC < 300 hectares

The variable RHEC has a value of \$350 in the objective function. An initial value of \$350 per hectare was chosen based on the study by 0'Grady et.al. (1983). This figure does not account for the off-farm capital and maintenance costs of the delivery system.

(VALUE) \$350 x RHEC

Domestic demands are fixed in the model as shown in equations (RDOD1C) to (RDOD12C). Because the amount of water used by households was relatively small, it was not worth building domestic use into the objective function. By fixing the amount of domestic water use, we are constraining the model to supply households a certain amount of water before allocating water to non-fixed uses.

The amount of water required for domestic purposes was calculated as follows. First the population of the sub-basin was estimated at 5,500 based on the populations of Kindersly and Leader. A per capita use figure of 350 liters per day was assumed which is the same figure used in the Acres International model (1986) for the Saskatoon area. Monthly totals were then calculated on this basis.

(RDOD1C) RDOD1 = 41 dam^3

(RDOD2C) RDOD2 = 41

(RDOD3C) RDOD3 = 41

(RDOD4C) RDOD4 = 59

(RDOD5C) RDOD5 = 76

(RDOD6C) RDOD6 = 76

(RODO7C) RDOD7 = 76

(RDOD8C) RDOD8 = 76

(RDOD9C) RDOD9 = 76

(RDOD10C) RDOD10 = 59

(RDOD11C) RDOD11 = 41

(RDOD12C) RDOD12 = 41

The annual total of domestic water use is also fixed as shown in equation (RDODOC).

(RDODOC) RDODO = 863 dam³

where:

RDODO = annual water withdrawn for domestic use

c. Supply Demand Balance

This set of constraints set out the condition that in each period the total water withdrawn must be less than the supply available after wastage. Equations (RBAL1C) to (RBAL12C) define the supply demand balance.

```
(RBAL1C) RTOD1 - RNSUP1 < 0
```

(RBAL2C) RTOD2 - RNSUP2 < 0

(RBAL3C) RTOD3 - RNSUP3 < 0

(RBAL4C) RTOD4 - RNSUP4 < 0

(RBAL5C) RTOD5 - RNSUP5 < 0

(RBAL6C) RTOD6 - RNSUP6 < 0

(RBAL7C) RTOD7 - RNSUP7 < 0

(RBAL8C) RTOD8 - RNSUP8 < 0

(RBAL9C) RTOD9 - RNSUP9 < 0

(RBAL10C) RTOD10 - RNSUP10 < 0

(RBAL11C) RTOD11 - RNSUP10 < 0

(RBAL12C) RTOD12 - RNSUP12 < 0

where:

RTOD1 to RTOD12 = total water demand in each period

RNSUP1 to RNSUP12 = net supply available after wastage

The final set of equations for this sub-basin define the remaining flows which will pass to the next reach. Remaining flows are equal to the gross supply minus the total demands as shown in equations (RREM1C) to (RREM12C).

```
RREM1 - RGSUP1 + RTOD1 = 0
(RREM1CO
         RREM2 - RGSUP2 + RTOD2 = 0
(RREM2C)
(RREM3C) RREM3 - RGSUP3 + RTOD3 = 0
(RREM4C) RREM4
               - RGSUP4 + RTOD4 = 0
(RREM5C) RREM5
               - RGSUP5 + RTOD5 = 0
(RREM6C) RREM6 - RGSUP6 + RTOD6 = 0
(RREM7C)
         RREM7
               -. RGSUP7 + RTOD7
                         + RTOD8
(RREM8C)
         RREM8
               - RGSUP8
(RREM9C) RREM9 - RGSUP9 + RTOD9
(RREM10C) RREM10 - RGSUP10 + RTOD10 = 0
(RREM11C) RREM11 - RGSUP11 + RTOD11 = 0
(RREM12C) RREM12 - RGSUP12 + RTOD12 = 0
```

where:

RREM1 to RREM12 = remaining flow passing to next reach

The remaining flows pass into Lake Diefenbaker where they form part of the gross supply to the Lake Diefenbaker sub-basin. Thus the remaining flow constraints are the means by which the sub-basin module is linked to the downstream reaches.

2. Swift Current Creek Sub-basin

This sub-basin is modeled in a similar manner to the river sub-basin. The only difference in model structure is that all water is supplied locally from the sub-basin by natural run-off, with no water entering from upstream.

a. Water Supply Activities

where:

The first set of equations define the total amount of water available in the sub-basin for each period. Total supply consists of the natural run-off in the sub-basin and any return flows from water use. Equations SSUP1C to SSUP12C define the monthly water supplies.

```
(SSUP1C)
         SROA1 + SRET1
                        - SGSUP1 = 0
         SROA2 + SRET2
                        - SGSUP2 = 0
(SSUP2C)
                        - SGSUP3 = 0
         SROA3 + SRET3
(SSUP3C)
(SSUP4C)
         SROA4 + SRET4
                        - SGSUP4 = 0
         SROA5 + SRET5
                        - SGSUP5 = 0
(SSUP5C)
         SROA6 + SRET6
                        - SGSUP6
(SUSP6C)
(SSUP7C) SROA7
                + SRET7
                        - SGSUP7 = 0
(SSUP8C) SROA8 + SRET8
                        - SGSUP8 = 0
(SSUP9C) SROA9 + SRET9
                        - SGSUP9 = 0
(SSUP10C) SROA10 + SRET10 - SGSUP10 = 0
(SSUP11C) SROA11 + SRET11 - SGSUP11 = 0
(SSUP12C) SROA12 + SRET12 - SGSUP12 = 0
```

SROA1 to SROA12 = natural run-off within the sub-basin
SRET1 to SRET12 = return flows within the sub-basin

SGSUP1 to SGSUP12 = gross water supply available

As in the previous sub-basin, a figure of 20% wastage was used to define net supply available. Equations (SWAS1C) to (SWAS12C) define the available water supply after wastage.

```
(SWAS1C) SNSUP1 - .8 SGSUP1 = 0
```

(SWAS2C) SNSUP2 - .8 SGUSP2 = 0

(SWAS3C) SNSUP3 - .8 SGSUP3 = 0

(SWAS4C) SNSUP4 - .8 SGSUP4 = 0

(SWAS5C) SNSUP5 - .8 SGSUP5 = 0

(SWAS6C) SNSUP6 - .8 SGSUP6 = 0

(SWAS7C) SNSUP7 - .8 SGSUP7 = 0

(SWAS8C) SNSUP8 - .8 SGSUP8 = 0

(SWAS9C) SNSUP9 - .8 SGSUP9 = 0

(SWAS10C) SNSUP10 - .8 SGSUP10 = 0

(SWAS11C) SNSUP11 - .8 SGSUP11 = 0

(SWAS12C) SNSUP12 - .8 SGSUP12 = 0

where:

SNSUP1 to SNSUP12 = monthly supply available after wastage

The fifty-six-year average natural run-off in the sub-basin is defined in equations (SNAT1C) to (SNAT12C). The run-off data used to calculate these averages were taken from the computerized files of the forecasting model developed in Acres International (1986).

(SNAT1C) SROA1 = 1213 dam³

(SNAT2C) SROA2 = 342

(SNAT3C) SROA3 = 606

(SNAT4C) SROA4 = 15 928

(SNAT5C) SROA5 = 2 198

```
(SNAT6C) SROA6 = 5799
```

(SNAT7C) SROA7 = 25 323

(SNAT8C) SROA8 = 12 734

(SNAT9C) SROA9 = 10 056

(SNAT10C) SROA10 = 5154

(SNAT11C) SROA11 = 2 642

(SNAT12C) SROA12 = 1516

Return flows are defined in equations (SRET1C) to (SRET12C). These are specified in a similar manner to the upstream South Saskatchewan River sub-basin except that there is also a small amount of return flow from industry in the sub-basin. Return flows from irrigation are slightly higher at about 38%. Return flows from industry are set at 92.4% from industrial water use and 87.6% from domestic water use. These figures are taken directly from Acres International (1986).

```
(SRET1C) SRET1 -.073 SDODO -.077 SINDO =0
```

(SRET2C) SRET2 - .073 SDODO - .077 SINDO = 0

(SRET3C) SRET3 - .073 SDOD0 - .077 SIND0 = 0

(SRET4C) SRET4 - .073 SDOD0 - .077 SIND0 = 0

(SRET5C) SRET5 - .073 SDODO - .077 SINDO - .362 SHEC = 0

(SRET6C) SRET6 - .073 SDODO - .077 SINDO - .581 SHEC = 0

(SRET7C) SRET7 - .073 SDODO - .077 SINDO - .806 SHEC = 0

(SRET8C) SRET8 -.073 SDODO -.077 SINDO -.637 SHEC =0

(SRET9C) SRET9 - .073 SDODO - .077 SINDO - .678 SHEC = 0

(SRET10C) SRET10 - .073 SD0D0 - .077 SIND0 = 0

(SRET11C) SRET11 - .073 SDODO - .077 SINDO = 0

(SRET12C) SRET12 - .073 SDODO - .077 SINDO = 0

where:

SDODO = gross annual water withdrawn for domestic use

SINDO = gross annual water withdrawn for industrial use

SHEC = number of hectares irrigated in the sub-basin

b. Water Demand Activities

Total water demands (gross withdrawal) are defined by equations (STOD1C) to (STOD12C). Agriculture, domestic and industry are the three water using sectors in the sub-basin.

(STOD1C) SAGD1 + SDOD1 + SIND1 - STOD1 = 0

(STOD2C) SAGD2 + SDOD2 + SIND2 - STOD2 = 0

(STOD3C) SAGD3 + SDOD3 + SIND3 - STOD3 = 0

(STOD4C) SAGD4 + SDOD4 + SIND4 - STOD4 = 0

(STOD5C) SAGD5 + SDOD5 + SIND5 - STOD5 = 0

(STOD6C) SAGD6 + SDOD6 + SIND6 - STOD6 = 0

(STOD7C) SAGD7 + SDOD7 + SIND7 - STOD7 = 0

(STOD8C) SAGD8 + SDOD8 + SIND8 - STOD8 = 0

(STOD9C) SAGD9 + SDOD9 + SIND9 - STOD9 = 0

(STOD10C) SAGD10 + SDOD10 + SIND10 - STOD10 = 0

(STOD11C) SAGD11 + SDOD11 + SIND11 - STOD11 = 0

(STOD12C) SAGD12 + SDOD12 + SIND12 - STOD12 = 0

where:

SAGD1 to SAGD12 = water withdrawn for irrigation in each period SD0D1 to SD0D12 = water withdrawn for domestic use in each period SIND1 to SIND12 = water withdrawn for industry in each period ST0D1 to ST0D12 = total water withdrawn in each period

The annual amount of water withdrawn for irrigation is expressed as a function of the number of irrigated hectares in the sub-basin as shown in equation (SAGDOC). The figure of 7.98 dam³ per hectare is the application rate used in Acres International (1986).

(SAGDOC)
$$7.98$$
 SHEC - SAGDO = 0

where:

SHEC = number of irrigated hectares

SAGDO = annual amount withdrawn for irrigation

The monthly withdrawals for irrigation are then expressed as percentages of the annual withdrawal as shown in equations (SAGD5C) TO SAGD9C).

(SAGD5C) SAGD5 - .116 SAGD0 = 0

(SAGD6C) SAGD6 - .184 SAGD0 = 0

(SAGD7C) SAGD7 - .259 SAGD0 = 0

(SAGD8C) SAGD8 - .222 SAGD0 = 0

(SAGD9C) SAGD9 - .219 SAGD0 = 0

where:

SAGD5 to SAGD9 = amount withdrawn for irrigation in each time period.

The next equation sets out an upper limit on the amount of land available for irrigation. This limit is set at the current level of irrigated hectares in the sub-basin, but could be increased when evaluating future levels of development. The current level is taken from Acres International (1986).

(SHECC) SHEC < 8000 hectares

The variable SHEC representing the number of irrigated hectares in the sub-basin also appears in the objective function. For every hectare irrigated, the value of the objective function increases by \$350. This is a very rough figure based on the study by O'Grady et.al. (1983). This figure does not account for the off-farm capital and maintenance costs of the delivery system.

(VALUE) \$350 x SHEC

There is a small amount of industrial water use in the sub-basin as reported in Acres International (1986). The model constrains the solution to supply water to industry at current levels. Total annual water use is defined in equation (SINDOC).

(SINDOC) SINDO = $329 \, \text{dam}^3$

where:

SINDO = annual water withdrawn for industry in the sub-basin.

The monthly water supplied to industry is expressed as a percentage of annual industrial water use as shown in equations (SIND1C) to (SIND12C).

```
(SIND1C) SIND1 - .0833 SIND0 = 0
(SIND2C) SIND2 - .0833 SIND0 = 0
(SIND3C) SIND3 - .0833 SIND0 = 0
(SIND4C) SIND4 - .0833 SIND0 = 0
(SIND5C) SIND5 - .0833 SIND0 = 0
(SIND6C) SIND6 - .0833 SIND0 = 0
(SIND7C) SIND7 - .0833 SIND0 = 0
(SIND8C) SIND8 - .0833 SIND0 = 0
(SIND9C) SIND9 - .0833 SIND0 = 0
(SIND10C) SIND10 - .0833 SIND0 = 0
(SIND11C) SIND11 - .0833 SIND0 = 0
(SIND11C) SIND11 - .0833 SIND0 = 0
```

where:

SIND1 to SIND2 = monthly withdrawals for industry

Domestic demands are fixed in the model as shown in equations (SDOD1C) to (SDOD12C). Domestic requirements are based on a population of 21 518 with a per capita use of 524 liters per day as used in Acres International (1986).

(SDOD1C) SDOD1 = 240 dam^3

(SDOD2C) SDOD2 = 240

(SDOD3C) SDOD3 = 240

(SDOD4C) SDOD4 = 343

(SDOD5C) SDOD5 = 446

(SDOD6C) SDOD6 = 446

(SODO7C) SDOD7 = 446

(SDOD8C) SDOD8 = 446

(SDOD9C) SDOD9 = 446

(SDOD1OC) SDOD10 = 343

(SDOD11C) SDOD11 = 240

(SDOD12C) SDOD12 = 240.

The annual total of domestic water use is also fixed as shown in equation (SDODOC).

(SDODOC) SDODO = 4.116 dam^3

where:

SDODO = annual water withdrawn for domestic use

c. Supply Demand Balance

This set of constraints set out the condition that in each period the total water withdrawn must be less than the supply available after wastage. Equations (SBAL1C) to (SBAL12C) define the supply demand balance.

```
STOD1 - SNSUP1 < 0
(SBAL1C)
(SBAL2C)
         STOD2 - SNSUP2
         STOD3 - SNSUP3
(SBAL3C)
         STOD4 - SNSUP4
                          < 0
(SBAL4C)
(SBAL5C)
         STOD5 - SNSUP5 < 0
         STOD6 - SNSUP6
                          < 0
(SBAL6C)
(SBAL7C)
         STOD7
               SNSUP7
         STOD8
               - SNSUP8
(SBAL8C)
(SBAL9C)
        STOD9 - SNSUP9 < 0
(SBAL10C) STOD10 - SNSUP10 < 0
(SBAL11C) STOD11 - SNSUP10 < 0
```

(SBAL12C) STOD12 - SNSUP12 < 0

where:

STOD1 to STOD12 = total water demands in each period

SNSUP1 to SNSUP12 = net supply of water available after wastage

The final set of constraints for this sub-basin defines the remaining flows which will pass to the next reach which is the Lake Diefenbaker sub-basin. Remaining flows are equal to the gross supply minus the total demands as shown in equations (SREM1C) to (SREM12C).

```
(SREM1CO SREM1 - SGSUP1 + STOD1 = 0

(SREM2C) SREM2 - SGSUP2 + STOD2 = 0

(SREM3C) SREM3 - SGSUP3 + STOD3 = 0

(SREM4C) SREM4 - SGSUP4 + STOD4 = 0
```

```
(SREM5C) SREM5 - SGSUP5 + ŠTOD5 = 0
```

(SREM6C) SREM6 - SGSUP6 + STOD6 = 0

(SREM7C) SREM7 - SGSUP7 + STOD7 = 0

(SREM8C) SREM8 - SGSUP8 + STOD8 = 0

(SREM9C) SREM9 - SGSUP9 + STOD9 = 0

(SREM10C) SREM10 - SGSUP10 + STOD10 = 0

(SREM11C) SREM11 - SGSUP11 + STOD11 = 0

(SREM12C) SREM12 - SGSUP12 + STOD12 = 0

where:

SREM1 to SREM12 = remaining flow passing to next reach

3. Lake Diefenbaker Sub-Basin

This sub-basin is more complex than the Swift Current Creek and the upstream river sub-basins. Many equations are necessary to model storage operations, hydroelectric generation and recreational values. Downstream flow requirements and the Qu'appelle diversion add further complexity to the model.

a. Water Supply Activities

The first set of equations defines the total amount of water available in the sub-basin for each period. Total supply consists of the natural run-off in the sub-basin, any return flows from water use and the remaining flows from the upstream river and Swift Current Creek sub-basins. Equations LSUP1C to LSUP12C define the monthly water supplies.

```
+ SREM1 + RREM1 - LGSUP1 = 0
    (LSUP1C)
              LROA1 + LRET1
                            + SREM2 + RREM2
                                              - LGSUP2 = 0
              LROA2 + LRET2
    (LSUP2C)
    (LSUP3C)
             LROA3
                    + LRET3
                             + SREM3
                                     + RREM3
                                              - LGSUP3 = 0
                                     + RREM4
    (LSUP4C) LROA4 + LRET4
                             + SREM4
                                               - LGSUP4 = 0
    (LSUP5C) LROA5 + LRET5
                             + SREM5 + RREM5
                                              - LGSUP5 = 0
    (LUSP6C) LROA6 + LRET6
                            + SREM6 + RREM6
                                              - LGSUP6 = 0
                             + SREM7
                                      + RREM7
    (LSUP7C) LROA7
                    + LRET7
                                               - LGSUP7 = 0
    (LSUP8C)
              LROA8 + LRET8
                             + SREM8
                                     + RREM8
                                              - LGSUP8 = 0
                            + SREM9 + RREM9
                                              - LGSUP9 = 0
    (LSUP9C) LROA9 + LRET9
    (LSUP10C) LROA10 + LRET10 + SREM10 + RREM10 - LGSUP10 = 0
    (LSUP11C) LROA11 + LRET11 + SREM11 + RREM11 - LGSUP11 = 0
    (LSUP12C) LROA12 + LRET12 + SREM12 + RREM12 - LGSUP12 = 0
where:
```

LROA1 to LROA12 = natural run-off within the sub-basin

LRET1 to LRET12 = return flows within the sub-basin

SREM1 to SREM12 = remaining flow from Swift Current sub-basin

RREM1 to RREM12 = remaining flow from upstream river sub-basin

LGSUP1 to LGSUP12 = gross water supply available

No wastage is assumed in this sub-basin because of the large amount of storage available. Evaporation losses are significant and are included in the lake level balance equations in a later section.

The fifty-six-year average natural run-off in the sub-basin is defined in equations (LNAT1C) to (LNAT12C). These run-off figures are based on

the data given in Acres International (1986) for the Saskatoon area (node) of their model. However, the Saskatoon node of the Acres model included the South Saskatchewan River as well as Lake Diefenbaker. It was assumed that 40% of the reported local run-off from this node occurred in the Lake Diefenbaker sub-basin.

(LNAT1C) LROA1 = 3.577 dam³

(LNAT2C) LROA2 = 960

(LNAT3C) LROA3 = 205

(LNAT4C) LROA4 = 86 143

(LNAT5C) LROA5 = 6 883

(LNAT6C) LROA6 = 22 226

(LNAT7C) LROA7 = 85 085

(LNAT8C) LROA8 = 33747

(LNAT9C) LROA9 = 31592

(LNAT10C) LROA10 = 15 463

(LNAT11C) LROA11 = 11 128

(LNAT12C) LROA12 = 3275

Return flows are defined in equations (LRET1C) to (LRET12C). These are specified in a similar manner to the Swift Current Creek sub-basin although the coefficients are somewhat lower. Based on Acres International (1986), return flows are estimated to be 75% from domestic use, 24% from industrial use and 36% from irrigation.

```
(LRET1C) LRET1 -.063 LDODO -.02 LINDO =0
```

(LRET2C) LRET2 -.063 LDOD0 -.02 LIND0 =0

(LRET3C) LRET3 -.063 LDODO -.02 LINDO =0

(LRET4C) LRET4 -.063 LDODO -.02 LINDO =0

(LRET5C) LRET5 - .063 LDODO - .02 LINDO - .362 LHEC = 0

(LRET6C) LRET6 - .063 LDODO - .02 LINDO - .581 LHEC = 0

(LRET7C) LRET7 - .063 LDODO - .02 LINDO - .806 LHEC = 0

(LRET8C) LRET8 - .063 LDODO - .02 LINDO - .637 LHEC = 0

(LRET9C) LRET9 -.063 LDODO -.02 LINDO -..678 LHEC =0

(LRET10C) LRET10 - .063 LDODO - .02 LINDO = 0

(LRET11C) LRET11 - .063 LDODO - .02 LINDO = 0

(LRET12C) LRET12 - .063 LDODO - .02 LINDO = 0

where:

LDODO = gross annual water withdrawn for domestic use

LINDO = gross annual water withdrawn for industrial use

LHEC = number of hectares irrigated

b. Water Demand Activities

Total water demands (gross withdrawal) are defined by equations (LTOD1C) to (LTOD12C). Agriculture, domestic and industry are the three consumptive users of water in the sub-basin. Non-consumptive uses including recreation and power generation are dealt with separately.

(LTOD1C) LAGD1 + LDOD1 + LIND1 - LTOD1 =
$$0$$

(LTOD2C) LAGD2 + LDOD2 + LIND2 - LTOD2 =
$$0$$

```
(LTOD3C) LAGD3 + LDOD3
                       + LIND3
                                LTOD3
(LTOD4C) LAGD4 + LDOD4
                        + LIND4
                                LTOD4
                               LTOD5
(LTOD5C) LAGD5 + LDOD5
                       + LIND5
(LTOD6C) LAGD6
                                LTOD6
               + LDOD6
                        + LIND6
                                LTOD7
(LTOD7C) LAGD7
               + LDOD7
                        + LIND7
               + LDOD8
                        + LIND8
                                LTOD8
(LTOD8C) LAGD8
(LTOD9C) LAGD9 + LDOD9 + LIND9 - LTOD9 = 0
(LTOD10C) LAGD10 + LDOD10 + LIND10 - LTOD10 = 0
(LTOD11C) LAGD11 + LDOD11 + LIND11 - LTOD11 = 0
(LTOD12C) LAGD12 + LDOD12 + LIND12 - LTOD12 = 0
```

LAGD1 to LAGD12 = water withdrawn for irrigation in each period

LDOD1 to LDOD12 = water withdrawn for domestic use in each period

LIND1 to LIND12 = water withdrawn for industry in each period

LTOD1 to LTOD12 = total water withdrawn in each period

The annual amount of water withdrawn for irrigation is expressed as a function of the number of irrigated hectares in the sub-basin as shown in equation (LAGDOC). The application rate of 6.74 dam³ per hectare is taken from Acres International (1986).

(LAGDOC) 6.74 LHEC - LAGDO = 0

where:

LHEC = number of irrigated hectares

LAGDO = annual amount withdrawn for irrigation

The monthly withdrawals for irrigation are then expressed as percentages of the annual withdrawal as shown in equations (LAGD5C) to (LAGD9C).

(LAGD5C) LAGD5 - .091 LAGD0 = 0

(LAGD6C) LAGD6 - .177 LAGD0 = 0

(LAGD7C) LAGD7 - .317 LAGD0 = 0

(LAGD8C) LAGD8 - .218 LAGD0 = 0

(LAGD9C) LAGD9 - .197 LAGD0 = 0

where:

LAGD5 to LAGD9 = amount withdrawn for irrigation in each period.

The next equation sets out an upper limit on the amount of land available for irrigation. This limit is set at the current level of irrigated hectares in the sub-basin, but could be increased when evaluating future levels of development. The irrigated area is estimated from information on irrigation districts and totals for the Saskatoon node in the forecasting model developed by Acres International (1986).

(LHECC) LHEC < 8800 hectares

The variable LHEC representing the number of irrigated hectares in the sub-basin appears in the objective function with a value of \$350 per hectare, based on the study by O'Grady et.al. (1983). This figure does not account for the off-farm capital and maintenance costs of the delivery system.

(VALUE) \$350 x RHEC

There is no significant industrial water use in the sub-basin. However, with the expectations of some future development in industry, the equations have been provided using right hand side values of zero as shown in equation (LINDOC).

(LINDOC) LINDO =
$$0$$

where:

LINDO = annual water withdrawn for industry in the sub-basin.

The monthly water supplied to industry is expressed as a percentage of annual industrial water use as shown in equations (LIND1C) to (LIND12C).

(LIND1C) LIND1 -.0833 LIND0 =0

(LIND2C) LIND2 -.0833 LIND0 = 0

(LIND3C) LIND3 -.0833 LIND0 =0

(LIND4C) LIND4 -.0833 LIND0 = 0

(LIND5C) LIND5 -.0833 LIND0 =0

(LIND6C) LIND6 -.0833 LIND0 = 0

(LIND7C) LIND7 -.0833 LIND0 =0

(LIND8C) LIND8 -.0833 LIND0 =0

(LIND9C) LIND9 -.0833 LIND0 = 0

(LIND10C) LIND10 - .0833 LIND0 = 0

(LIND11C) LIND11 - .0833 LIND0 = 0

(LIND12C) LIND12 - .0833 LIND0 = 0

where:

LIND1 to LIND12 = monthly withdrawals for industry

Domestic demands are fixed in the model as shown in equations (LDOD1C) to (LDOD12C). They are based on a population of 4 500 and a per capita daily consumption of 575 liters.

```
dam<sup>3</sup>
(LDOD1C) LDOD1 =
                    34
(LDOD2C)
         LDOD2 =
                    34
(LDOD3C) LDOD3 =
                    34
(LDOD4C) LDOD4 =
                    48
                   62
(LDOD5C) LDOD5 =
(LDOD6C) LDOD6 =
                    62
(LODO7C) LDOD7 =
                    62
(LDOD8C) LDOD8 =
                    62
(LDOD9C) LDOD9. =
                    62
(LDOD10C) LDOD10 =
```

(LDOD11C) LDOD11 =

(LDOD12C) LDOD12 =

where:

LDOD1 to LDOD12 = monthly domestic water use

The amount of water diverted to the Qu'Appelle system is shown in equations (LQAP1C) to (LQAP12C). The monthly diversion of 15 608 dam^3 was obtained by dividing the annual diversion of 187 300 dam^3 as reported in Acres International (1986) by twelve.

```
(LQAP1C) LQAP1 = 15608 dam<sup>3</sup>
```

(LQAP2C) LQAP2 = 15608

(LQAP3C) LQAP3 = 15608

(LQAP4C) LQAP4 = 15608

(LQAP5C) LQAP5 = 15608

(LQAP6C) LQAP6 = 15608

(LQAP7C) LQAP7 = 15608

(LQAP8C) LQAP8 = 15608

(LQAP9C) LQAP9 = 15 608

(LQAP10C) LQAP10 = 15 608

(LQAP11C) LQAP11 = 15 608

(LQAP12C) LQAP12 = 15 608

Net evaporation from Lake Diefenbaker is considered as a water use activity and is shown in equations (LEVAP1C) to (LEVAP12C).

(LEVAP1C) LEVAP1 = $0 mtext{dam}^3$

(LEVAP2C) LEVAP2 = 0

(LEVAP3C) LEVAP3 = 0

(LEVAP4C) LEVAP4 = 0

(LEVAP5C) LEVAP5 = 6992

(LEVAP6C) LEVAP6 = 15808

(LOD07C) LEVAP7 = 35 568

(LEVAP8C) LEVAP8 = 39824

(LEVAP9C) LEVAP9 = 33744

(LEVAP10C)LEVAP10 = 23 104

(LEVAP11C)LEVAP11 = 2 128

(LEVAP12C)LEVAP12 = 0

where:

LEVAP1 to LEVAP12 = net evaporation from Lake Diefenbaker

c. Lake Level Equations

They define the lake level in each period as a function of the previous period's level and inflows and outflows to the lake. They implicitly balance out supply and demand in each time period so no supply-demand balance equations are required. Equations (LLAK1) to (LLAK12) define lake levels in each period. Note that the lake level is actually expressed in terms of volume of Lake Diefenbaker in dam³.

+ LLAKO + LGSUP1 - LTOD1 (LLAK1) - LLAK1 - LREL1 - LCREL1 - LQAP1 = 0 - LEVAP1 (LLAK2) - LLAK2 + LLAK1 + LGSUP2 - LTOD2 - LREL2 - LCREL2 - LQAP2 = 0 - LEVAP2 + LLAK2 + LGSUP3 - LTOD3 (LLAK3) - LLAK3 - LEVAP3 - LREL3 - LCREL3 - LQAP3 = 0 (LLAK4) - LLAK4 + LLAK3 + LGSUP4 - LTOD4 LEVAP4 - LREL4 - LCREL4 - LQAP4 = 0 (LLAK5) - LLAK5 + LLAK4 + LGSUP5 - LTOD5 - LREL5 - LCREL5 - LQAP5 = 0 - LEVAP5

```
+ LLAK5 + LGSUP6 - LTOD6
(LLAK6) - LLAK6
        - LEVAP6 - LREL6 - LCREL6 - LQAP6 = 0
                  + LLAK6 + LGSUP7 - LTOD7
(LLAK7) - LLAK7
        - LEVAP7 - LREL7 - LCREL7 - LQAP7 = 0
                  + LLAK7 + LGSUP8 - LTOD8
(LLAK8) - LLAK8
        - LEVAP8 - LREL8 - LCREL8 - LQAP8 = 0
                  + LLAK8 + LGSUP9 - LTOD9
(LLAK9) - LLAK9
        - LEVAP9 - LREL9 - LCREL9 - LQAP9 = 0
(LLAK10) - LLAK10 + LLAK9 + LGSUP10 - LTOD10
        - LEVAP10 - LREL10 - LCREL10 - LQAP10 = 0
(LLAK11) - LLAK11 + LLAK10 + LGSUP11 - LTOD11
        - LEVAP11 - LREL11 - LCREL11 - LQAP11 = 0
(LLAK12) - LLAK12 + LLAK11 + LGSUP12 - LTOD12
        - LEVAP12 - LREL12 - LCREL12 - LQAP12 = 0
```

LLAKO = lake level at the start of first period

LLAK1 TO LLAK12 = lake level at the end of each period

LREL1 TO LREL12 = release to river downstream

LCREL1 to LCREL12 = release to canal

LQAP1 to LQAP12 = release to Qu'Appelle diversion

Because the model works on a sustained water yield basis the storage in the lake cannot be mined. This condition is imposed by equation (LLAKOC) which states that the end-of-year storage must be equal to the beginning-of-year storage.

(LLAKOC) LLAKO - LLAK12 = 0

There are also some restrictions on maximum and minimum lake levels. The maximum lake levels are based on maximum no-flood levels from the operating rule curve for Lake Diefenbaker. Equations (LMAX1C) to (LMAX12C) define the maximum lake level expressed in volume of water contained in Lake Diefenbaker.

- (LMAX1C) LLAK1 < 9 372 800 dam³
- (LMAX2C) LLAK2 < 9 372 800
- (LMAX3C) LLAK3 < 9 372 800
- (LMAX4C) LLAK4 < 9 372 800
- (LMAX5C) LLAK5 < 9 372 800
- (LMAX6C) LLAK6 < 9 372 800
- (LMAX7C) LLAK7 < 9 372 800
- (LMAX8C) LLAK8 < 9 372 800
- (LMAX9C) LLAK9 < 9 372 800
- (LMAX10C) LLAK10 < 9 372 800
- (LMAX11C) LLAK11 < 9 372 800
- (LMAX12C) LLAK12 < 9 372 800

The minimum lake levels as set out in the Lake Diefenbaker rule curve are shown in equations (LMIN1C) to (LMIN12C).

- (LMIN1C) LLAK1 > 5 424 000 dam³
- (LMIN2C) LLAK2 > 5 424 000
- (LMIN3C) LLAK3 > 5 424 000

```
(LMIN4C) LLAK4 > 5 424 000
```

(LMIN5C) LLAK5 > 5 424 000

(LMIN6C) LLAK6 > 5 424 000

(LMIN7C) LLAK7 > 5 424 000

(LMIN8C) LLAK8 > 5 424 000

(LMIN9C) LLAK9 > 5 424 000

(LMIN10C) LLAK10 > 5 424 000

(LMIN11C) LLAK11 > 5 424 000

(LMIN12C) LLAK12 > 5 424 000

Minimum flow requirements for the river downstream are imposed on the model. These are based on a minimum flow of 1500 cubic feet per second required for the city of Saskatoon. Equations (LINS1C) to (LINS12C) define the minimum flows expressed as total monthly release in dam^3 .

(LINS1C) LREL1 > 113 382 dam³

(LINS2C) LREL2 > 102 816

(LINS3C) LREL3 > 113 382

(LINS4C) LREL4 > 110 160

(LINS5C) LREL5 > 113 382

(LINS6C) LREL6 > 110 160

(LINS7C) LREL7 > 113 382

(LINS8C) LREL8 > 113 382

(LINS9C) LREL9 > 110 160

(LINS10C) LREL10 > 113 382

(LINS11C) LREL11 > 110 106

(LINS12C) LREL12 > 113 382

Maximum downstream flows are also imposed. Flows above these levels are considered to cause flood damages. These constraints are shown in equations (LFLD1C) to (LFLD12C).

(LFLD1C) LREL1 < 1 607 040 dam³

(LFLD2C) LREL2 < 1 451 520

(LFLD3C) LREL3 < 1 607 040

(LFLD4C) LREL4 < 1 555 200

(LFLD5C) LREL5 < 1 607 040

(LFLD6C) LREL6 < 1 555 520

(LFLD7C) LREL7 < 1 607 040

(LFLD8C) LREL8 < 1 607 040

(LFLD9C) LREL9 < 1 555 520

(LFLD10C) LREL10 < 1 607 040

(LFLD11C) LREL11 < 1 555 520

(LFLD12C) LREL12 < 1 607 040

d. Hydropower Generation

Hydropower generation is a non-linear function of head and flows.

Modeling this relationship requires a number of equations to represent
maximum turbine capacity, flows through the turbines and reservoir

head. Some scaling adjustments were also made to keep objective function coefficients from becoming too small relative to the coefficients in the constraints.

The first set of constraints define the amount of flow through turbines and limit this flow to being less than or equal to the actual flow released from the reservoir as shown in equations (HCTF1C) to (HCTF12C). For scaling purposes the reservoir release is expressed in thousands of dam³ in these equations. This change in units is accounted for by multiplying the objective function value for hydropower by 1000 in a later set of equations.

```
(HCTF1C) HREL1 - .001 LREL1 < 0
```

(HCTF2C) HREL2 - .001 LREL2 < 0

(HCTF3C) HREL3 -.001 LREL3 < 0

(HCTF4C) HREL4 - .001 LREL4 < 0

(HCTF5C) HREL5 -.001 LREL5 < 0

(HCTF6C) HREL6 - .001 LREL6 < 0

(HCTF7C) HREL7 - .001 LREL7 < 0

(HCTF8C) HREL8 -.001 LREL8 < 0

(HCTF9C) HREL9 -.001 LREL9 < 0

(HCTF10C) HREL10 - .001 LREL10 < 0

(HCTF11C) HREL11 - .001 LREL11 < 0

(HCTF12C) HREL12 - .001 LREL12 < 0

The amount of water used for power generation is also limited by the capacity of the turbines. Equations (HCTF1C) to (HCTF12C) define monthly maximum flows for power generation based on turbine capacity. The monthly maximum flows are based on a maximum discharge total of 425 cubic meters per second for the Coteau Creek station as reported in the Saskatchewan Nelson Basin study by the Prairie Provinces Water Board (1986).

(HCTT1C) HREL1 < 1116.9 thousands of dam³

(HCTT2C) HREL2 < 1116.9

(HCTT3C) HREL3 < 1116.9

(HCTT4C) HREL4 < 1116.9

(HCTT5C) HREL5 < 1116.9

(HCTT6C) HREL6 < 1116.9

(HCTT7C) HREL7 < 1116.9

(HCTT8C) HREL8 < 1116.9

(HCTT9C) HREL9 < 1116.9

(HCTT10C) HREL10 < 1116.9

(HCTT11C) HREL11 < 1116.9

(HCTT12C) HREL12 < 1116.9

A power generation formula was estimated from a graph of reservoir elevation versus generating capacity. The graph was used to determine the change in power generating capacity associated with a unit drop in reservoir elevation at various elevations (heads). Full details of the calculation are presented in appendix two. It is recommended that actual power generation formulae from Saskatchewan Power Corporation should be used in the model in place of the estimates made from the power capacity curve for Lake Diefenbaker.

The amount of power generated is a function of head as well as flow through the turbines. The head is stated as a function of the average lake level during the month as shown in equations (LHED1C) to (LHED12C). For scaling purposes the head is expressed in millimetres.

```
(LHED1C) - HED1 + .00143 LLAKO + .00143 LAK1 = 7 223 millimetres

(LHED2C) - HED2 + .00143 LLAK1 + .00143 LAK2 = 7 223

(LHED3C) - HED3 + .00143 LLAK2 + .00143 LAK3 = 7 223

(LHED4C) - HED4 + .00143 LLAK3 + .00143 LAK4 = 7 223

(LHED5C) - HED5 + .00143 LLAK4 + .00143 LAK5 = 7 223

(LHED6C) - HED6 + .00143 LLAK5 + .00143 LAK6 = 7 223

(LHED7C) - HED7 + .00143 LLAK6 + .00143 LAK7 = 7 223

(LHED7C) - HED8 + .00143 LLAK7 + .00143 LAK8 = 7 223

(LHED9C) - HED9 + .00143 LLAK8 + .00143 LAK8 = 7 223

(LHED10C) - HED10 + .00143 LLAK8 + .00143 LAK9 = 7 223

(LHED10C) - HED10 + .00143 LLAK9 + .00143 LAK10 = 7 223

(LHED11C) - HED11 + .00143 LLAK9 + .00143 LAK10 = 7 223
```

(LHED12C) - HED12 + .00143 LLAK11 + .00143 LAK12 = 7 223

LHED1 to LHED12 = head for power generation (millimetres)

The production of power is based on the product of head and flow. The following formula gives power generation in megawatt hours for a given month (see appendix A).

power generation = .007403 LHED x HREL (MWH)

Multiplying by the price of hydro-power gives the value of power generation.

value of power = price $x \cdot 007403$ LHED x HREL generation

A two price system for hydropower is used. In the winter months, October to March, a price of \$118 per megawatt-hour is used. In the remaining summer months a lesser price of \$43 per megawatt-hour is used in the objective function. This price system is roughly based on B.C. Hydro costs of power production, with the high price being for thermo generated power and the low price being for hydro power. It is recommended that these prices be reviewed and revised in order to reflect power values for Saskatchewan. Using these prices the following objective function values are generated. This non-linear portion of the objective function will appear in a separate sub-routine (see Chapter 3).

(VALUE) .8764 LHED1 X HREL1

(VALUE) .8764 LHED2 X HREL2

(VALUE) .31949 LHED4 X HREL4

(VALUE) .31949 LHED5 X HREL5

(VALUE) .31949 LHED6 X HREL6

(VALUE) .31949 LHED7 X HREL7

(VALUE) .31949 LHED8 X HREL8

(VALUE) .31949 LHED9 X HREL9

(VALUE) .8764 LHED10 X HREL10

(VALUE) .8764 LHED11 X HREL11

(VALUE) .8764 LHED12 X HREL12

e. Recreational Activity

Instead of maximizing recreational gains from lake level operations, the model is set out to minimize recreational losses, which has the same effect and is easier to model. A variable representing recreational lake levels is introduced in equations (LRLV5C) to (LRLV9C). Recreational activity in the other time periods is assumed to be insignificant. These equations state that the recreational lake level is less than or equal to the actual lake level.

(LRLV5C) LREC5 - LLAK5 < 0

(LRLV6C) LREC6 - LLAK6 < 0

(LRLV7C) LREC7 - LLAK7 < 0

(LRLV8C) LREC8 - LLAK8 < 0

(LRLV9C) LREC9 - LLAK9 < 0

LREC5 to LREC9 = recreational lake levels

Maximum recreational values are reached when the lake level is at a height of 555.3 meters (8,682,000) dam³ in volume). No additional recreational benefits are considered to occur if lake levels rise above this figure. This limit is modeled by specifying that the recreational lake levels LREC5 to LREC9 must be less than 8,682,000 dam³ as shown in equations (LRMX5C) to (LRMX9C).

- (LRMX5C) LREC5 < 8 682 000 dam³
- (LRMX6C) LREC6 < 8 682 000
- (LRMX7C) LREC7 < 8 682 000
- (LRMX8C) LREC8 < 8 682 000
- (LRMX9C) LREC9 < 8 682 000

The next step is to define a lake level deficiency variable which is the amount by which the lake volume is below 8,682,000 dam³. Recreational losses will be proportional to this variable. This is done in equations (LRLS5C) to (LRLS9C).

- (LRLS5C) $LREC5 + LRLS5 = 8.682\ 000\ dam^{3}$
- (LRLS6C) LREC6 + LRLS6 = 8 682 000
- (LRLS7C) LREC7 + LRLS7 = 8 682 000
- (LRLS8C) LREC8 + LRLS8 = 8 682 000
- (LRLS9C) LREC9 + LRLS9 = 8 682 000

LRLS5 to LRLS9 = lake level difference from optimum recreational elevation

Finally an objective function value for the lake level deficiency variable must be determined. A study by Bjonback(1986) was used as the basis for determing the value associated with recreational losses. His study estimated a loss of \$560,000 (1984 dollars) associated with a lake level 3.5 meters below optimal elevations for recreation. This figure did not reflect losses at Elbow Harbour and Paliser parks or by summer inhabitants of recreational cottages. Based on visitation rates to Elbow and Paliser parks, it was possible to calculate the losses in these areas from a similar drop in lake levels. Including these areas increased the total loss figure to \$763,728. It was assumed that there were 150 cottages at 60% occupancy rate over three summer months. Applying the same loss rate to cottage inhabitants increased the total loss figure to \$819,000. In 1986 dollars the total recreational loss figure is \$884,000.

A drop of 3.5 meters in the elevation of Lake Diefenbaker is equivalent in a loss of volume of 1,233,500 dam³. Dividing the total loss by 1,233,500 dam³ gives an annual loss of \$0.717 per dam³. This figure was then weighted by the relative visitation rates in each month to give monthly values lost per dam³. The monthly values lost are then entered in the objective function as coefficients on the LRLS variables.

(VALUE) - .0541 LRLS5

(VALUE) - .1446 LRLS6

(VALUE) - .2827 LRLS7

(VALUE) - .2203 LRLS8

(VALUE) - :0151 LRLS9

4. SSEWS Canal Sub-basin

The structure of the model for this sub-basin is similar to the river upstream and the Swift current sub-basins. There is no significant storage within the sub-basin with almost all of the water supply coming in the form of release from Lake Diefenbaker. Supply-demand balance equations are the key constraints in the model.

a. Water Supply Activities

Water supply consists of release from Lake Diefenbaker, natural run-off within the sub-basin and return flows. The gross water supply is defined in equations (KSUP1C) to (KSUP12C).

(KSUP1C) LCREL1 + KROA1 + KRET1 - KGSUP1 = 0

(KSUP2C) LCREL2 + KROA2 + KRET2 - KGSUP2 = 0

(KSUP3C) LCREL3 + KROA3 + KRET3 - KGSUP3 = 0

(KSUP4C) LCREL4 + KROA4 + KRET4 - KGSUP4 = 0

(KSUP5C) LCREL5 + KROA5 + KRET5 - KGSUP5 = 0

(KUSP6C) LCREL6 + KROA6 + KRET6 - KGSUP6 = 0

(KSUP7C) LCREL7 + KROA7 + KRET7 - KGSUP7 = 0

```
(KSUP8C) LCREL8 + KROA8 + KRET8 - KGSUP8 = 0
```

LCREL1 to LCREL12 = release of water to canal from lake

KROA1 to KROA12 = natural run-off within the sub-basin

KRET1 to KRET12 = return flows within the sub-basin

KGSUP1 to KGSUP12 = gross water supply available

As in other sub-basins, a figure of 20% wastage was used to define net supply available. Equations (KWAS1C) to (KWAS12C) define the available water supply after wastage.

```
(KWAS1C) KNSUP1 - .8 KGSUP1 = 0
```

(KWAS2C) KNSUP2 - .8 KGUKP2 = 0

(KWAS3C) KNSUP3 - .8 KGSUP3 = 0

(KWAS4C) KNSUP4 - .8 KGSUP4 = 0

(KWAS5C) KNSUP5 - .8 KGSUP5 = 0

(KWAS6C) KNSUP6 - .8 KGSUP6 = 0

(KWAS7C) KNSUP7 - .8 KGSUP7 = 0

(KWAS8C) KNSUP8 - .8 KGSUP8 = 0

(KWAS9C) KNSUP9 - .8 KGSUP9 = 0

(KWAS10C) KNSUP10 - .8 KGSUP10 = 0

(KWAS11C) KNSUP11 - .8 KGSUP11 = 0

(KWAS12C) KNSUP12 - .8 KGSUP12 = 0

where:

KNSUP1 to KNSUP12 = monthly supply available after wastage

At the time of writing it was not known if there was any significant natural run-off in the sub-basin. However, because of its small area, it was assumed that local supply was insignificant as shown in equations (KNAT1C) to (KNAT12C).

(KNAT1C) KROA1 = 0

(KNAT2C) KROA2 = 0

(KNAT3C) KROA3 = 0

(KNAT4C) KROA4 = 0

(KNAT5C) KR0A5 = 0

(KNAT6C) KROA6 = 0

(KNAT7C) KROA7 = 0

(KNAT8C) KROA8 = 0

(KNAT9C) KROA9 = 0

(KNAT10C) KROA10 = 0

(KNAT11C) KROA11 = 0

(KNAT12C) KROA12 = 0

Return flows are defined in equations (KRET1C) to (KRET12C). Return flows from domestic, agricultural and industrial uses are shown in

equations (KRET1C) to (KRET12C). Agricultural return flows are based on an annual return of 34.6%. Annual return flows from industry and domestic are 92% and 88% respectively. All of the above figures are taken from Acres International (1986).

```
(KRET1C) KRET1 -.073 KDODO -.077 KINDO =0
```

(KRET2C) KRET2
$$-.073$$
 KDODO $-.077$ KINDO $=0$

(KRET3C) KRET3 -
$$.073$$
 KD0D0 - $.077$ KIND0 = 0

(KRET4C) KRET4
$$-.073$$
 KDODO $-.077$ KINDO $=0$

(KRET5C) KRET5
$$-.073$$
 KDODO $-.077$ KINDO $-.117$ KHEC $=0$

(KRET6C) KRET6
$$- .073 \text{ KD0D0} - .077 \text{ KIND0} - .361 \text{ KHEC} = 0$$

(KRET7C) KRET7 -
$$.073$$
 KDODO - $.077$ KINDO - $.769$ KHEC = 0

(KRET9C) KRET9 -
$$.073$$
 KDODO - $.077$ KINDO - $.361$ KHEC = 0

(KRET10C) KRET10 - .073 KD0D0 - .077 KIND0 = 0

(KRET11C) KRET11 - .073 KDODO - .077 KINDO = 0

(KRET12C) KRET12 - .073 KDODO - .077 KINDO = 0

where:

KDODO = gross annual water withdrawn for domestic use

KINDO = gross annual water withdrawn for industrial use

KHEC = number of hectares irrigated

b. Water Demand Activities

Total water demands (gross withdrawal) are defined by equations (KTOD1C) to (KTOD12C). Agriculture, domestic and industry are included

in these equations, although there is no significant industrial water use in the sub-basin. A later set of equations fixes the industrial water use at zero.

```
+ KIND1
                                  - KTOD1 = 0
(KTOD1C)
         KAGD1
                + KDOD1
                         + KIND2
                                  - KTOD2 = 0
         KAGD2
                + KDOD2
(KTOD2C)
                                  - KTOD3
(KTOD3C)
         KAGD3
                + KDOD3
                         + KIND3
                                  - KTOD4
                + KDOD4
                         + KIND4
(KTOD4C)
         KAGD4
(KTOD5C)
         KAGD5
                + KDOD5
                         + KIND5
                                  KTOD5
                                  KTOD6
(KTOD6C)
         KAGD6
                + KDOD6
                         + KIND6
(KTOD7C) KAGD7
                 + KDOD7
                          + KIND7
                                   KTOD7
                                   - KTOD8
(KTOD8C) KAGD8
                + KDOD8
                         + KIND8
(KTOD9C) KAGD9 + KDOD9
                         + KIND9
                                   - KTOD9
(KTOD10C) KAGD10 + KDOD10 + KIND10 - KTOD10 = 0
(KTOD11C) KAGD11 + KDOD11 + KIND11 - KTOD11 = 0
(KTOD12C) KAGD12 + KDOD12 + KIND12 - KTOD12 = 0
```

where:

KAGD1 to KAGD12 = water withdrawn for irrigation in each period

KDOD1 to KDOD12 = water withdrawn for domestic use in each period

KIND1 to KIND12 = water withdrawn for industry in each period

KTOD1 to KTOD12 = total water withdrawn in each period

The annual amount of water withdrawn for irrigation is expressed as a function of the number of irrigated hectares in the sub-basin as shown in equation (KAGDOC). The annual application rate of 5.83 dam³ per

hectare is taken from Acres International (1986).

(KAGDOC) 5.83 KHEC - KAGDO = 0

where:

KHEC = number of irrigated hectares

KAGDO = annual amount withdrawn for irrigation

The monthly withdrawals for irrigation are then expressed as percentages of the annual withdrawal as shown in equations (KAGD5C) TO KAGD9C).

(KAGD5C) KAGD5 - .059 KAGD0 = 0

(KAGD6C) KAGD6 - .178 KAGD0 = 0

(KAGD7C) KAGD7 - .381 KAGD0 = 0

(KAGD8C) KAGD8 - .191 KAGD0 = 0

(KAGD9C) KAGD9 - .190 KAGD0 = 0

where:

KAGD5 to KAGD9 = amount withdrawn for irrigation in each period.

The next equation sets out an upper limit on the amount of land available for irrigation. This limit is set at the current level of irrigated hectares in the sub-basin, but could be increased when evaluating future levels of development. The figure of 20 950 hectares is an estimate based on data from Acres International (1986) and the estimated area of the South Saskatchewan River Irrigation District.

(KHECC) KHEC < 20 950 hectares

The variable KHEC representing the number of irrigated hectares in the sub-basin is an objective function activity with a value of \$350 per hectare based on the study by O'Grady et.al. (1983). This figure does not account for the off-farm capital and maintenance costs of the delivery system.

(VALUE) $$350 \times KHEC$

It is not known whether there is any significant industrial water use in the sub-basin. Initially the model sets industrial water use at nil as shown in equation (KINDOC).

(KINDOC) KINDO = 0

where:

KINDO = annual water withdrawn for industry in the sub-basin.

The monthly water supplied to industry is expressed as a percentage of annual industrial water use as shown in equations (KIND1C) to (KIND12C).

(KIND1C) KIND1 -.0833 KIND0 = 0

(KIND2C) KIND2 -.0833 KIND0 = 0

(KIND3C) KIND3 -.0833 KIND0 = 0

(KIND4C) KIND4 -.0833 KIND0 = 0

(KIND5C) KIND5 -.0833 KIND0 = 0

(KIND6C) KIND6 -.0833 KIND0 =0

(KIND7C) KIND7 -.0833 KIND0 = 0

```
(KIND8C) KIND8 -.0833 KIND0 = 0
```

(KIND9C) KIND9 -.0833 KIND0 =0

(KIND10C) KIND10 - .0833 KIND0 = 0

(KIND11C) KIND11 - .0833 KIND0 = 0

(KIND12C) KIND12 - .0833 KIND0 = 0

where:

KIND1 to KIND2 = monthly withdrawals for industry

Domestic demands are fixed in the model as shown in equations (KDOD1C) to (KDOD12C). These demands are based on a population of 20,150 with a per capita consumption of 350 liters per day. The population figure is an estimate derived by taking one half of the rural population of the Saskatoon node used in the forecasting model developed by Acres International (1986).

(KDOD1C) KDOD1 = 150 dam^3

(KDOD2C) KDOD2 = 150

 $(KDOD3C) \cdot KDOD3 = 150$

(KDOD4C) KDOD4 = 215

(KDOD5C) KDOD5 = 279

(KDOD6C) KDOD6 = 279

 $(KODO7C) \quad KDOD7 = 279$

(KDOD8C) KDOD8 = 279

(KDOD9C) KDOD9 = 279

(KDOD10C) KDOD10 = 215

(KDOD11C) KDOD11 = 150

(KDOD12C) KDOD12 = 150

The annual total of domestic water use is also fixed as shown in equation (KDODOC).

(KDODOC) KDODO = 2.575 dam^3

where:

KDODO = annual water withdrawn for domestic use

c. Supply Demand Balance

This set of constraints set out the condition that in each period the total water withdrawn must be less than the supply available after wastage. Equations (KBAL1C) to (KBAL12C) define the supply demand balance.

(KBAL1C) KTOD1 - KNSUP1 < 0

(KBAL2C) KTOD2 - KNSUP2 < 0

(KBAL3C) KTOD3 - KNSUP3 < 0

(KBAL4C) KTOD4 - KNSUP4 < 0

(KBAL5C) KTOD5 - KNSUP5 < 0

(KBAL6C) KTOD6 - KNSUP6 < 0

(KBAL7C) KTOD7 - KNSUP7 < 0

(KBAL8C) KTOD8 - KNSUP8 < 0

(KBAL9C) KTOD9 - KNSUP9 < 0

(KBAL10C) KTOD10 - KNSUP10 < 0

(KBAL11C) KTOD11 - KNSUP10 < 0

(KBAL12C) KTOD12 - KNSUP12 < 0

where:

KTOD1 to KTOD12 = total water demand in each period
KNSUP1 to KNSUP12 = water available after wastage

The final set of equations for this sub-basin define the remaining flows which will leave the South Saskatchewan River system. Remaining flows are equal to the gross supply minus the total demands as shown in equations (KREM1C) to (KREM12C).

(KREM1CO KREM1 - KGSUP1 + KTOD1 = 0

(KREM2C) KREM2 - KGSUP2 + KTOD2 = 0

(KREM3C) KREM3 - KGSUP3 + KTOD3 = 0

(KREM4C) KREM4 - KGSUP4 + KTOD4 = 0

(KREM5C) KREM5 - KGSUP5 + KTOD5 = 0

(KREM6C) KREM6 - KGSUP6 + KTOD6 = 0

(KREM7C) KREM7 - KGSUP7 + KTOD7 = 0

(KREM8C) KREM8 - KGSUP8 + KTOD8 = 0

(KREM9C) KREM9 - KGSUP9 + KTOD9 = 0

(KREM10C) KREM10 - KGSUP10 + KT0D10 = 0

(KREM11C) KREM11 - KGSUP11 + KTOD11 = 0

(KREM12C) KREM12 - KGSUP12 + KTOD12 = 0

KREM1 to KREM12 = remaining flow passing out of the basin

5. Downstream River Sub-basin

This is the final sub-basin of the system. The model structure is similar to the Swift Current Creek and upstream river sub-basins. The major difference is that remaining flows from the downstream river sub-basin leave the Basin rather than acting as linkages with downstream sub-basins.

a. Water Supply Activities

The first set of equations define the total amount of water available in the sub-basin for each period. Total supply consists of water released from Lake Diefenbaker, the natural run-off in the sub-basin and any return flows from water use. Equations DSUP1C to DSUP12C define the monthly water supplies.

```
(DSUP1C) LREL1 + DROA1 + DRET1 - DGSUP1 = 0
```

(DSUP2C) LREL2 + DROA2 + DRET2 - DGSUP2 = 0

(DSUP3C) LREL3 + DROA3 + DRET3 - DGSUP3 = 0

(DSUP4C) LREL4 + DROA4 + DRET4 - DGSUP4 = 0

(DSUP5C) LREL5 + DROA5 + DRET5 - DGSUP5 = 0

(DUSP6C) LREL6 + DROA6 + DRET6 - DGSUP6 = 0

(DSUP7C) LREL7 + DROA7 + DRET7 - DGSUP7 = 0

(DSUP8C) LREL8 + DROA8 + DRET8 - DGSUP8 = 0

(DSUP9C) LREL9 + DROA9 + DRET9 - DGSUP9 = 0

(DSUP10C) LREL10 + DROA10 + DRET10 - DGSUP10 = 0

```
(DSUP11C) LREL11 + DROA11 + DRET11 - DGSUP11 = 0
```

(DSUP12C) LREL12 + DROA12 + DRET12 - DGSUP12 = 0

where:

DROA1 to DROA12 = natural run-off within the sub-basin.

DRET1 to DRET12 = return flows within the sub-basin

DGSUP1 to DGSUP12 = gross water supply available

As in other sub-basins, a figure of 20% wastage was used to define net supply available. Equations (DWAS1C) to (DWAS12C) define the available water supply after wastage.

```
(DWAS1C) DNSUP1 - .8 DGSUP1 = 0
```

(DWAS2C) DNSUP2 - .8 DGUSP2 = 0

(DWAS3C) DNSUP3 - .8 DGSUP3 = 0

(DWAS4C) DNSUP4 - .8 DGSUP4 = 0

(DWAS5C) DNSUP5 - .8 DGSUP5 = 0

(DWAS6C) DNSUP6 - .8 DGSUP6 = 0

(DWAS7C) DNSUP7 - .8 DGSUP7 = 0

(DWAS8C) DNSUP8 - .8 DGSUP8 = 0

(DWAS9C) DNSUP9 - .8 DGSUP9 = 0

(DWAS10C) DNSUP10 - .8 DGSUP10 = 0

(DWAS11C) DNSUP11 - .8 DGSUP11 = 0

(DWAS12C) DNSUP12 - .8 DGSUP12 = 0

where:

DNSUP1 to DNSUP12 = monthly supply available after wastage

The fifty-six-year average natural run-off in the sub-basin is defined in equations (DNAT1C) to (DNAT12C). It was assumed that natural run-off in this sub-basin was equal to 30% of the local natural supply at the Saskatoon node of the forecasting model developed by Acres International (1986).

- (DNAT1C) DROA1 = 2.683 dam³
- (DNAT2C) DROA2 = 720
- (DNAT3C) DROA3 = 154
- (DNAT4C) DROA4 = 64 607
- (DNAT5C) DROA5 = $5 \cdot 162$
- (DNAT6C) DROA6 = 16 669
- (DNAT7C) DROA7 = 63814
- (DNAT8C) DROA8 = 25 310
- (DNAT9C) DROA9 = 23694
- (DNAT10C) DROA10 = 11 597
- (DNAT11C) DROA11 = 8 346
- (DNAT12C) DROA12 = 2 456

Return flows are defined in equations (DRET1C) to (DRET12C). The coefficients are based on an annual return flow of 37% from agriculture, 88% from domestic and 73% from industry.

- (DRET1C) DRET1 .073 DD0D0 .061 DIND0 = 0
- (DRET2C) DRET2 .073 DD0D0 .061 DIND0 = 0

```
(DRET3C) DRET3 -.073 DDOD0 -.061 DIND0 = 0
```

(DRET4C) DRET4 - .073 DD0D0 - .061 DIND0 = 0

(DRET5C) DRET5 - .073 DDODO - .061 DINDO - .362 DHEC = 0

(DRET6C) DRET6 - .073 DDOD0 - .061 DIND0 - .581 DHEC = 0

(DRET7C) DRET7 -.073 DDOD0 -.061 DIND0 -.806 DHEC =0

(DRET8C) DRET8 -.073 DDODO -.061 DINDO -.637 DHEC =0

(DRET9C) DRET9 - .073 DDODO - .061 DINDO - .678 DHEC = 0

(DRET10C) DRET10 - .073 DDOD0 - .061 DIND0 = 0

(DRET11C) DRET11 - .073 DD0D0 - .061 DIND0 = 0

(DRET12C) DRET12 - .073 DD0D0 - .061 DIND0 = 0

where:

DDODO = gross water withdrawn for domestic use

DINDO = gross water withdrawn for industrial use

DHEC = number of hectares irrigated

b. Water Demand Activities

Total water demands (gross withdrawal) are defined by equations (DTOD1C) to (DTOD12C). Agriculture, domestic and industry are the three water using sectors in the sub-basin.

(DTOD1C) DAGD1 + DDOD1 + DIND1 - DTOD1 = 0

(DTOD2C) DAGD2 + DDOD2 + DIND2 - DTOD2 = 0

(DTOD3C) DAGD3 + DDOD3 + DIND3 - DTOD3 = 0

(DTOD4C) DAGD4 + DDOD4 + DIND4 - DTOD4 = 0

(DTOD5C) DAGD5 + DDOD5 + DIND5 - DTOD5 = 0

(DTOD6C) DAGD6 + DDOD6 + DIND6 - DTOD6 = C

(DTOD7C) DAGD7 + DDOD7 + DIND7 - DTOD7 = 0

(DTOD8C) DAGD8 + DDOD8 + DIND8 - DTOD8 = 0

(DTOD9C) DAGD9 + DDOD9 + DIND9 - DTOD9 = 0

(DTOD10C) DAGD10 + DDOD10 + DIND10 - DTOD10 = 0

(DTOD11C) DAGD11 + DDOD11 + DIND11 - DTOD11 = 0

(DTOD12C) DAGD12 + DDOD12 + DIND12 - DTOD12 = 0

where:

DAGD1 to DAGD12 = water withdrawn for irrigation in each period

DDOD1 to DDOD12 = water withdrawn for domestic use in each period

DIND1 to DIND12 = water withdrawn for industry in each period

DTOD1 to DTOD12 = total water withdrawn in each period

The annual amount of water withdrawn for irrigation is expressed as a function of the number of irrigated hectares in the sub-basin as shown in equation (DAGDOC). The annual application of 6.86 dam³ per hectare is taken from Acres International (1986).

(DAGDOC) 6.86 DHEC - DAGDO = 0

where:

DHEC = number of irrigated hectares

DAGDO = annual amount withdrawn for irrigation

The monthly withdrawals for irrigation are then expressed as percentages of the annual withdrawal as shown in equations (DAGD5C) to (DAGD9C).

(DAGD5C) DAGD5 - .094 DAGD0 = 0

(DAGD6C) DAGD6 - .178 DAGD0 = 0

(DAGD7C) DAGD7 - .308 DAGD0 = 0

(DAGD8C) DAGD8 - .223 DAGD0 = 0

(DAGD9C) DAGD9 - .196 DAGD0 = 0

where:

DAGD5 to DAGD9 = amount withdrawn for irrigation in each period.

The next equation sets out an upper limit on the amount of land available for irrigation. This limit is set at the current level of irrigated hectares in the sub-basin, but could be increased when evaluating future levels of development. The current total of 10,070 hectares was arived at by subtracting the sum of the irrigated area in the Lake Diefenbaker, upstream river and Canal sub-basins from the Saskatoon node total of 40,810 hectares in the Acres International (1986) forecasting model.

(DHECC) DHEC < 10 070 hectares

The variable DHEC representing the number of irrigated hectares in the sub-basin has a value in the objective function of \$350 per hectare based on the study by O'Grady et.al. (1983). This figure does not account for the off-farm capital and maintenance costs of the delivery system.

(VALUE) \$200 x RHEC

There is some industrial water use in the sub-basin. The amount of water withdrawn for industry was calculated from the Acres International (1986) model by adding the figures for the Saskatoon and St. Louis nodes. The model constrains the solution to supply water to industry at current levels. Total annual water use is defined in equation (DINDOC).

(DINDOC) DINDO = 7933 dam^3 where:

DINDO = annual water withdrawn for industry in the sub-basin.

The monthly water supplied to industry is expressed as a percentage of annual industrial water use as shown in equations (DIND1C) to (DIND12C).

(DIND1C) DIND1 -.0833 DIND0 = 0

(DIND2C) DIND2 -.0833 DIND0 = 0

(DIND3C) DIND3 -.0833 DIND0 = 0

(DIND4C) DIND4 -.0833 DIND0 =.0833

(DIND5C) DIND5 - .0833 DIND0 = 0

(DIND6C) DIND6 -.0833 DIND0 = 0

(DIND7C) DIND7 -.0833 DIND0 = 0

(DIND8C) DIND8 -.0833 DIND0 =0

(DIND9C) DIND9 -.0833 DIND0 = 0

(DIND10C) DIND10 - .0833 DIND0 = 0

(DIND11C) DIND11 - .0833 DIND0 = 0

(DIND12C) DIND12 - .0833 DIND0 = 0

where:

DIND1 to DIND12 = monthly withdrawals for industry

Domestic demands are fixed in the model as shown in equations (DDOD1C) to (DDOD12C). The consumption figures are based on a population of 205,932 which is the sum of the urban populations of the St. Louis and Saskatoon nodes plus one half of the rural population of the Saskatoon node in Acres International (1986). Per capita daily consumption was estimated at 350 liters per day.

(DDOD1C) DDOD1 = 1534 dam³

(DDOD2C) DDOD2 = 1534

(DDOD3C) DDOD3 = 1534

(DDOD4C) DDOD4 = 2191

(DDOD5C) DDOD5 = 2850

(DDOD6C) DDOD6 = 2850

(DODO7C) DDOD7 = 2850

(DDOD8C) DDOD8 = 2850

(DDOD9C) DDOD9 = 2850

(DDOD10C) DDOD10 = 2191

(DDOD11C) DDOD11 = 1534

(DDOD12C) DDOD12 = 1534

The annual total of domestic water use is also fixed as shown in

equation (DDODOC).

(DDODOC) DDODO = 26 302

where:

DDODO = annual water withdrawn for domestic use

c. Supply Demand Balance

This set of constraints set out the condition that in each period the total water withdrawn must be less than the supply available after wastage. Equations (DBAL1C) to (DBAL12C) define the supply demand balance.

(DBAL1C) DTOD1 - DNSUP1 < 0

(DBAL2C) DTOD2 - DNSUP2 < 0

(DBAL3C) DTOD3 - DNSUP3 < 0

(DBAL4C) DTOD4 - DNSUP4 < 0

(DBAL5C) DTOD5 - DNSUP5 < 0

(DBAL6C) DTOD6 - DNSUP6 < 0

(DBAL7C) DTOD7 - DNSUP7 < 0

(DBAL8C) DTOD8 - DNSUP8 < 0

(DBAL9C) DTOD9 - DNSUP9 < 0

(DBAL10C) DTOD10 - DNSUP10 < 0

(DBAL11C) DTOD11 - DNSUP10 < 0

(DBAL12C) DTOD12 - DNSUP12 < 0

The final set of equations for this sub-basin define the remaining

flows which will leave the Basin. Remaining flows are equal to the gross supply minus the total demands as shown in equations (DREM1C) to (DREM12C).

```
- DGSUP1 + DTOD1 = 0
(DREM1CO
         DREM1
                - DGSUP2
                         + DTOD2 = 0
(DREM2C)
         DREM2
(DREM3C)
         DREM3
                - DGSUP3
                          + DTOD3 = 0
(DREM4C)
         DREM4
                - DGSUP4
                          + DTOD4 = 0
         DREM5
                - DGSUP5 + DTOD5 = 0
(DREM5C)
                          + DTOD6 = 0
(DREM6C)
         DREM6
                - DGSUP6
(DREM7C)
         DREM7
                - DGSUP7
                          + DTOD7
(DREM8C)
         DREM8
                - DGSUP8
                          + DTOD8
         DREM9 - DGSUP9 + DTOD9 = 0
(DREM9C)
(DREM10C) DREM10 - DGSUP10 + DTOD10 = 0
(DREM11C) DREM11 - DGSUP11 + DTOD11 = 0
(DREM12C) DREM12 - DGSUP12 + DTOD12 = 0
```

where:

DREM1 to DREM12 = remaining flow leaving the Basin.

III. HOW TO RUN THE MODEL

This chapter describes how the model can be accessed on the University of British Columbia mainframe computer. A brief description of some basic MTS editor commands is given in order that the user can make changes to the model or test the sensitivity of the solution to certain parameters. Procedures for transferring the model to another installation are also discussed.

A. Signing on to the U.B.C. System

From a remote terminal the U.B.C. system can be accessed either through a modem connection or through DATAPAC. If you have a modem the 2400 baud port can be reached by dialing (604) 222-2400. If you are using DATAPAC the U.B.C. address number is 67200900. Once a connection is established the following identification line should appear on the terminal:

University of B.C. -- General MTS.

Further information may appear on the same line identifying the device and task number. To sign on to the system enter the following command:

\$SIGNON EDPR

The system will print out any messages from the computing centre then will respond with:

Enter user password:

?

Type the password after the questionmark. Note that it will not appear on

the screen as you type it. For security reasons, the password is not stated in this report but can be obtained from the author. The EDPR on the signon command is the Computer ID for a commercial account belonging to Water Planning and Management Branch in Pacific and Yukon Region.

If you have signed on correctly the system will respond with a few lines telling you the time of the sign-on, the remaining funds in the account and the disk usage. When you see the prompt symbol # at the beginning of a blank line you are ready to enter commands to the system.

The University of B.C. mainframe computer uses the MTS operating system. Basic MTS commands necessary to run the model or edit the data files are discussed in this write-up. For further information on using MTS, a basic MTS manual should be consulted. In particular, information on using the editor will likely be valuable if extensive changes to the model are planned.

When you have finished using the system you can sign off with the following command.

\$SIGNOFF

The system will respond by giving you a breakdown of the time you were signed on and the charges that were applied.

B. Running the Quadratic Programming Model

The model can be run by entering the following command:

SRUN R.OBJMOD+NA:MINOS 5=R.CONMOD 6=-OUT 9=R.BASIS

As a result of this command the solution is written into a file named -OUT. It may take a few seconds for the solution to be obtained; possibly longer if the system is very busy. To see the solution, the following command will list the contents of -OUT on the screen:

\$COPY -OUT

The solution is about 30 pages long and gives values of the objective function and of each variable and constraint in the model. The file -OUT is a temporary file which is only maintained during the current signon. A different name for this file can be specified on the run command if so desired. The only requirement is that the file name begin with a '-' (minus sign) which signifies that it is a temporary file and that it be attached to unit 6 on the run command.

If you want to keep the solution on a disk file that is saved for future reference, then leave off the minus sign from the file name. In this case the file you use for the solution output should first be created before you enter the run command. For example, to put the output in a file called 'SOLUTION', enter the following commands:

SCREATE SOLUTION

\$RUN R.OBJMOD+NA:MINOS 5=R.CONMOD 6=SOLUTION 9=R.BASIS

The unit specification 9=R.BASIS is not always required but is recommended in most cases. It is used when a starting basis for the model solution is provided by the user, and can save considerable computing time. Currently a starting basis is provided in the file called R.BASIS.

C. File Structure of the Model

The four files which drive the model are a MINOS command file, a file containing the linear constraint and objective function equations, and FORTRAN source and object files which contain the non-linear portion of the objective function. The names of these files are given below.

<u>Filename</u>	Function
R.SPECSMOD	MINOS command file
R.MOD.DAT	Contains the linear constraints and linear portion of the objective function in MPSX code
R.SUBMOD	Contains the fortran source code for the non-linear portion of the objective function.
R.OBJMOD	Contains the FORTRAN object code for the non-linear portion of the objective function

MINOS Command File (R.SPECSMOD)

This file contains commands that control the MINOS routines that solve the quadratic programming problem. The commands specify the size of the problem, the degree of precision required, whether or not there is a starting basis and a number of other parameters. It is not recommended that any changes be made to this file at this time. The commands contained in it will be appropriate for most changes that might be made to the data files for the model. However, if more non-linear terms are added to the objective function it will be necessary to change the R.SPECSMOD file. If it becomes necessary to make any changes to the MINOS commands, it is

recommended that you consult the author and the MINOS manuals. A listing of the R.SPECSMOD file can be obtained with an MTS command as follows:

\$LIST R.SPECSMOD

2. Linear Portion of the Model (R.MOD.DAT)

The constraints (which are all linear) and the linear elements of the objective function are contained in this file. Standard MPSX format is used for coding the problem. MPSX format is a widely used method of coding linear programs. The file is divided into several sections each divided by a header line as shown below.

NAME (name of run provided by user)

ROWS - section showing the kind and name of each row one row per line

COLUMNS - section showing identifiers for each variable in each
equation including row name, variable name and
coefficient

RHS - section showing the right hand side value for each constraint - one line for each right hand side value

ENDATA - (last line signifying end of data)

The NAME line must be first in the file, containing the word NAME in columns 1 to 4 and a name for the problem in columns 15-22.

a. The ROWS Section

Each row in the model represents a single equation and is classified as either a less than constraint, greater than constraint, equality constraint or a free (objective) row. Each line in the ROWS section specifies the name of the row and the type of row. The row type is represented by a one letter symbol in column 2 or 3 and a user supplied name for the row is shown in columns 5 to 12. The row type symbols are as follows:

- E equality constraint
- L less-than constraint
- G greater-than constraint
- N objective function

The objective function in the model is given the name "VALUE". The constraint names are as shown in chapter II.

b. The COLUMNS Section

The columns section contains one line for each occurrence of a variable in the model. There are usually several lines for each variable because variables often occur in more than one equation. Lines containing the same variable must occur consecutively. Each line in this section contains the variable name in columns 5-12, the name of the row in which it occurs in columns 15-22 and the coefficient on the variable in columns 25-36.

This is a somewhat cumbersome and non-intuitive method of representing

the equations in the model although it is the standard method for many mathematical programming prackages. In a sense, the COLUMNS section is expressing the model column by column instead of row by row (equation by equation). However, once you have worked with the model and studied the listing provided, the method of coding the equations should become more clear.

c. The RHS Section

The RHS section contains the the right-hand-side value for each constraint in the model. There is an option to include more than one set of right-hand-side variables for the model, and then specify which set is to be used with the MINOS control commands. Only one set of right-hand-side variables is included in the model in its current form. A name identifying which set the values belong to is placed in columns 5-12. In the model provided, the name given to the set is "RHS1". Therefore this name appears first on every line in the RHS section. The name of the constraint (row name) appears in columns 15-22, and the actual right-hand-side value appears in columns 25-36.

3. The Non-linear Portion of the Model (R.SUBMOD and R.OBJMOD)

The non-linear portion of the model refers to the section of the objective function which contains the quadratic terms, either cross terms or square terms. Two files are necessary to represent these non-linearities. The first file, R.SUBMOD, contains a fortran subroutine representing the

non-linear portion of the objective function. The second file, R.OBJMOD, is the compiled version (executable code) of the subroutine. To make changes to the non-linear portion of the objective function, R.SUBMOD must be edited then recompiled with the recompiled version going into R.OBJMOD.

A listing of R.SUBMOD is provided in Table 2. It can be seen that there are two vectors X(i) and G(i) represented in this file, both with 24 elements. The X(i) vector is composed of all variables which occur in the non-linear portion of the objective function. The subscript number occurs to the order in which these variables occur in the COLUMNS section of the R.MOD.DAT file. For example X(1) to X(12) are the variables LHED1 to LHED12 while X(13) to X(24) represent the variables HREL1 to HREL12. Lines 12-24 specify the power generation function which is composed of LHED multiplied by HREL in each time period (as discussed in Chapter 2). The price of power is represented by the two variables PHIGH and PLOW which are defined in lines 10 and 11. This is the two tiered price system discussed in chapter 2, where PHIGH represents the high value of power in the winter and PLOW represents the lower power values in the summer months.

The G(i) vector is a vector of partial derivatives of the objective function in corresponding order with the X(i) variables. For example, the element G(1) is the partial derivative of the non-linear objective function with respect to X(1). It is necessary to supply this vector of partial derivatives for use in the MINOS search procedures.

TABLE 2
FORTRAN Program for Non-Linear Portion of Objective Function

```
LIST R.SUBMOD
                                                                                                                                                                                                       SUBROUTINE CALCFG (MODE, N, X, F, G, NSTATE, NPROB)
IMPLICIT REAL*8(A-H, O-Z)
REAL*8 X(N), G(N)
                                                                                                                                                                                               IROUTINE FOR NON-LINEAR OBJECTIVE FUNCTION IN MINOS

IF (NPROB.GT.1) GO TO 999
IF (NSTATE.EQ.1) WRITE (6,101)NPROB
FORMAT ('0', 'FIRST CALL TO CALCFG. PROBLEM # -',13/)
PHIGH=.31949
PLOW=0.31949
AA=X(1)*X(13)*PHIGH
BB=X(2)*X(14)*PHIGH
CC=X(3)*X(15)*PHIGH
DD=X(4)*X(16)*PLOW
EE=X(5)*X(17)*PLOW
FF=X(6)*X(18)*PLOW
GG=X(7)*X(19)*PLOW
OO=X(9)*X(21)*PLOW
OO=X(9)*X(21)*PLOW
OO=X(9)*X(21)*PLOW
PP=X(10)*X(22)*PHIGH
QQ=X(11)*X(23)*PHIGH
RR=X(12)*X(24)*PHIGH
G(1) = X(13)*PHIGH
G(3) = X(15)*PHIGH
G(4) = X(16)*PLOW
G(5) = X(11)*PLOW
G(6) = X(18)*PLOW
G(6) = X(18)*PLOW
G(7) = X(19)*PLOW
G(8) = X(20)*PLOW
G(9) = X(21)*PHIGH
G(11) = X(22)*PHIGH
G(11) = X(22)*PHIGH
G(11) = X(23)*PHIGH
G(11) = X(24)*PHIGH
G(11) = X(24)*PHIGH
G(11) = X(24)*PHIGH
G(11) = X(24)*PHIGH
G(11) = X(25)*PHIGH
G(11) = X(27)*PHIGH
G(11) = X(27)*PHIGH
G(12) = X(24)*PHIGH
G(13) = X(11)*PHIGH
G(14) = X(2)*PHIGH
G(15) = X(31)*PHIGH
G(16) = X(4)*PLOW
G(17) = X(5)*PLOW
G(18) = X(6)*PLOW
G(21) = X(24)*PHIGH
G(21) = X(24)*PHIGH
G(21) = X(44)*PLOW
G(22) = X(10)*PHIGH
G(22) = X(10)*PHIGH
G(22) = X(11)*PHIGH
G(23) = X(11)*PHIGH
G(24) = X(12)*PHIGH
G(24) = X(12)*PHIGH
                                                                                                                                                               SUBROUTINE FOR NON-LINEAR OBJECTIVE FUNCTION IN MINOS
                                                        8901234567890123456789012345678
                                                                                                                                           101
                                                                                                                                               999
                                                                                                                                                                                                            RETURN
```

Lines 1 to 11 and 49 to 51 are standard and will remain unchanged even if changes to the X(i) and G(i) vectors are made. The compiled version of the FORTRAN program is contained in R.OBJMOD. Any changes made to the source code will necessitate a recompiling of the source program with the output copied to R.OBJMOD. This is described in section E-2 below.

D. Using the MTS Editor

It is necessary to use the editor to make any changes to the model. It is also very useful for reading portions of the output file without displaying the complete contents of the file after a run. To invoke the editor use the following command:

SEDIT FILENAME

where FILENAME is the name of the file to be edited. Once you are in edit mode the prompt character changes to a ':' instead of the "#" which signifies MTS command mode.

A few of the basic commands used in the editor are discussed below. Note that these commands to not begin with a \$ sign as do MTS commands.

To print a portion or all of the file use the command:

PRINT (range)

where range is a line number range specified by the user. For example to print lines 5 to 15 of the file use:

PRINT 5 15

To print the whole file use the /FILE range specifier:

PRINT /FILE

If you wish to change the contents of a line of the file there are a couple of ways of doing this. The simplest way is to use the replace command which replaces a specified line of text. For example to replace line six use:

REPLACE 6

The editor will then type the current line 6 and then cue you for the replacement. Type in the new line and press return. If you just want to change a part of the line without replacing the whole line then the ALTER command can be used. For example if the string "IWD" is contained in line 6 and we want to change it to "IWL" then following command will accomplish this:

ALTER 6 'IWD'IWL'

If we want to change every occurrence of IWD to IWL from lines 10 to 20 then the following command would be used:

ALTER@A 10 20 'IWD'IWL'

The modifier @A has been added to the ALTER command to specify that the change is to apply to all occurrences of IWD. Without this modifier, only the first occurrence would be changed. To delete lines from a file use the DELETE command. The following example deletes lines 70 to 100.

DELETE 70 100

After you enter a delete command which deleted five or more lines, the editor will ask you if this is okay. Type in yes or no if you do not really mean to delete the lines. To move lines around in a file use the MOVE command. This command takes a specified range of line numbers and inserts them at a specified location in the file. For example the

following command takes lines 30 to 40 and inserts them at line 10.

MOVE 30 40 to 10

The COPY command is used when you want to copy a range of lines to another location in the file without removing them from their original location. For example the following command copies lines 5 to 10 to a location starting immediately after line 20.

COPY 5 10 to 20

To insert new lines into the file or to tack new lines on to the end of the file it is necessary to go into insert mode. The following command puts you in insert mode ready to insert after line 10. It is important to specify a line number on the INSERT command or else it will assume that you want to insert after the last line that you have accessed.

INSERT 10

After typing the above command you can insert as many lines as you like by just typing them in. When you have finished typing them in hit the break key to get out of insert mode. This places you back in the standard edit mode. A useful function of the editor when examining large files is the SCAN function. This command searches for occurrences of a particular string and prints out all lines containing the string. For example the following command will search the file for all occurrences of "objective function" and print out the lines containing this string.

SCAN@A /FILE 'objective function'

When you have finished your edit session, use the STOP command to put you back into the MTS operating system. For example the following command puts you back into MTS.

STOP

You can tell when you are back in MTS because the prompt character becomes a "#". It is recommended that you create a practice file before you try to edit any of the model files. Use the \$CREATE command in MTS, then use the \$EDIT command on the created file. Begin by using the INSERT edit command to enter some lines. Get out of insert mode by pressing the break key, and then try some of the other edit commands. The attached sample shows a printout of an example edit session.

E. Making Changes to the Model

Before making changes to any of the model files it is recommended that back-up copies be made. A backup copy of a file can be made using the \$CREATE and \$COPY commands in MTS. The following sequence will back-up an existing file which is called MODEL by creating a copy of it called MODEL.BAK.

\$CREATE MODEL.BAK

SCOPY MODEL MODEL.BAK

1. Making Changes to the Linear Portion of the Model

These changes are done simply by editing the R.MOD.DAT file and making the required changes. There are some hints that can facilitate this process which are illustrated by some of the examples below.

Most changes will be fairly simple and will not change the structure of the model. For example if the upper limit on irrigable area were to be changed

in the Swift-Current sub-basin one could search for the row identifier 'SHECC'which is the name of the constraint which specifies the upper bound on irrigated hectares. Go into edit mode and then enter the following command:

SCAN@A /f 'SHECC'

The editor will then print all lines in the file that contain the string 'SHECC'. Note that SHECC appears three places in the file. First it appears in the ROWS section, identified as a less-than row, then it appears in the column section where the variable SHEC is given a coefficient of 1, and finally it appears in the right hand side section where the value of xxxx is given. The search command should print all of these occurrences along with their line numbers. Suppose that the RHS line number containing the value for SHECC is line 2350, then to change the upper limit on irrigated area from 8000 hectares to 10000 hectares use the ALTER command as follows.

ALTER 2350 '8000'10000'

Often the only change desired is to alter a coefficient in one of the equations of the model. For example, suppose we wanted to increase the return flow coefficient from the domestic sector in the Swift Current Basin from .077 to .085 for the sixth period. The name of the constraint function which specifies this relationship is SRET6C. The name of the variable whose coefficient we want to change is SDOMO. First of all use the scan command as above to find all occurrences of SRET6C, then find which occurrence contained the column entry for SDOMO. Say for example

that this turns out to be line 1266, then the following ALTER command would be used.

ALTER 1266 '.077'.085'

Sometimes it might be necessary to add another constraint to the model. To do this it will be necessary to insert lines in the ROWS, COLUMNS and RHS sections. For example say we want to add a constraint that specifies that the remaining flow from the Swift Current Basin is not to exceed 5000 Dam³ in the fourth period. If we give this constraint the name SRMAX4, it would be expressed in equational form as:

(SRMAX4) SREM4 < 5000

To enter this relatively simple constraint in the model requires three edit additions to the R.MOD.DAT file. First the new row name must be entered in the ROWS section. It can be inserted at any point in the ROWS section - but say you have decided to insert it at line 88. Use the following sequence.

INSERT 80

L SRMAX4

< press break key to get out of insert >

Second, the RHS value for the row should be entered in the RHS section. Again it does not matter exactly where this is inserted as long as it is in the RHS section - say for example you want to insert it at line 3049. Then use the following sequence:

INSERT 3049

RHS1 SRMAX4 5000

< press break key to get out of insert >

Finally you will have to enter the value for the coefficient on the SREM4 variable in the new constraint in the COLUMNS section. In this case it does matter where this line is inserted. Recall that entries for the same variable in the COLUMNS section must appear consecutively. Therefore you must use the SCAN command to find out where the SREM4 variables occur in the COLUMNS section. Suppose that the last entry for the SREM4 variable in the COLUMNS section is line 1311, then insert the new entry immediately after this line:

INSERT 1311

SREM4 SRMAX4

1

< press break key to get out of insert >

Note that the coefficient on SRMAX has the value of 1. This completes the entry of the new constraint SRMAX4.

If you wish to completely delete a constraint from the model, the procedure is straightforward. First use the SCAN function to search for all lines containing the constraint name. Then simply delete these lines one by one using a series of DELETE commands.

It may become necessary at some point to add more sub-basins to the model. Using the editor COPY commands you can take advantage of the similar structure for most of the sub-basins. For example if you want to add another sub-basin which has a similar equational structure as the Swift Current Sub-basin, you could simply copy all the lines pertaining to Swift

Current to another section of the file and then use some global alter commands to change the name of the constraints and variables to the correct names for the new sub-basin. As an example, consider adding a new sub-basin for Queen's Creek. All variable and constraint names for this sub-basin are to begin with the letter Q. Suppose that the row names for the Swift Current sub-basin are contained in lines 150 to 300 in the ROWS section, and that the last line of the ROWS section is line 1200. The following command would make a copy of all the Swift Current row names and insert them just after line 1200:

COPY 150 300 to 1200

Because all these lines are being inserted between lines 1200 and the next line, the new line numbers will be represented by decimal fractions such as 1200.001 or 1200.023 etc. You might want to renumber the file at this point using only integer line numbers by entering the following command:

RENUMBER

Note that all Swift Current entries begin with the letter S. Identify the first and last line numbers of the new block you have created (if you have renumbered the file the block will be contained in lines 1201 to 1350) and do a global ALTER command on this block, to change the S's to Q's.

ALTER@A 1201 1350 ' S' Q'

Note that we have identified the string as 'S' instead of 'S' to avoid altering all occurrences of the letter S. The 'S' ensures that S will be changed to Q only when it is the first letter of a constraint or variable name. The same procedure can be followed for the RHS and COLUMNS section to create new blocks starting with the letter Q. Then the only changes to

be made will be to alter the coefficients in the new Q blocks to their correct values for the QUEENS sub-basin.

2. Making Changes to the Non-Linear Portion of the Model

Again the use of the editor will also be required to change the file

R.SUBMOD. However, an additional complication is that the changed file

will have to be recompiled with the resulting object code being placed in

the file R.OBJMOD. Some example edits and re-compiling are given below.

For example, say we want to change the low summer price from .33 to .66. First, edit the R.SUBMOD file:

EDIT R.SUBMOD

Then make the line alteration:

ALTER 6 '.33'.66'

Get back into MTS mode by typing:

STOP

Now that you are back in MTS it is necessary to recompile the FORTRAN program in R.SUBMOD. Do this with the following command:

SRUN *FTN SCARDS=R.SUBMOD

You should then receive a message on the screen saying that there are no errors in the subroutine. If you do get an error message, go back and check the changes you have made to the R.SUBMOD file. The object (executable code) is automatically placed in a temporary file called -LOAD. The next step is to copy the -LOAD file into the R.OBJMOD file.

This is done with the following MTS command.

\$COPY -LOAD R.OBJMOD

You are now ready to re-run the model with the changes.

F. Transferring the Model to Another Computer

Both the MINOS software and the model files would have to be transferred before the model could be run on another computer system. Environment Canada has a licence to use MINOS and the software can legally be transferred to other computers within the department. A copy of the tape containing the MINOS software and user's manuals is available at the Pacific and Yukon Regional Office. If the host computer has tape reading facilities, the tape probably could be read directly. The software is FORTRAN based, and the host computer must have a FORTRAN compiler in order to implement MINOS. According to our discussions with the software distribution center at Stanford University, the MINOS software is flexible and can be adapted to numerous different computer systems. However, the services of a computer specialist with some experience in FORTRAN is recommended.

There are several options for transfer of the model files. It is possible to arrange a direct connection between the U.B.C. computer and the receiving computer system through a modem or through DATAPAC. File transfer programs such as KERMIT could then be used. If the receiving computer has a tape reading facility, the model files could be copied to tape and the tape transferred to the new installation. Probably the

easiest option from the point of view of Pacific and Yukon region is to copy the model files to IBM PC floppy disks and send the disks to the Western and Northern office. The files could then be transferred from the floppy disks to the receiving computer system either through a modem or DATAPAC line.

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APPENDIX A

DERIVATION OF HYDROPOWER EQUATIONS

The general form of a power generation function is shown in equation (A1).

Power = $C \times Head \times Flow$

(A1)

where:

C = a constant
Head = operating head

Flow = flow through the turbines

Equation (A1) is applicable to power generation in any specified time period. In the model, the formula is applied on a monthly basis, using average head during the month and total flow through the turbines in the month.

The specific power generation formula for Lake Diefenbaker (Coteau Rapids station) was estimated indirectly from a graph of reservoir elevation versus potential generating capacity. More accurate and direct estimates might be made available from Saskatchewan Power Corporation and should eventually be incorporated into the model if possible. The graph of potential generating capacity from Lake Diefenbaker, reproduced from Prairie Provinces Water Board (1982), is shown in Figure A1.

The basic procedure was to read-off the drop in power potential at various lake elevations for a unit release from Lake Diefenbaker. This data was then used to estimate an equation which expressed power generation for a unit flow as a function of elevation. This equation could then be transformed into a standard power generation formula as a function of head and flow.

Table A1 shows the basic data used to estimate the power-head relationship. The first column represents power in MWH and is equal to the loss in pontential generating capacity for a one meter drop in elevation of Lake Diefenbaker. The second column represents the average head in meters at which the corresponding power loss in column one was derived. The head is derived by subtracting the base elevation of 545.6 meters. Note that a one meter drop in elevation represents an approximate flow of 404,691 dam through the turbines.

Figure A1
POTENTIAL GENERATING CAPACITY
FOR LAKE DIEFENBAKER

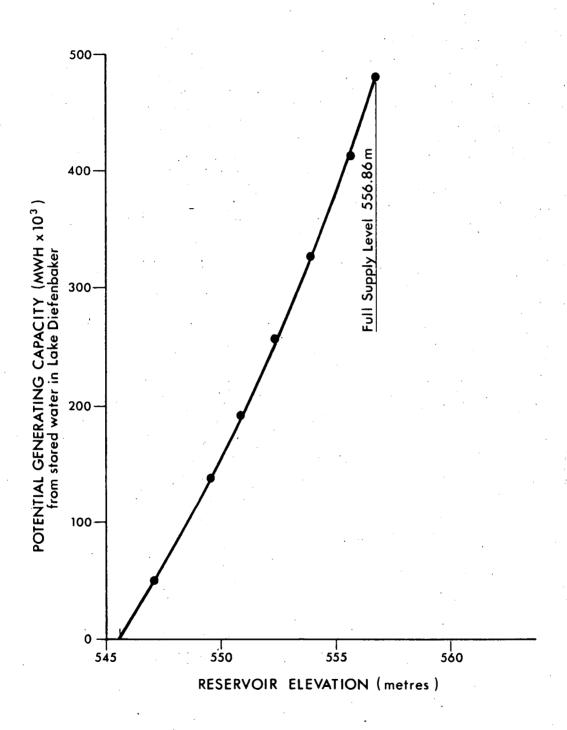


Table A1

Data Used to Estimate Power-Head Relationship

Power (MWH)	Head (Meters)
53 000	10.3
51 000	8.9
50 000	7.9
44 000	6.9
40 000	5.9
38 000	4.9
36 000	2.9
33 000	1.9
31 000	0.9
20 000	0.3

Based on the above data, the following regression equation was estimated.

Power (in MWH) =
$$24\ 864 + 2\ 996\ x\ Head$$
 (in meters) (A2)

Equation (A2) represents power generation for a flow of 404,691 dam³, which is equivalent to a one meter drop in elevation of Lake Diefenbaker. The equation can be transformed to represent power for a flow of one dam by dividing the right-hand-side by 404,691 as shown in equation (A3).

Power (in MWH) =
$$.0614 + .007403 \times \text{Head}$$
 (in meters) per dam³ flow (A3)

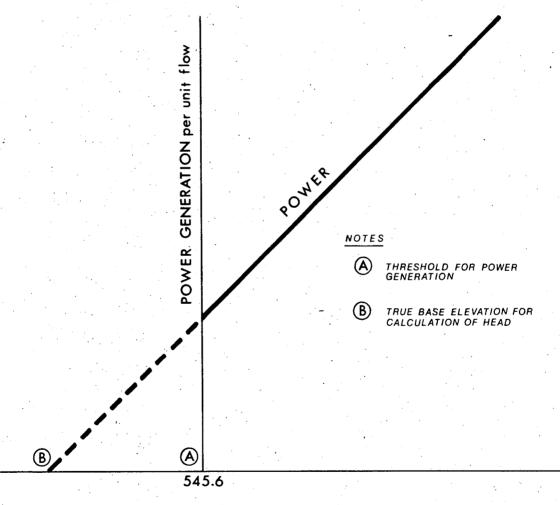
The next step is to derive a more general function which expresses power as a function of any given flow. This is done by simply multiplying equation (A3) by flow as shown in equation (A4).

Power (in MWH) =
$$(.0614 + .007403 \times \text{Head}) \times \text{Flow (in dam}^3)$$
 (A4)

Equation (A4) is slightly different from the form of the standard power generation formula in that it has a separate constant term which has a value of .0614. This constant term originally occured in the regression equation (A2). It suggests that a lower base elevation lower than 545.6 meters should have been used to calulate head. However, the elevation of 545.6 meters would still be considered as a threshhold below which no power could be generated. This relationship is shown in Figure A2.

The power function in Figure A1 can be expressed as a function of head and flow without a separate constant term if the base elevation is measured from point B on the horizontal axis. However, if the lake elevation is below 545.6 meters, power generation would not be possible. Thus the

Figure A2 POWER-HEAD RELATIONSHIP



LAKE ELEVATION in metres

separate constant term which appears in equation (A4) can be dropped if the extra distance from point B to 545.6 meters is added to the value of Head. This extra distance is equal to 8.29 meters (calulated by dividing the slope of equation (A2) by the separate constant term). If this distance is added to the head (in effect recalculating Head from a base elevation of 537.31 meters), then the constant term can be dropped from equation (A4) giving equation (A5) which has the standard power generation format.

Power (MWH) =
$$.007403 \times \text{Head (meters)} \times \text{Flow (dam}^3$$
) (A5)

Equation (A5) is the basic power generation formula used in the objective function described in chapter two. Some changes in units are made for scaling purposes, expressing Head in millimeters and Flow in thousands of ${\rm dam}^3$.

Now that the power generation formula has been derived, the next problem is to derive equations that define the Head variables in each of the twelve time periods. In general, operating head can be defined as in equation (A6).

537.31 meters = base elevation for calculating head

The problem with the above equation is that it is based on lake elevation expressed in meters. However, the only variables in the model related to lake elevation are the variables LLAK1 to LLAK123 which are measures of the volume of Lake Diefenbaker expressed in dam3. These variables can be converted to lake elevation in meters using Figure A3 which is a graph of elevation versus volume of Lake Diefenbaker. The capacity curve in figure A3 is non-linear, which causes some difficulty in transforming volume into elevation. However, in the normal operating range between 545.6 meters and 556.9 meters, the capacity curve can be approximated by a linear segment. Based on the slope of this segment the following equation will convert the lake volume into elevation in meters.

Elevation (meters) =
$$545.6 + .00000286 \times (Lake Volume - 5 424 000)$$
 (A7)

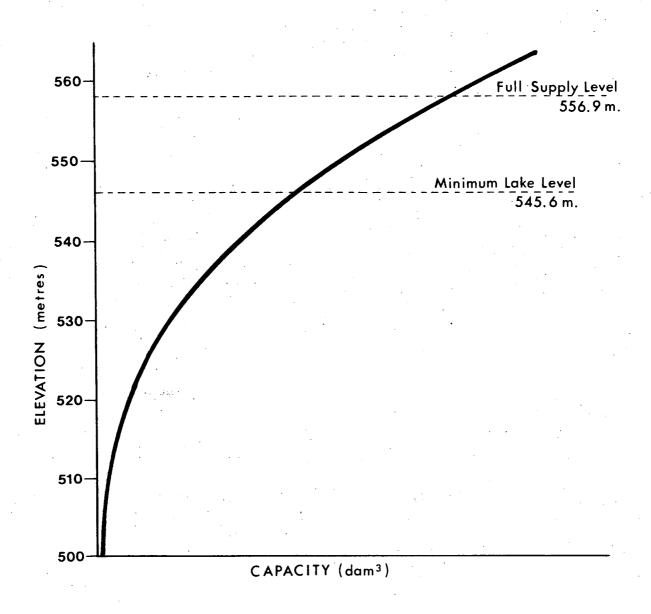
where:

545.6 = minimum lake level in meters
.00000286 = slope of the capacity curve in figure 1.
5 424 000 = volume equivalent in dam of 545.6 meters.

This equation can be simplified and restated in millimeters as shown in equation (A8).

Elevation
$$(mm) = 530\ 087 + .00286 \times Lake\ Volume\ (dam3)$$
 (A8)

Figure A3 CAPACITY AND ELEVATION OF LAKE DIEFENBAKER



Average Head during any of the time periods is expressed in equation (A9).

Average Head = .5 x (Lake Elevation at beginning of period (A9) (meters) + Lake Elevation at end of period) 537.31 where:

537.31 = base elevation for calculating operating head

By substituting equation (A8) for beginning and end of period elevations, we can restate average head as a function of lake volume rather than lake elevation as shown in equation (A10).

Average Head = $.00143 \times (lake \ volume \ at \ beginning \ of \ period)$ (A10) (mm) + lake volume at end of period) - 7 223

Equation (A10) is then used as the basis of the LHED1C to LHED12C constraints shown in chapter two which define average head in each period.

APPENDIX B

LISTING OF R.MOD.DAT

This appendix lists the file R.MOD.DAT which contains the MPSX standard code for the constraint equations and linear portion of the objective function. The first line of the file identifies the name of the run, while lines 2 to 100 form the ROWS section which gives the name and type of each row. Lines 701 to 2291 represent the COLUMNS section which gives the coefficients for each variable for whatever equation it occurs in. Lines 2292 to 2989 represent the RHS section which gives the right-hand side values of each constraint equation. Each line of the RHS section begins with the identifier TRY1. An identifier is required because it is possible to incorporate more than one set of right hand side values in the file and solve the model for each set.

Full details on the formatting and structure of this file are given in Chapter III, section C.2.

#list r.mod.dat

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1	NAMI	3
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4	Ë	VALUE AREL1C AREL1OC AREL11C
5	Ē	AREL10C
6 7	E	ARELIIC
8	Ë	AREL11C AREL12C AREL2C
9	Ē	AREL3C
10	E	AREL4C
12	Ē	AREL6C
13	Ē	AREL7C
14	E	AREL8C
16	E	DAGDOC
1 7	Ē	DAGD5C
18	E	DAGD6C
20	E	DAGD/C
ŽĬ	Ē	DAGD9C
22	Ë	DDODOC
23 24	E	DDOD10C
25	Ē	AREL12C AREL2C AREL3C AREL4C AREL5C AREL6C AREL6C AREL9C DAGD0C DAGD5C DAGD5C DAGD6C DAGD7C DAGD8C DAGD9C DD0D1C DD0D1C DD0D1C DD0D11C DD0D12C
26	E	DDOD12C
27 28	E	DDOD11C DDOD12C DDOD2C DDOD3C
29	Ē	DDOD10C DDOD11C DDOD12C DDOD2C DDOD3C DDOD4C
30	E	DDOD5C DDOD6C
37	E	DDODOC DDOD7C
33	Ē	DDÖD8Č
34	E	DDOD7C DDOD8C DDOD9C DIND0C DIND1C DIND1C DIND11C DIND12C DIND2C DIND3C DIND4C DIND5C DIND6C DIND6C DIND7C DIND7C DIND8C DIND8C DIND9C DIND9C DIND9C DIND9C DIND9C DIND9C DIND9C
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43 44	E E	DIND6C
45	Ē	DIND7C
46	E	DIND8C
47	E	DNAT1C
49	Ē	DNAT10C
50	E	DNAT11C DNAT12C
52	Ē	DNAT2C
53	Ē	DNAT3C
54	E	DNAT4C
56	Ē	DNAT6C
57	E	DNAT7C
28 59	E	DNATSC DNATSC
<u>6</u> 0	Ē	DREM1C
61 .	E	DREM10C
63	E E	DREM11C
64	Ĕ	DREM2C
65	E	DREM3C
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70	-	DDDM10
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75	E	DRET12C
76	E	DRET2C
לל ל	Ē	DRET3C
ŹŔ	ਬ	DRETAC
70	두	DRETTSC
60	늗	DEFTSC
90	먇	DREIDC
87	E 10	DEET/C
82	E	DREIBC
83	Ē	DRETYC
84	E	DSUPIC
85	Е	DSUPIOC
86	E	DSUP11C
87	Ε	DSUP12C
88	E	DSUP2C
89	Ē	DSUP3C
ğή	Ē	DSIIP4C
άĭ	Ē	DSIIP5C
áż	Ĕ	DSUPAC
02	臣	DOULDC
93	臣	DOUT/C
94	臣	DOULOC
85 .	Ē	DSUP9C
96	Ē	DTODIC
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98	E	
99	E	DTOD12C
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102	E E	DTODIC
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108	Ë	DWASIC
109	Ē	DWASIUC
110	Ē	DWASTIC
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112	Е	DWAS2C
113	E	DWAS3C
114	Ε	DWAS4C
115	E	DWAS5C
116	Е	DWAS6C
117	E	DWAS7C
118	Ē	DWAS8C
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ižĭ	Ē	KAGDSC
155	ដ	KAGD6C KAGD7C
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12%	귶	KAGDAC
125	5	KAGD8C KAGD9C
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132	E	KDOD3C
133	Ē	KDOD4C
134	E	KDOD5C
135	Ē	KDOD6C
136	E	KDOD7C
137	E	KD0D8C
138	· E	KDOD9C
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155	Ē	KNAT12C
156	Ē	KNAT2C
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158	Ë	KNA14C
128	Ē	KNATOC
160	Е	KNAT6C
161	Ε	KNAT7C
162	Ε	KNAT8C
163	Ε	KNAT9C
164	E	KREM1C
165	Ē	KREM10C
166	Ē	KREM11C
167	5	VDEM12C
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174	E	KREM8C
1 75	Ē	KREM9C
176	Ē	KRET1C
177	듄	VPFT10C
170	10	VDET11C
1/0	E	KREITIC
1/9	Ľ	KKEIIZU
180	E	KRETZC
181	E	KRET3C
182	E	KRET4C
183	Ε	KRET5C
184	Ε	KRET6C
185	E	KRET7C
186	Ε	KRET8C
า ั ลัวั	Ē	KRET9C
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180	귶	KSUP10C
100	Ë	VCIID11C
190	12 12	KOUD13C
191	Ë	KOUPIZO
192	Ē	KSUPZC
183	Ē	KSUP3C
194	E	KSUP4C
195	E	KSUP5C
196	Ε	KSUP6C
197	Е	KSUP7C
198	E	KSUP8C
199	Е	KSUP9C
200	E	KTOD1C
žčí	Ē	KTODIOC
202	Ē	KTOD11C
203	Ē	KTOD12C
204	Ē	KTOD2C
205	5	KTODEC
203	5	KIODAC
200	E	VTODAC
207	댠	KEODEC
208	Ë	KIODOC
209	E	KTOD/C
210	E	KTODEC
211	Ε	KTOD9C
212	E	KWAS1C
213	E	KWAS10C
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215	Ē	KWAS12C
216	Ē	KWAS2C
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RDOD7C
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RWAS10C
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