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**PREDICTION OF SEDIMENT SORTING USING MOBED:**

**USERS MANUAL UPDATE II**

by

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## ABSTRACT

A sediment sorting algorithm has been added to model MOBED to extend the model to predict the effects of armouring in degrading stream beds. Details of the algorithm and the changes that have to be made to the source code and input data arrangements are described in this update. Test data sets and sample model outputs are also included.

## RESUME

On a ajouté au modèle d'écoulement MOBED un algorithme de sélection des sédiments afin d'accroître sa capacité à prévoir les effets du recouvrement du fond par les sédiments sur la détérioration des lits. Cette mise à jour fournit des précisions sur l'algorithme et les modifications à apporter au code des sources et à l'arrangement des données d'entrée. Des groupes de données d'essai et des exemples de résultats figurent également dans ce document.

## MANAGEMENT PERSPECTIVE

River flow transports sediment both in suspension and by bottom movement. The interactions of sediment movement and the water flows are very complex especially when neither the sediment supply nor the water flow is steady.

The MOBED model mimics the real world very well and this update improves it.

Because river flow moves the smaller sediment first, there tends to be a residue of larger sediment left in place which armours the bed against further movement.

This update to the model gives a new routine which incorporates the sorting and armouring effect which should bring the model even closer to reality. All river engineers should include MOBED in their "lexicon" of tools.

T. Milne Dick  
Chief  
Hydraulics Division

## PERSPECTIVE DE GESTION

Les cours d'eau transportent des sédiments en suspension et des dépôts de fond. L'étude des interactions entre le déplacement des sédiments et les débits est très complexe, surtout si on considère que ni l'accumulation de sédiments ni le débit n'est un phénomène stable.

Le modèle MOBED reproduit adéquatement les conditions véritables de la nature, et cette mise à jour permet de l'améliorer davantage.

Etant donné que les cours d'eau entraînent d'abord les petites particules, il se produit une accumulation des plus gros sédiments dans le fond. Ces sédiments tendent à recouvrir le lit d'une couche qui empêche tout déplacement ultérieur.

Cette mise à jour du modèle crée un nouveau parcours qui permet de faire un choix dans la sélection et la protection des effets afin de rapprocher le modèle des conditions naturelles. MOBED devrait faire partie du «lexique» des outils de génie hydraulique.

Le chef,  
T. Milne Dick  
Division de l'hydraulique

## 1.0 INTRODUCTION

The model MOBED described in the original Users Manual (1) and the Users Manual Update I (2) assumes that the hydraulic sorting of bed sediment is negligible and considers the sediment size characteristics in terms of only two representative sizes, namely  $D_{50}$  and  $D_{65}$ . It also assumes that these two sizes remain constant throughout the river reach being modelled. Such an assumption is justifiable in applications in which the river flow is in equilibrium with sediment supply and all fractions of the sediment bed are transported by the flow. For reaches of river where the equilibrium condition is upset by man-made or natural causes, and where a degradation of the river is anticipated, the hydraulic sorting and consequently the armouring of the river bed may become an important factor which should be considered in predicting the ultimate response of a river to changes imposed on its regime. Therefore, it was decided to incorporate a sediment sorting algorithm into MOBED. With this algorithm, the model is now able to predict the size distribution of bed sediment as a function of time and distance along the river in addition to predicting the usual water and bed level changes and other hydraulic and sediment transport characteristics. In this update, a brief description of the sediment sorting algorithm is given together with a description of all the changes that were made to the source code and to the arrangement of input data.

## 2.0 SEDIMENT SORTING ALGORITHM

The initial size distribution of the bed sediment at a number of cross-sections within the study reach has to be specified to the model. The size distribution is expressed in terms of weight-percentage of sediment in different size ranges. Ten different size ranges were selected starting with a sediment size of 0.062 mm and ending with 64.00 mm. The selected size ranges are similar to the ones used in the model HEC-6 (3) and they are shown schematically in Fig. 1. The

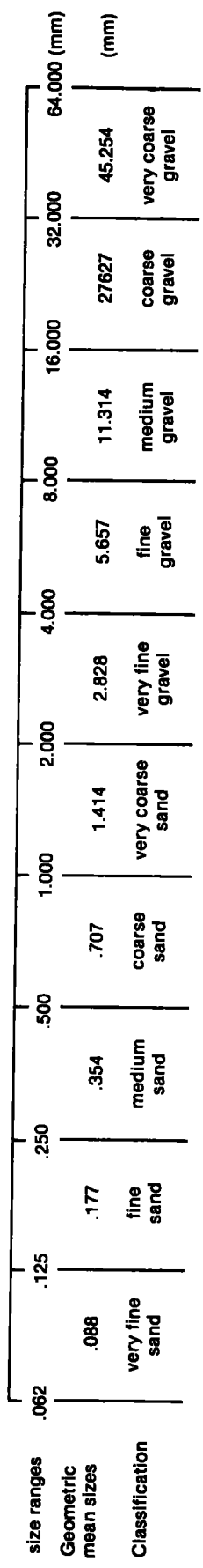


Fig. 1: Size ranges adopted for MOBED model



percentages of sediment in all of the ten size ranges have to be specified to the model. If a particular fraction is not present, then a zero value has to be specified.

Given the initial distribution of the bed sediment, the model then computes the distribution of the bed sediment after every time step as follows:

First, the shear velocity of the flow is compared with the critical shear velocity evaluated from the shields diagram for the finest size fraction of the sediment material.

If the flow-shear velocity is less than the critical shear velocity, i.e., if the flow is not able to transport even the finest fraction of the sediment, then the size distribution of the sediment bed is assumed to remain unaltered.

Secondly, the shear velocity of the flow is compared with the critical shear velocity for the coarsest size fraction. If the flow-shear velocity is greater than the critical shear velocity of the coarsest fraction, i.e., if the flow is able to transport all fractions of the bed sediment, then also the size distribution is assumed to remain unaltered. In other words, it is assumed that when all fractions of the sediment material are moving, the hydraulic sorting is considered to be negligible. Hydraulic sorting is assumed to be present only when the flow shear velocity lies in the range of critical shear velocities bounded by those of the finest and of the coarsest size fractions respectively. The size distribution of the bed sediment undergoing hydraulic sorting is computed by considering an active bed layer and the probability of a sand particle of certain size to stay when subjected to a particular bed shear stress. The derivation of the relationships for both degrading and aggrading beds is given below.

## 2.1 Degrading Bed

Let the thickness of the active bed layer be  $ABL$  and let  $\Delta Z$  be the amount of bed degradation during one time step (see Fig. 2).

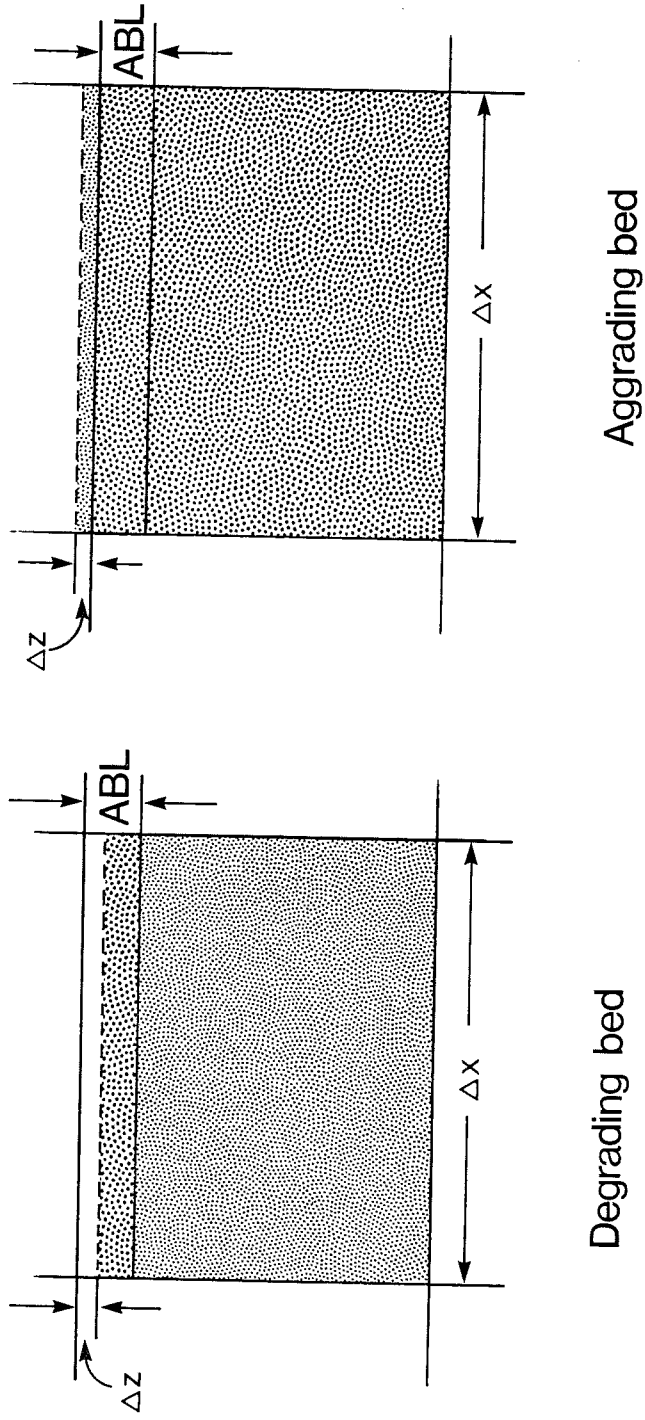


Fig. 2 Schematic representation sediment sorting in active bed layer in degrading and aggrading beds.

Weight of a fraction, say  $i$ , remaining in the active bed layer is composed of two parts. Part one is the weight of the fraction in the layer  $(ABL - \Delta Z)$ :

$$[(ABL - \Delta Z) \Delta X P p \rho_s g r_i] \quad (1)$$

where  $\Delta X$  is the length of a control element of the stream bed,  $P$  is the wetted perimeter,  $p$  denotes the volume of sediment in unit volume of bedlayer,  $\rho_s$  is the density of sediment,  $g$  is the acceleration due to gravity and  $r_i$  is the fraction of sediment in size range  $i$ .

Part two is the component left behind when the flow degrades the bed by the amount  $\Delta Z$ . This part is given by:

$$[\Delta Z \Delta X P p \rho_s g r_i] q_i \quad (2)$$

where  $q_i$  is the probability that a particle belonging to size range,  $i$ , will not be transported by the flow that existed during the time interval  $\Delta t$ .

Therefore, the weight of fraction,  $i$ , in the active bed layer is:

$$[(ABL - \Delta Z) \Delta X P p \rho_s g r_i] + [\Delta Z \Delta X P p \rho_s g r_i] q_i \quad (3)$$

The total weight of all fractions in the active bed layer is:

$$\sum_{i=1}^{10} [(ABL - \Delta Z) \Delta X P p \rho_s g r_i + \Delta Z \Delta X P p \rho_s g r_i q_i] \quad (4)$$

$$= (ABL - \Delta Z) \Delta X P p \rho_s g + \sum_{i=1}^{10} \Delta Z \Delta X P p \rho_s g r_i q_i \quad (5)$$

The percentage weight of fraction,  $i$ , is:

$$\frac{(ABL - \Delta Z) \Delta X Pp \rho_S g r_i + [\Delta Z \Delta X Pp \rho_S g r_i] q_i}{(ABL - \Delta Z) \Delta X Pp \rho_S g + \sum_{i=1}^{10} \Delta Z \Delta X Pp \rho_S g r_i q_i} \quad (6)$$

$$= \frac{(ABL - \Delta Z) r_i + \Delta Z r_i q_i}{(ABL - \Delta Z) + \sum_{i=1}^{10} \Delta Z \cdot r_i q_i} \times 100 \quad (7)$$

## 2.2 Aggrading Bed

As in the previous case, the weight of a fraction, say,  $i$ , consists of the weight in the deposited layer as well as the weight in the active bed layer. The weight in the deposited layer is:

$$[\Delta Z \Delta X Pp \rho_S g r_i] q_i \quad (8)$$

The weight in the active bed layer is:

$$[ABL \Delta X Pp \rho_S g r_i] \quad (9)$$

The total weight of fraction  $i$  is:

$$[\Delta Z \Delta X Pp \rho_S g r_i q_i + ABL \Delta X Pp \rho_S g r_i] \quad (10)$$

The total weight of all fractions in both the deposited and active bed layer is

$$\sum_{i=1}^{10} (\Delta Z \Delta X Pp \rho_S g r_i q_i + ABL \Delta X Pp \rho_S g r_i) \quad (11)$$

$$= ABL \Delta X Pp \rho_S g + \sum_{i=1}^{10} \Delta Z \Delta X Pp \rho_S g r_i q_i \quad (12)$$

The percentage weight of fraction  $i$ , therefore, is:

$$\frac{\Delta Z \Delta X Pp \rho_s g r_i q_i + ABL \Delta X Pp \rho_s g r_i}{ABL \Delta X Pp \rho_s g + \sum_{i=1}^{10} \Delta Z \Delta X Pp \rho_s g r_i q_i} \times 100 \quad (13)$$

$$= \frac{\Delta Z r_i q_i + ABL r_i}{ABL + \sum_{i=1}^{10} \Delta Z r_i q_i} \times 100 \quad (14)$$

To use Equations (7) and (14) and to predict the percentage weight distribution of the sorted sediment after one time step, the quantities  $q_i$  and ABL have to be known. In the present model, the quantity  $q_i$  is evaluated using the Gessler's (4) expression which is

$$q_i = \frac{1}{\sqrt{2\pi}} \int_{-\infty}^{x_{u_i}} \exp\left(-\frac{1}{2} x_0^2\right) dx_0 \quad (15)$$

where  $x_{u_i} = \left(\frac{\tau_{x_i}}{\bar{\tau}} - 1\right)/0.57 \quad (16)$

$\tau_{x_i}$  is the critical shear stress for initiation of motion of fraction,  $i$ , as determined from Shield's diagram and  $\bar{\tau}$  is the average bed shear stress. The thickness of active bed layer, ABL, is assumed to be related to  $D_{65}$

$$ABL = 2 D_{65} \quad (17)$$

The above assumption was tested using the laboratory data of Little and Mayer (5), and a satisfactory agreement between model prediction and the laboratory measurement was observed.

In a recent paper on the development of equilibrium armour layers, Shen and Lu (6) evaluated  $q_i$  using Equation (15) but with a different expression for  $x_{u_i}$ . Shen and Lu expressed  $x_{u_i}$  as:

$$x_{u_i} = \frac{\xi \frac{\tau_{c_i}}{\bar{\tau}} - 1}{(CV)_{\tau}} \quad (18)$$

where  $\xi$  is a hiding factor and  $(CV)_{\tau}$  is the coefficient of variation of bottom shear stress fluctuations. Note that in Gessler's expression  $(CV)_{\tau}$  is taken as a constant equal to 0.57. Shen and Lu treated the hiding factor as a function of the grain size, the apparent roughness of the bed surface and the thickness of laminar sublayer. They treated the coefficient of variation  $(CV)_{\tau}$  as a function of the standard deviation of the initial distribution of the bed sediment. They also used a slightly different Shield's diagram to estimate the critical shear stress for the initiation of sediment motion. They did all this to match their predicted armour layer distributions exactly with the measured distribution of Little and Mayer (5). However, it has been established by Kellerhals and Church (7) and Ettema (8) that the measured distributions of Little and Mayer do not reflect the true distribution of the armour coat because of the sampling technique used. They argued that the hot-wax method used by Little and Mayer biases the measured distribution towards the larger sizes. Therefore, the method of Shen and Lu which was formulated on the basis of a biased distribution may not be valid to predict the true armour coat distribution. In a recent paper, Odgaard (9) has proposed that the grain size distribution of the armoured bed follows the normal distribution with mean 1.0 and standard deviation 0.57 when the grain size is normalized as follows:

$$\frac{\theta}{S} \left( \frac{\rho_s - S}{\rho} \right) \frac{D}{R} \quad (19)$$

where  $\theta$  is the Shield's parameter,  $S$  is the slope of uniform flow,  $\rho$  is the density of the fluid,  $D$  is the diameter of the particle and  $R$  is the hydraulic radius. The method of Odgaard was derived for the fully developed armour layers.

### 2.3 Calibration of the Proposed Method

As pointed out earlier the thickness of the active bed layer ABL was determined by running the model for the data of Little and Mayer (5). The value of ABL is selected so that the final computed distribution is somewhat finer than the measured distributions. Three runs were selected (runs Nos. 2.1, 3.1 and 6.1). The model was run until the equilibrium armour layers were developed. The size distribution resulting from the model is compared with the measured distribution as shown in Figs. 3, 4 and 5. A deliberate attempt has been made to ensure that the predicted distributions do not match the measured distribution and the deviation is such that the predicted distribution is finer than the measured distribution. The predicted distributions are then analyzed according to the method of Odgaard (9) and the results are shown in Fig. 6. The open symbols represent the predicted values, while the solid symbols denote the measured values. The solid line is the one proposed by Odgaard. It can be seen from Fig. 6 that the predicted values agree with the curve closer near the finer end of the distribution.

In the light of the uncertainty in the measurement, it was felt that any further refinement to the prediction method is not warranted. Improved experimental techniques to measure the grain size distributions of armour layers are urgently needed to improve our predictive capability.

After computing the percentage of sediment in each size range using Eqns. 7 and 14, the model then computes  $D_{35}$  and  $D_{65}$  by

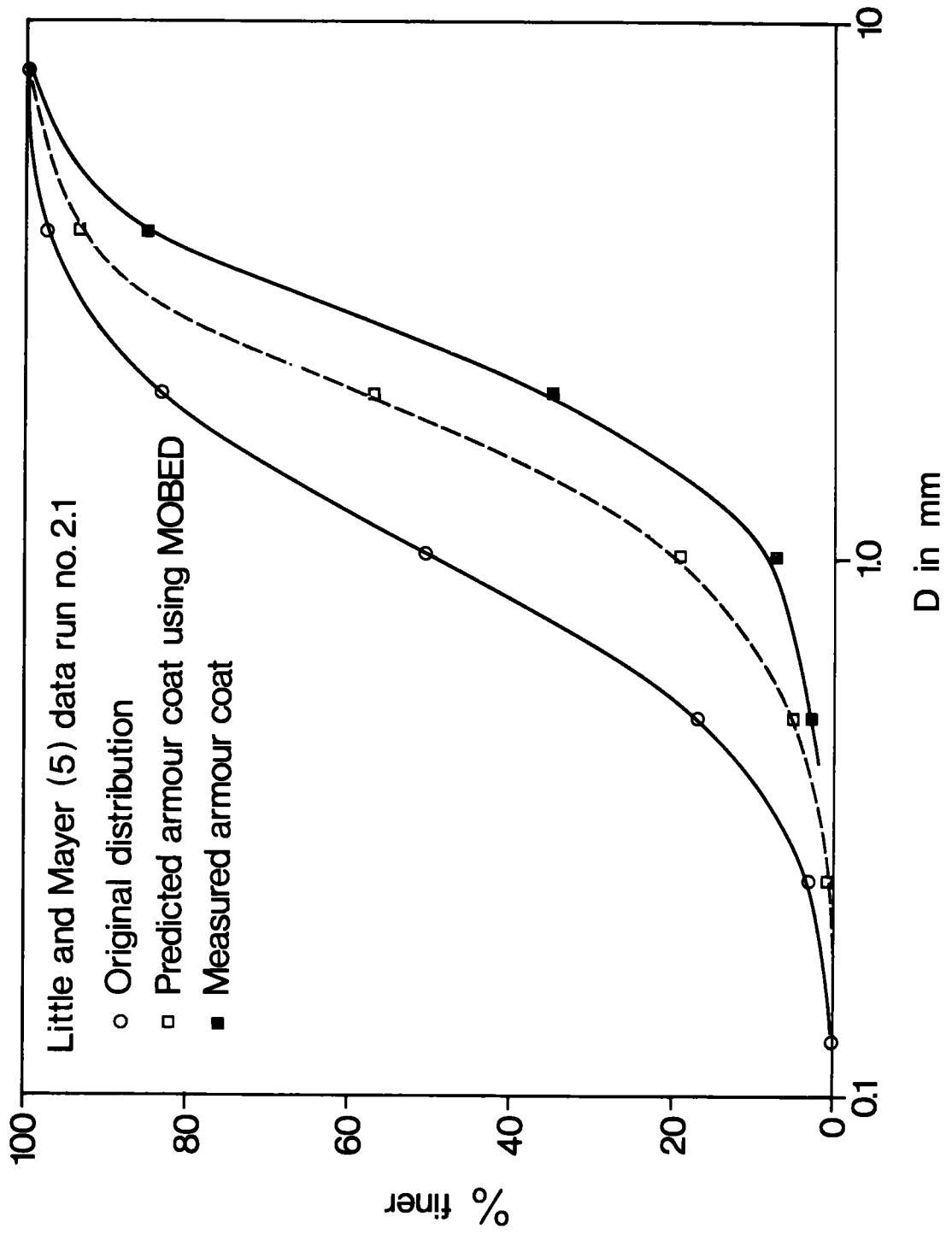


Fig. 3 Measured and Predicted distributions of armour coat



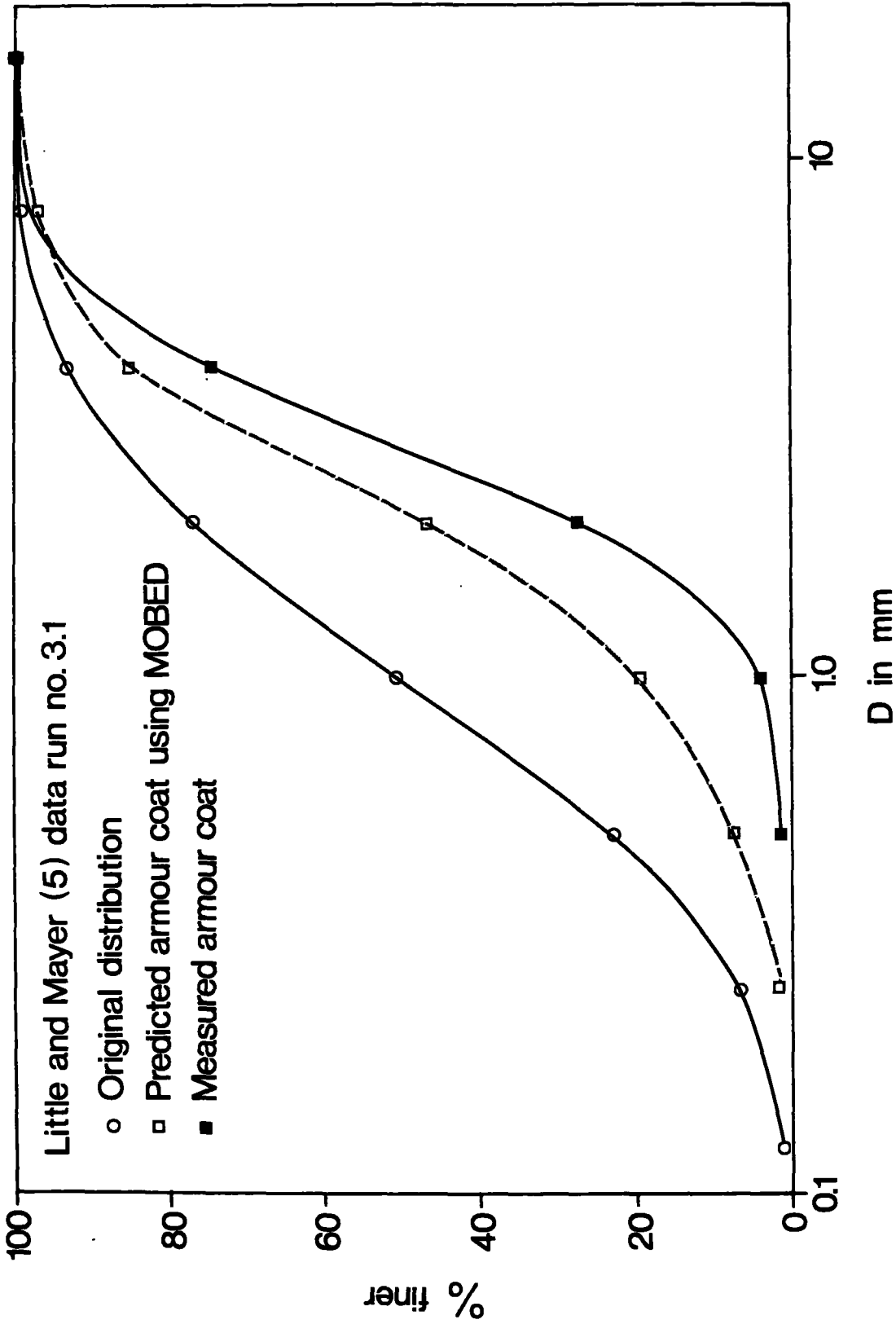


Fig. 4 Measured and Predicted distributions of armour coat

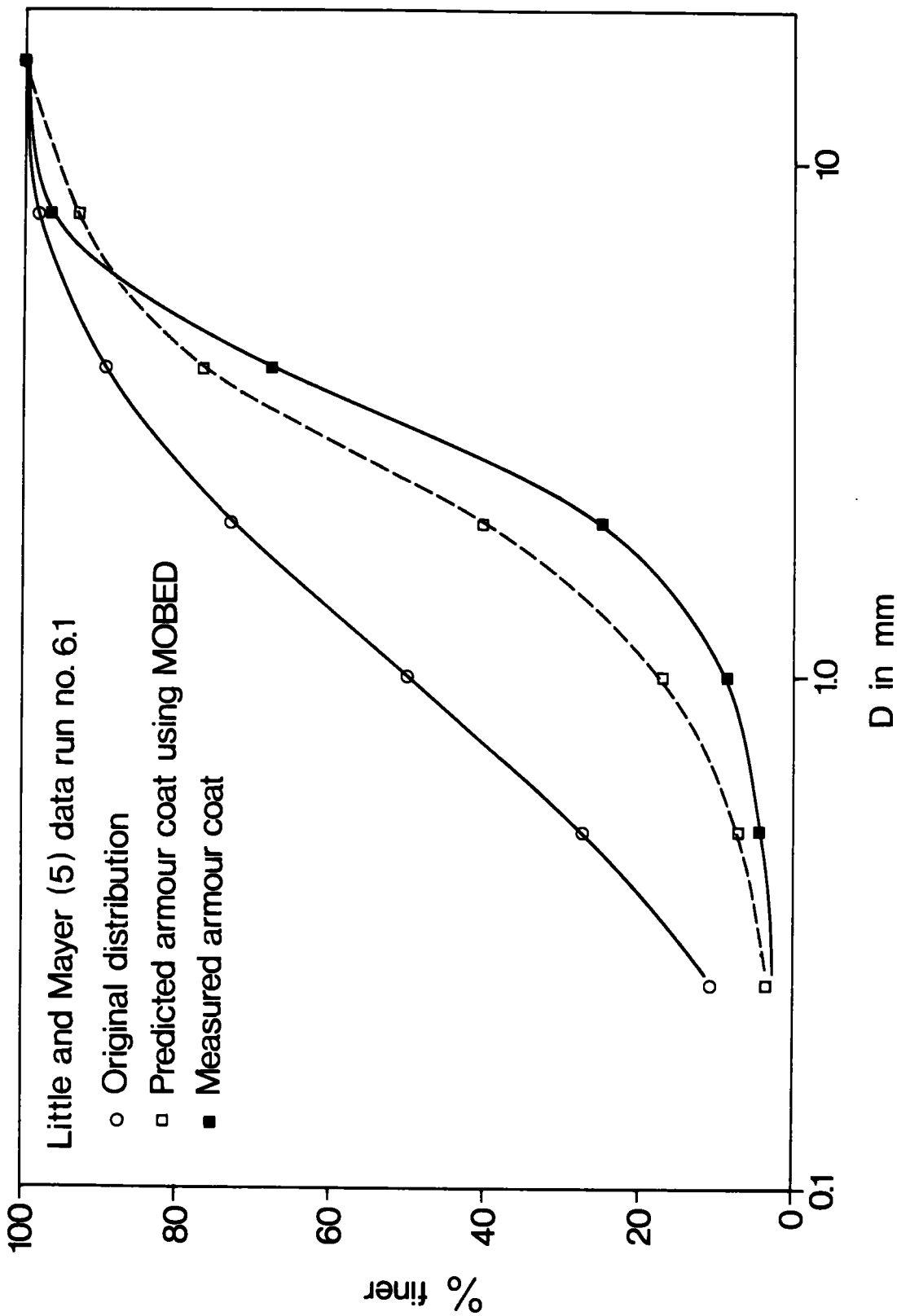


Fig. 5 Measured and Predicted distributions of armour coat

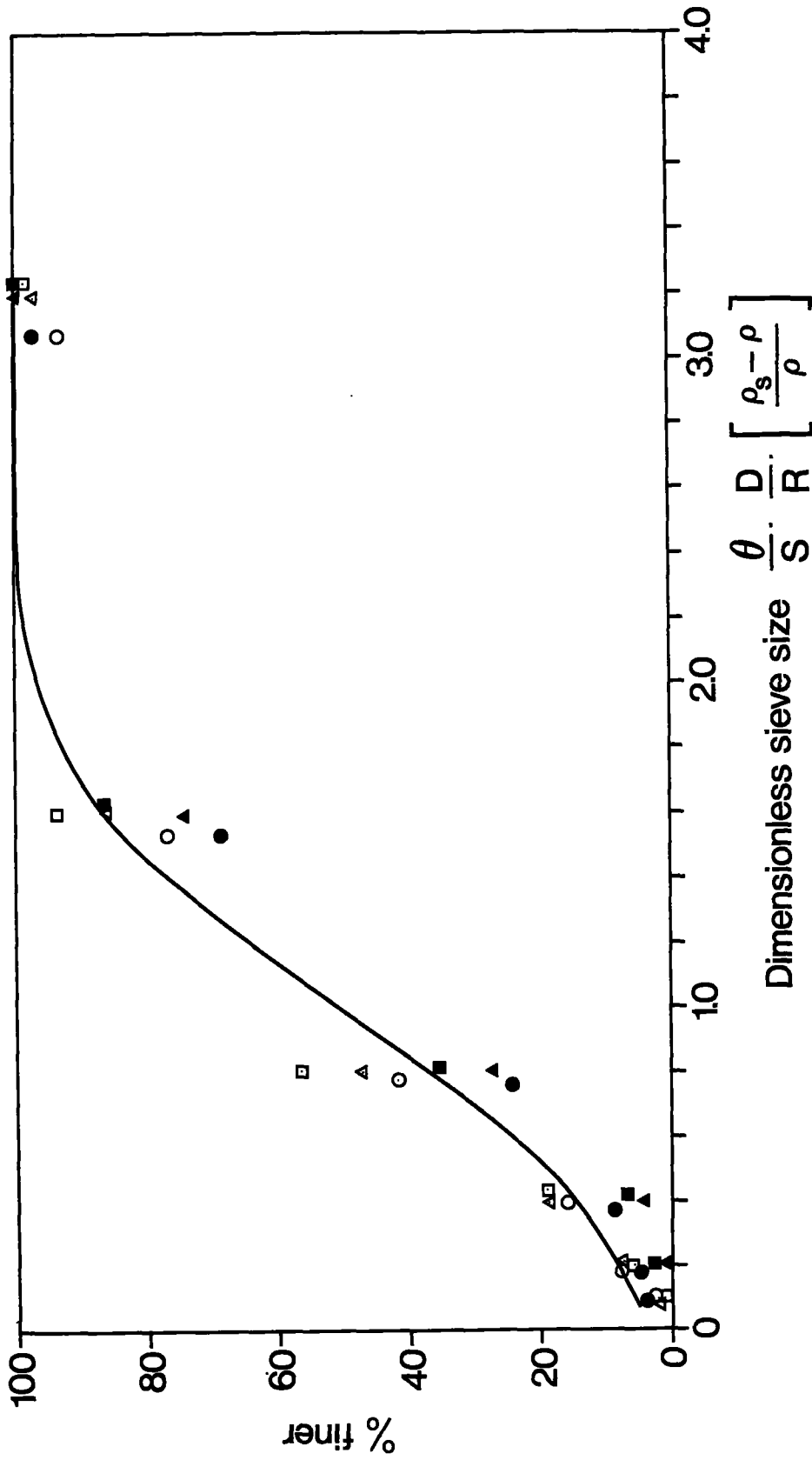


Fig. 6 Comparison with Odgaard's curves

constructing the cumulative distribution curve and linear interpolation as follows:

Let  $P_1, P_2, \dots, P_{10}$  be the weight percentages of the bed sediment. Then the cumulative distribution is computed as shown in the table below.

<u>Size in mm</u>	<u>Percentage Finer</u>
0.125	$P_1$
0.250	$P_1+P_2$
0.500	$P_1+P_2+P_3$
1.000	$P_1+P_2+P_3+P_4$
2.000	$P_1+P_2+P_3+P_4+P_5$
4.000	$P_1+P_2+P_3+P_4+P_5+P_6$
8.000	$P_1+P_2+P_3+P_4+P_5+P_6+P_7$
16.000	$P_1+P_2+P_3+P_4+P_5+P_6+P_7+P_8$
32.000	$P_1+P_2+P_3+P_4+P_5+P_6+P_7+P_8+P_9$
64.000	$P_1+P_2+P_3+P_4+P_5+P_6+P_7+P_8+P_9+P_{10}$

The grain sizes for which the percentage finer values are 35 and 65, i.e.,  $D_{35}$  and  $D_{65}$  are determined by interpolation. Referring to Fig. 7, the values of  $D_{35}$  and  $D_{65}$  are determined from the following two relations.

$$\frac{\log D'_u - \log D'_l}{\log D_{35} - \log D'_l} = \frac{P'_u - P'_l}{35 - P'_l} \quad (20)$$

$$\frac{\log D''_u - \log D''_l}{\log D_{65} - \log D''_l} = \frac{P''_u - P''_l}{65 - P''_l} \quad (21)$$

where  $D'_u$  and  $D'_l$  are the grain size values that encompass  $D_{35}$ .  $P'_u$  and  $P'_l$  are corresponding percentage finer values. Similarly,  $D''_u$  and  $D''_l$  are

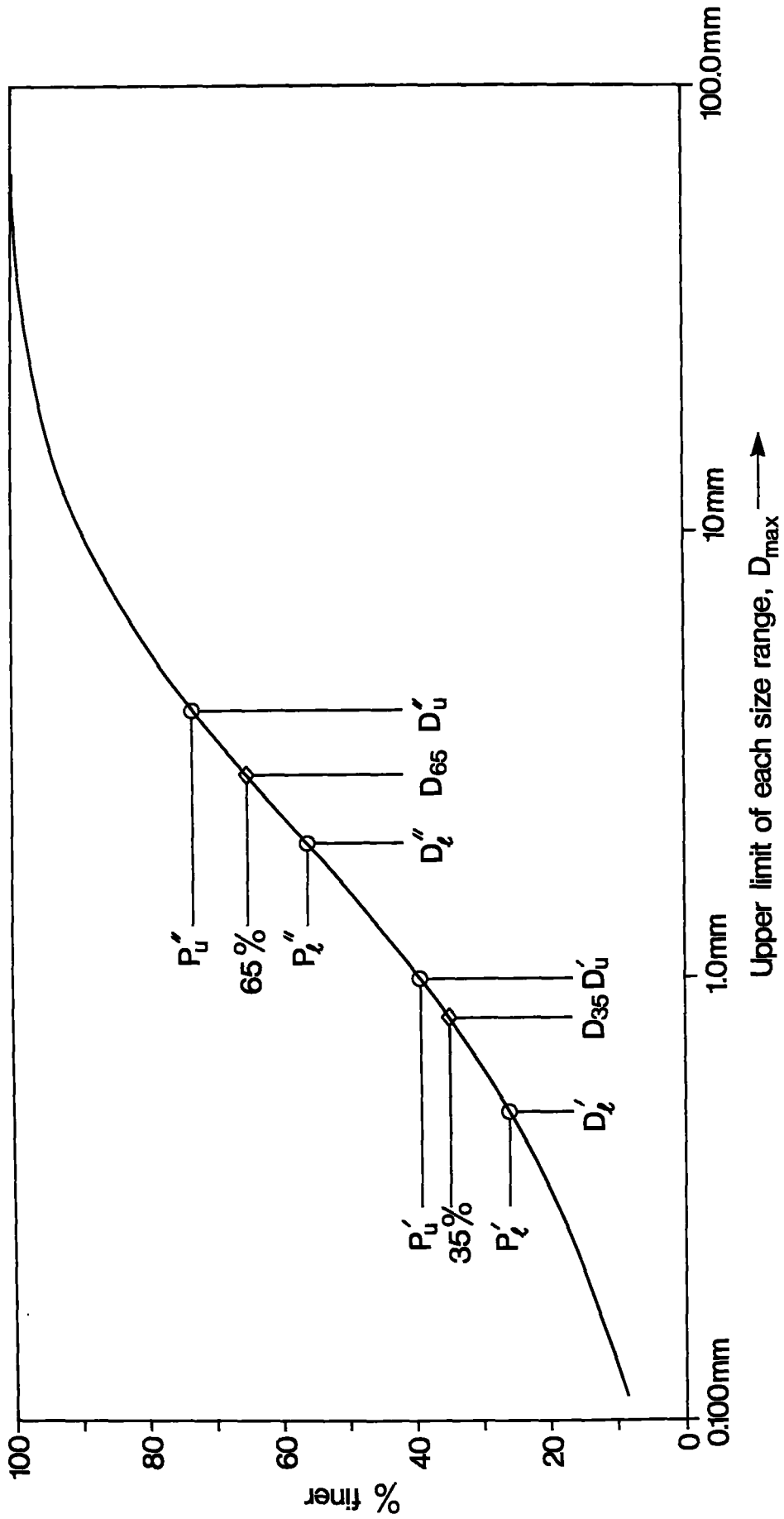


Fig. 7 Evaluation of  $D_{35} + D_{65}$

the grain sizes that encompass  $D_{65}$ , and  $P'_u$  and  $P'_l$  are the corresponding percentage finer values.

The values of  $D_{35}$  and  $D_{65}$  are then used to compute the sediment transport rate and the friction factor of the mobile boundary flow in the model. As the armour layer develops, the values of  $D_{35}$  and  $D_{65}$  increase which in turn affects both the sediment transport rate and the friction factor. For the same flow condition, the sediment transport rate will decrease as  $D_{35}$  increases and the skin friction will increase as  $D_{65}$  increases. Therefore, the effect of armouring is felt in the sediment transport rate and the friction factor via the values of  $D_{35}$  and  $D_{65}$  respectively.

#### 2.4 Changes to Source Code

Since the grain size distribution is treated as a function of distance along the river reach, the grain size parameters  $D_{35}$  and  $D_{65}$  have to become subscripted variables. Similarly since the friction parameters CONST, EM and EN depend on the grain size distribution, these parameters have to be also subscripted variables. These changes were introduced into the source code. A listing of the revised code is given in Appendix I.

#### 2.5 Changes to Data Files

The grain size distribution information is specified to the model along with the geometric information in TAPE1. The grain size distribution is specified at the same stations for which the geometry data are specified. The model then assigns the grain size distribution data to the grid station in the same way as it assigns the geometry data to the grid stations. The revised arrangement of tape 1 is shown schematically in Fig. 8. The format of the data of tape 1 is shown in Table 1.

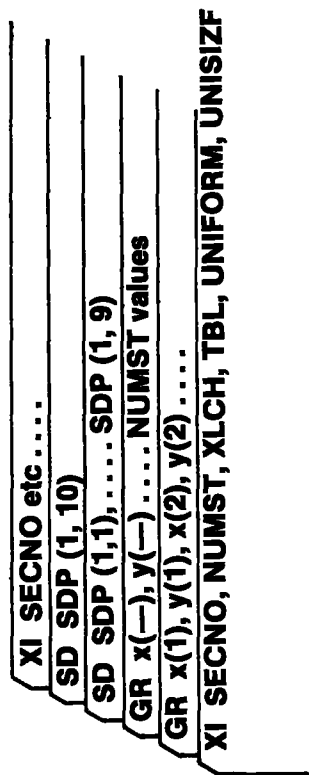


Fig. 8: GEOMETRIC DATA ARRANGEMENT (TAPE 1)

TABLE 1  
MOBED Geometric Data (TAPE1)

Group	Format	Columns		Variable Name
X1	F8.0	1-8	Card identification parameter	X1
	F8.3	9-16	An identification number for for the station	SECNO
	I4	17-20	Number of points to define the shape of the cross-section	NUMST
	4X	21-24	Gap	
	F8.3	25-32	Distance in metres or ft between adjacent stations. Zero for the first station.	XLCH
	F8.3	33-40	Thickness of bedlayer in metres or in ft.	TBL
	I3	41-43	Control parameter which specifies whether the sediment is uniform or not	UNIFORM
	F8.3	44-51	A factor which when multiplied by the upper limit of the size range for which a value of 1.0 is specified gives the size of uniform sediment	UNISIZF
GR	F8.0	1-8	Card identification parameter	GR
	9F8.3	9-16	Lateral distance in metres or ft. of a point on the perimeter of the cross-section	X(1)
		17-24	Elevation of the same point in metres or in ft.	Y(1)
		25-32	Lateral distance of the second point	X(2)



TABLE 1. MOBED Geometric Data (TAPE1) (continued)

Table 1.2

Group	Format	Columns	Variable Name
		33-40	Elevation of the 2nd point Y(2)
		41-48	Lateral distance of the 3rd point X(3)
		49-56	Elevation of the 3rd point Y(3)
		57-64	Lateral distance of the 4th point X(4)
		73-80	Lateral distance of the 5th point X(5)
GR	F8.0	1-8	Card identification parameter
	9F8.3	9-16	Elevation of the 5th point Y(5)
		17-24	Lateral distance of the 6th point X(6)
		25-32	Elevation of the 6th point Y(6)
		33-40	Lateral distance of 7th point X(7)
		41-48	Elevation of 7th point Y(7)
		49-56	Lateral distance of 8th point X(8)
		57-64	Elevation of 8th point Y(8)
		65-72	Lateral distance of 9th point X(9)
		73-80	Elevation of 9th point Y(9)
		(This group contains coordinates of NUMST points)	
SD	F8.0	1-8	Card identification parameter SD
	9F8.3	9-16	Fraction of sediment by weight in size range #1 SDP(1,1)
		17-24	Fraction of sediment by weight in size range #2 SDP(1,2)
		25-32	Fraction of sediment by weight in size range #3 SDP(1,3)
		33-40	Fraction of sediment by weight in size range #4 SDP(1,4)
		41-48	Fraction of sediment by weight in size range #5 SDP(1,5)

TABLE 1. MOBED Geometric Data (TAPE1) (continued)

Table 1.3

Group	Format	Columns	Variable Name	
		49-56	Fraction of sediment by weight in size range #6	SDP(1,6)
		57-64	Fraction of sediment by weight in size range #7	SDP(1,7)
		65-72	Fraction of sediment by weight in size range #8	SDP(1,8)
		73-80	Fraction of sediment by weight in size range #9	SDP(1,9)
		9-16	Fraction of sediment by weight in size range #10	SDP(1,10)

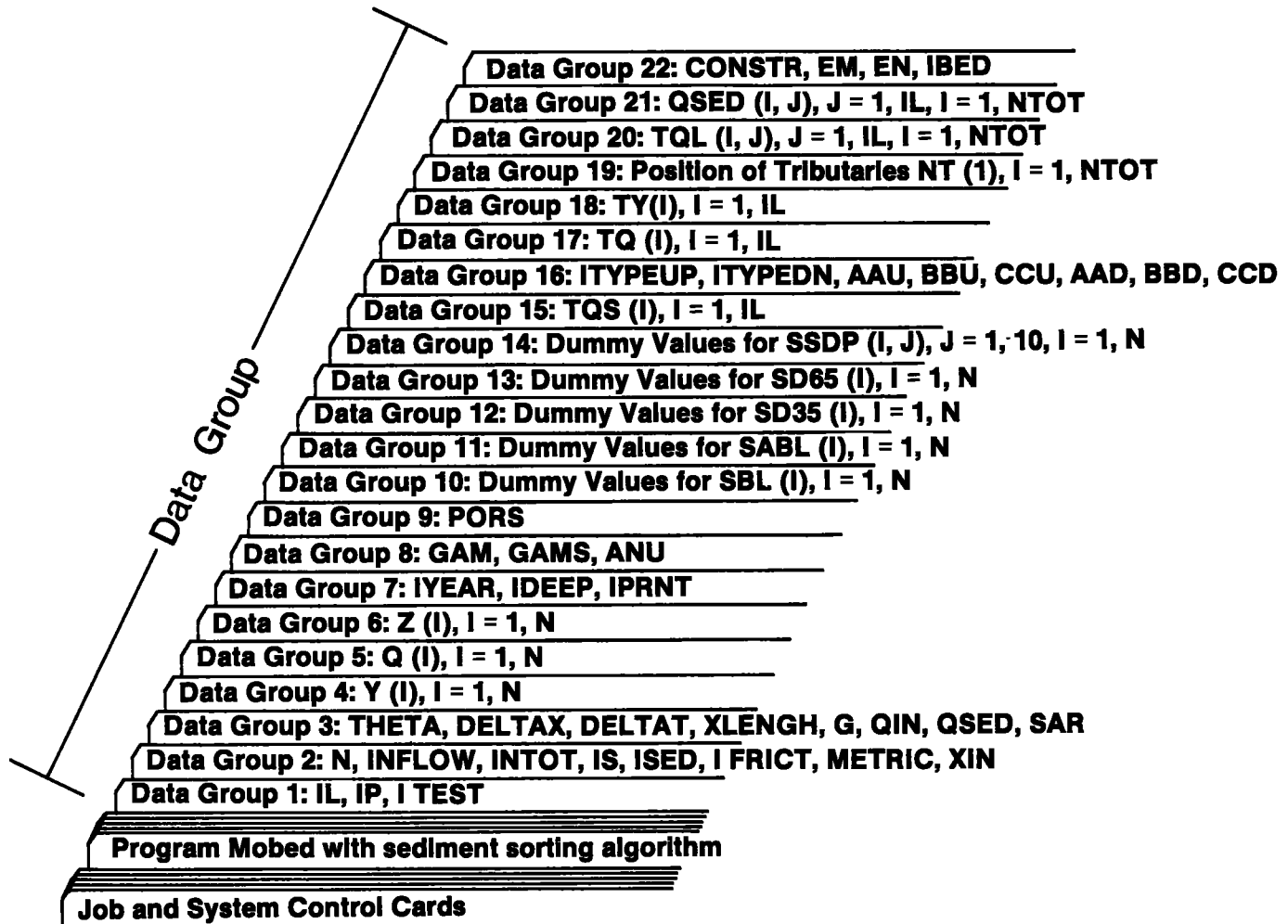
This completes the geometric data specification to Station #1. The data for other stations are continued in a similar fashion. Note that the data specification is from upstream station to downstream station.

The structure of the input data file, TAPE8, is also affected by inclusion of the sediment sorting algorithm. The arrangement of data in TAPE8 is shown schematically in Fig. 9. The format is shown in Table 2. Note that additional data groups are added to the original TAPE8 arrangement. The values of  $D_{35}$  and  $D_{65}$  specified under data group 8 of the previous TAPE8 are dropped and data groups 17 and 18 are added to provide space for storing  $(D_{35}(I), I = 1, N)$  and  $(D_{65}(I), I = 1, N)$  for the last time step in a batch simulation in which the total simulation period is covered by a series of shorter duration simulations. Initially, the data groups 17 and 18 contain zero values. Data groups 15 and 16 are added to provide space for storing the bed layer thickness, BL and the active bed layer thickness at the end of the simulation period. At the start of simulation, data groups 15 and 16 also contain zero values. Data group 19 is to provide space for storing the percentage of sediment in different size ranges for each grid station. Zero values are specified at the very start of the simulation period.

## 2.6 Miscellaneous Changes

A control parameter was introduced into the model so that the model can be run for uniform size sediment. The name of the control parameter is UNIFORM. When the bed sediment is uniform, a value of unity is specified to this parameter, otherwise, a value of zero is specified. When running the model for uniform size sediments an additional parameter called UNISIZF is introduced to calculate the actual size of the sediment from the size range. When specifying grain size distribution for a uniform sediment, a value of 1.0 will be specified to a size range and the actual size of the sediment will be computed as:

$$\text{Actual sediment size} = \text{UNISIZF} \times \text{upper limit of the size range} \quad (22)$$



**Fig. 9: INPUT DATA ARRANGEMENT FOR MOBED (TAPE 8)**

TABLE 2  
MOBED INPUT DATA (TAPE8)

Group	Format	Columns	Variable Name	
1	3I5	1-5	Number of time steps for which the model predictions have to be carried out	IL
		6-10	An integer parameter to set the time counters at the start of the model predictions	IP
		11-15	A control parameter to print out intermediate results of a number of computations. ITest=1 will cause the printout. ITEST=0 will bypass the printout.	ITEST
2	6I5, 2F10.4	1-5	Number of grid points along the river reach (maximum=61).	N
		6-10	Control parameter for tributary INFLOW=1, indicates presence of tributary inflows in this reach	INFLOW
		11-15	Total number of tributaries present in the model reach	NTOT
		16-20	An integer between 1 and N, to indicate the position of a storage basin in terms of the grid positions. IS=0 indicates absence of storage basins	IS
		21-25	A control parameter to indicate the nature of river reach. If ISED=0, the reach is rigid boundary. IF ISED=1, the reach is loose boundary	ISED

TABLE 2. MOBED INPUT Data (TAPE8) (continued)

Group	Format	Columns	Variable Name	
		26-30	A control parameter to choose the friction factor relation. IF IFRIC=1, then the relations of kishi and kuroki are used.	IFRICT
		31-40	A conversion factor to convert the geometric data specified in TAPE1 from British units to metric units. If data are specified in British units then METRIC=.3048. If data are in metric units then METRIC=1.0	METRIC
		41-50	Location of the first cross-section for which the geometric data are specified. The location is specified in terms of distance in metres measured from the upstream boundary	XIN
3	6F9.3	1-9	A weighting coefficient. A value of 0.67 is recommended	THETA
		10-18	Distance in metres between grid points	DELTAX
		19-27	Time increments, in seconds	DELTAT
		28-36	Total distance of reach (m)	XLENGH
		37-45	Acceleration due to gravity ( $m/s^2$ )	G
		46-54	Water surface area of storage basin, in $m^2$	SAR

TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns		Variable Name
4	8F10.3 (Upstream to Downstream)	1-10	Initial flow depth (m)	Y(1)
		11-20	Initial flow depth (m)	Y(2)
		21-30	Initial flow depth (m)	Y(3)
		31-40	Initial flow depth (m)	Y(4)
		41-50	Initial flow depth (m)	Y(5)
		51-60	Initial flow depth (m)	Y(6)
		61-70	Initial flow depth (m)	Y(7)
		71-80	Initial flow depth (m)	Y(8)
N such values have to be specified				
5	8F10.4 (Upstream to Downstream)	1-10	Initial flow rate (m <sup>3</sup> /s)	Q(1)
		11-20	Initial flow rate (m <sup>3</sup> /s)	Q(2)
		21-30	Initial flow rate (m <sup>3</sup> /s)	Q(3)
		31-40	Initial flow rate (m <sup>3</sup> /s)	Q(4)
		41-50	Initial flow rate (m <sup>3</sup> /s)	Q(1)
		51-60	Initial flow rate (m <sup>3</sup> /s)	Q(5)
		61-70	Initial flow rate (m <sup>3</sup> /s)	Q(7)
		71-80	Initial flow rate (m <sup>3</sup> /s)	Q(8)
N such values have to be specified				
6	8F10.6	1-10	The bottom elevation (m)	Z(1)
		11-20	The bottom elevation (m)	Z(2)
		21-30	The bottom elevation (m)	Z(3)
		31-40	The bottom elevation (m)	Z(4)
		41-50	The bottom elevation (m)	Z(5)
		51-60	The bottom elevation (m)	Z(6)
		61-70	The bottom elevation (m)	Z(7)
		71-80	The bottom elevation (m)	Z(8)
N such values have to be specified				

TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns		Variable Name
7	I4,I5, I2	1-4	An identifier for the geometric data	IYEAR
		5-9	Maximum flow depth expected in ft.	IDEEP
		10-11	Control parameter to print out all the geometric properties of the river reach. If IPRNT=1, the calculated geometric properties such as the flow cross-sectional area wetted perimeter and top width for all the cross-sections are printed out	
8	3E10.4	1-10	Specific weight of water ( $\text{kg/m}^3$ )	GAM
		11-20	Submerged specific weight of sediment ( $1650 \text{ kg/m}^3$ )	GAMS
		21-30	Kinematic viscosity ( $10^{-6} \text{ m}^2/\text{s}$ )	ANU
9	F5.2	1-5	Volume of sediment in unit volume of bed layer	PORS
10	8F10.6 (upstream to downstream)	1-10	Dummy Bedlayer thickness (m)	SBL(1)
		11-20	Dummy Bedlayer thickness (m)	SBL(2)
		21-30	Dummy Bedlayer thickness (m)	SBL(3)
		31-40	Dummy Bedlayer thickness (m)	SBL(4)
		41-50	Dummy Bedlayer thickness (m)	SBL(5)
		51-60	Dummy Bedlayer thickness (m)	SBL(6)
		61-70	Dummy Bedlayer thickness (m)	SBL(7)
		71-80	Dummy Bedlayer thickness (m)	SBL(8)

N such values have to be specified. The model will update the SBL values with actual bed layer thickness when it rewrites TAPE8 at the end of the first simulation in a batch mode. When preparing initial TAPE8, please specify zero values for SBL.



TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns		Variable Name
11	8F10.6	1-10	Dummy value for active bed layer thickness (m)	SABL(1)
		(upstream 11-20	Dummy value for active bed layer thickness (m)	SABL(2)
		to 21-30	Dummy value for active bed layer thickness (m)	SABL(3)
		downstream)31-40	Dummy value for active bed layer thickness (m)	SABL(4)
		41-50	Dummy value for active bed layer thickness (m)	SABL(5)
		51-60	Dummy value for active bed layer thickness (m)	SABL(6)
		61-70	Dummy value for active bed layer thickness	SABL(7)
		71-80	Dummy value for active bed layer thickness (m)	SABL(8)
<p>N such values have to be specified. The model will update the SABL values with actual bed layer thickness values when it rewrites TAPE8 at the end of the first simulation. Therefore, when preparing initial TAPE8, please specify zero values for SABL.</p>				
12	8F10.6	1-10	Dummy value for $D_{35}$ (m)	SD35(1)
		11-20	Dummy value for $D_{35}$ (m)	SD35(2)
		21-30	Dummy value for $D_{35}$ (m)	SD35(3)
		31-40	Dummy value for $D_{35}$ (m)	SD35(4)
		41-50	Dummy value for $D_{35}$ (m)	SD35(5)
		51-60	Dummy value for $D_{35}$ (m)	SD35(6)

TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns	Variable Name
		61-70	Dummy value for $D_{35}$ (m) SD35(7)
		71-80	Dummy value for $D_{35}$ (m) SD35(8)
		<p>N such values have to be specified. The model will update the SD35 values with the actual <math>D_{35}</math> values when it rewrites TAPE8 at the end of the first batch simulation. Therefore, when preparing initial TAPE8, please specify zero values for SD35.</p>	
13	8F10.6	1-10	Dummy value for $D_{65}$ (m) SD65(1)
		11-20	Dummy value for $D_{65}$ (m) SD65(2)
		21-30	Dummy value for $D_{65}$ (m) SD65(3)
		31-40	Dummy value for $D_{65}$ (m) SD65(4)
		41-50	Dummy value for $D_{65}$ (m) SD65(5)
		51-60	Dummy value for $D_{65}$ (m) SD65(6)
		61-70	Dummy value for $D_{65}$ (m) SD65(7)
		71-80	Dummy value for $D_{65}$ (m) SD65(8)
		<p>N such values need to be specified. The model will update the SD65 values with the actual values of <math>D_{65}</math> when it rewrites TAPE8 at the end of the first batch simulation. Therefore, when preparing initial TAPE8, please specify zero values for SD65.</p>	
14	8F10.6	1-10	Dummy values for percent by weight of particles in size range 1 SSDP(I,1)
		11-20	Dummy values for percent by weight of particles in size range 2 SSDP(I,2)
		21-30	Dummy values for percent by weight of particles in size range 3 SSDP(I,3)
		31-40	Dummy values for percent by weight of particles in size range 4 SSDP(I,4)
		41-50	Dummy values for percent by weight of particles in size range 5 SSDP(I,5)

TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns	Variable Name	
		51-60	Dummy values for percent by weight of particles in size range 2	SSDP(I,6)
		61-70	Dummy values for percent by weight of particles in size range 2	SSDP(I,7)
		71-80	Dummy values for percent by weight of particles in size range 2	SSDP(I,8)
<p>N such values need to be specified. The model will update the SSDP values with the actual grain size distribution values when it rewrites TAPE8 at the end of the first batch simulation. Therefore, when preparing initial TAPE8, please specify zero values for these parameters.</p>				
15	8F10.6	1-10	Sediment transport rate at upstream boundary in kg/s.	TQS(1)
		11-20	Sediment transport rate at upstream boundary in kg/s.	TQS(2)
		21-30	Sediment transport rate at upstream boundary in kg/s.	TQS(3)
		31-40	Sediment transport rate at upstream boundary in kg/s.	TQS(4)
		41-50	Sediment transport rate at upstream boundary in kg/s.	TQS(5)
		51-60	Sediment transport rate at upstream boundary in kg/s.	TQS(6)
		61-70	Sediment transport rate at upstream boundary in kg/s.	TQS(7)
		71-80	Sediment transport rate at upstream boundary in kg/s.	TQS(8)

(This group contains 11 such values)

TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns		Variable Name
16	2I, 6F5.0	1-3	Type of upstream boundary condition	ITYPEUP
		4-6	Type of downstream boundary condition	ITYPEDN
		11-15	Parameter of upstream stage-discharge relationship	AAU
		16-20	Parameter of upstream stage-discharge relationship	BBU
		21-25	Parameter of upstream stage-discharge relationship	CCU
		26-30	Parameter of downstream stage-discharge relationship	AAD
		31-35	Parameter of downstream stage-discharge relationship	BBD
		36-40	Parameter of downstream stage-discharge relationship	CCD
17	8F10.4	1-10	Flow rate as a function of time at the upstream or downstream boundary	TQ(1)
		11-20	Flow rate as a function of time at the upstream or downstream boundary	TQ(2)
		21-30	Flow rate as a function of time at the upstream or downstream boundary	TQ(3)
		31-40	Flow rate as a function of time at the upstream or downstream boundary	TQ(4)
		41-50	Flow rate as a function of time at the upstream or downstream boundary	TQ(5)

TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns	Variable Name	
		51-60	Flow rate as a function of time at the upstream or downstream boundary	TQ(6)
		61-70	Flow rate as a function of time at the upstream or downstream boundary	TQ(7)
		71-80	Flow rate as a function of time at the upstream or downstream boundary	TQ(8)
		(this group contains IL such values)		
18	8F10.4	1-10	Flow depth as a function of time at the downstream or upstream boundary	TY(1)
		11-20	Flow depth as a function of time at the downstream or upstream boundary	TY(2)
		21-30	Flow depth as a function of time at the downstream or upstream boundary	TY(3)
		31-40	Flow depth as a function of time at the downstream or upstream boundary	TY(4)
		41-50	Flow depth as a function of time at the downstream or upstream boundary	TY(5)
		51-60	Flow depth as a function of time at the downstream or upstream boundary	TY(6)

TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns	Variable Name	
		61-70	Flow depth as a function of time at the downstream or upstream boundary	TY(7)
		71-80	Flow depth as a function of time at the downstream or upstream boundary	TY(8)
		(This group contains IL such values)		
19	6I5	1-5	Position of 1st Tributary	NT(1)
		6-10	Position of 2nd Tributary	NT(2)
		11-15	Position of 3rd Tributary	NT(3)
		16-20	Position of 4th Tributary	NT(4)
		21-25	Position of 5th Tributary	NT(5)
		26-30	Position of 6th Tributary	NT(6)
20	8F10.4	1-10	Tributary inflow rate of the first tributary	TQL(1,1)
		11-20	Tributary inflow rate of the first tributary	TQL(1,2)
		21-30	Tributary inflow rate of the first tributary	TQL(1,3)
		31-40	Tributary inflow rate of the first tributary	TQL(1,4)
		41-50	Tributary inflow rate of the first tributary	TQL(1,5)
		51-60	Tributary inflow rate of the first tributary	TQL(1,6)
		61-70	Tributary inflow rate of the first tributary	TQL(1,7)

TABLE 2. MOBED Input Data (TAPE8) (continued)

Group	Format	Columns	Variable Name
		71-80	Tributary inflow rate of the first tributary
			TQL(1,8)
		This group contains IL such values for each tributary specified.	
21	8F10.4	1-10	Lateral sediment inflow rate at first tributary
			QSED(1,1)
		11-20	Lateral sediment inflow rate at first tributary
			QSED(1,2)
		21-30	Lateral sediment inflow rate at first tributary
			QSED(1,3)
		31-40	Lateral sediment inflow rate at first tributary
			QSED(1,4)
		41-50	Lateral sediment inflow rate at first tributary
			QSED(1,5)
		51-60	Lateral sediment inflow rate at first tributary
			QSED(1,6)
		61-70	Lateral sediment inflow rate at first tributary
			QSED(1,7)
		71-80	Lateral sediment inflow rate at first tributary
			QSED(1,8)
		This group contains IL such values for each tributary specified.	
22	F20.5,	1-10	Friction parameter
			CONSTR
	F210.3,	11-20	Friction parameter
			EM
	I5	21-30	Friction parameter
			EN
		31-35	Parameter defining the nature of bed form
			IBED

The parameters, UNIFORM, UNISIZE are specified to the model through TAPE1. (See Fig. 8 and Table 1.)

In this version of MOBED, the ratio between the suspended load and the total sediment load is computed and is printed out as an additional model output. As pointed out earlier, the sediment transport rate calculations are carried out using  $D_{35}$  value for the sediment and not the individual fraction of sediment in each size range. For calculating the ratio between the suspended load and total load, a critical shear velocity for initiation of suspension proposed by van Rijn (10) was adopted. According to this method, the critical shear velocity  $U_{*cr}$  for sediment suspension is given by:

$$\frac{U_{*cr}}{w} = \frac{4}{D_*} \quad \text{for } 1 < D_* \leq 10 \quad (23)$$

$$\frac{U_{*cr}}{w} = 0.4 \quad \text{for } D_* > 10$$

where  $w$  is the fall velocity of sediment.  $D_*$  is a dimensionless grain size given by

$$D_* = D_{35} \left[ \left( \frac{\rho_s}{\rho} - 1 \right) \frac{g}{\nu^2} \right] \quad (24)$$

In the above expression,  $\nu$  stands for the kinematic viscosity of fluid.

The fall velocity of particles  $w$  is evaluated from the following relation

$$w = C [D_*^3]^n \sqrt{\left( \frac{\rho_s - \rho}{\rho} \right) g D_{35}} \quad (25)$$

where  $C$  and  $n$  are given as follows:



$$\left. \begin{array}{l} c = .056 \\ n = 0.50 \end{array} \right\} \text{ if } 0 < D_*^3 \leq 100$$

$$\left. \begin{array}{l} c = .1585 \\ n = 0.20 \end{array} \right\} \text{ if } 100 \leq D_*^3 \leq 5 \times 10^5$$

$$\left. \begin{array}{l} c = 1.825 \\ n = 0.00 \end{array} \right\} \text{ if } D_*^3 > 5 \times 10^5$$

The method of Einstein (11) was employed to compute the ratio between the suspended sediment load and the bed load transport. According to this method the ratio is given as:

$$A = \frac{q_{ss}}{q_{sb}} = \frac{1}{\kappa} \frac{1}{23.2} \frac{h}{D_{65}} \left( \frac{\eta_a}{1 - \eta_a} \right)^Z \left[ \ln \left( \frac{A_s}{2.72} \frac{h}{2.5 D_{65}} \right) \int_{\eta_a}^1 \left( \frac{1 - \eta}{\eta} \right) d\eta + \int_{\eta_a}^1 \left( \frac{1 - \eta}{\eta} \right)^Z \ln \eta d\eta \right] \quad (26)$$

where  $q_{ss}$  is the suspended sediment transport and  $q_{sb}$  is the bed load transport rate.  $\eta_a$ ,  $Z$ ,  $A_s$  are defined as follows:

$$\eta_a = \frac{2 D_{65}}{h} \quad (27)$$

$$Z = \frac{1}{\kappa} \frac{w}{U_*} \quad (28)$$

$$A_s = e^{\kappa B_s} \quad (29)$$

where  $B_s = (2.50 x + 5.50)e^{-0.217x^2} + 8.5 (1.0 - e^{-0.217x^2})$  (30)

in which  $x = \ln (2.5 U_* D_{65}/v)$  (31)

Knowing the ratio between  $q_{ss}$  and  $q_{sb}$ , the ratio between  $q_{ss}$  and  $q_{st}$  can be computed as

$$\frac{q_{ss}}{q_{st}} = \left\{ 1 - \left( \frac{1}{1 + A} \right) \right\} \quad (32)$$

( $q_{st}$  is the total sediment transport rate.)

A new subroutine called TRIBUT is introduced in this version of MOBED. This subroutine allows the model to treat up to six tributaries in the study reach. The tributary inflow hydrographs are read into the model through the input data file TAPE8 (see Table 2 and Fig. 9). The lateral sediment inputs from the tributaries are also considered. They are also read into the model through TAPE8.

### 3.0 TEST DATA FILES AND MODEL OUTPUTS

Data files, tape 8 and tape 1 prepared for one of the runs of Little and Mayor are shown in Appendices II and III respectively as examples of these files and could serve as a set of test data for model implementation. The model output that was generated using the test data is shown in Appendix IV. The model output consists of percentages of sediment by weight in different size ranges, the values of  $D_{35}$ ,  $D_{65}$ , the total bed depth, active bed layer thickness, the type of bed forms present at each node, the friction parameters at each node and the ratio between the suspended sediment load and the total load at each grid node, in addition to the usual output of flow parameters.

#### **4.0 SUMMARY**

A sediment sorting algorithm incorporated into MOBED is described in this report. Some of the additional changes that were introduced to the model are also described. A set of sample test data and the model output generated using the test data are presented.

#### **5.0 ACKNOWLEDGMENTS**

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## REFERENCES

1. Krishnappan, B.G. "Users Manual: Unsteady, non-uniform, mobile boundary flow model - MOBED", Hydraulics Division, NWRI, Burlington, Ontario, Feb. 1981.
2. Krishnappan, B.G. "MOBED Users Manual update I", Hydraulics Division, NWRI, Burlington, Ontario, Aug. 1983.
3. HEC-6 Scour and Deposition in Rivers and Reservoirs: Users Manual, U.S. Army Corps of Engineers, the Hydrologic Engineering Center, California, March 1977.
4. Gessler, J. "Beginning and ceasing of sediment motion", Proceedings of the Institute of River Mechanics, Colorado State University, Fort Collins, Colorado, 15-26 June 1970.
5. Little, W.C. and Mayer, P.G. "The Role of Sediment Gradation on Channel Armouring", School of Engineering, Georgia Institute of Technology, Atlanta, Georgia, May 1972.
6. Shen, H.W. and Lu, J.Y., "Development and Prediction of Bed Armouring, Journal of Hydraulic Engineering, ASCE, Vol. 109, No. 4, April 1983.
7. Kellerhals, R., and Church, M. Discussion of "stability of channel beds by armouring", Journal of the Hydraulics Division, ASCE, Vol. 103, No. HY7, 1977, pp. 826-827.
8. Ettema, R., "Sampling armour-layer sediments", Journal of Hydraulic Engineering, Vol. 110, No. 7, July 1984, pp. 992-996.
9. Odgaard, A.J. "Grain-size distribution of river bed armour layers", Journal of Hydraulic Engineering, Vol. 110, No. 10, Oct. 1984.

**APPENDIX I**

**Listing of MOBED with Armouring Routine**











C

CCCCCCCC

CCCCCCCC

CCCCCCCC

```

      READ (60,520) (TQ(I),I=1,IL)
      GO TO 90
70  READ (60,520) (TQ(I),I=1,IL)
80  READ (60,520) (TY(I),I=1,IL)
90  IF (INFLOW.NE.0) READ(60,511) (NT(I),I=1,NTOT)
      IF (INFLOW.NE.0) READ(60,520) ((TQL(I,J),J=1,IL),I=1,NTOT)
      IF (INFLOW.NE.0) READ(60,520) ((CSED(I,J),J=1,IL),I=1,NTOT)
      IF ((ISED.NE.1).OR.(IFRICT.NE.1)) READ(60,580) CONSTR,(EM(I),
      EN(I),IBED(I),I=1,N)

```

\*\*\*\*\*

CHAPTER BBBB \*\*\*\* PRINT OUT ALL THE PARAMETERS \*\*\*\*

\*\*\*\*\*

```

WRITE (61,590)
IF (ITEST.EQ.1) WRITE (61,600)
WRITE (61,610) N
WRITE (61,620) INFLOW
WRITE (61,630) NTOT
WRITE (61,650) ISE
WRITE (61,660) ISED
WRITE (61,670) THETA
WRITE (61,680) DELTAX
WRITE (61,690) DELTAT
WRITE (61,700) XLNNGH
WRITE (61,710) G
WRITE (61,740) SAR
WRITE (61,770) ANJ
WRITE (61,780) ITYPEUP
WRITE (61,790) ITYPEDN
WRITE (61,800)
WRITE (61,810)
IF (TQ(1).EQ.999999.) GO TO 94
WRITE (61,815)
WRITE (61,810) (TQ(I),I=1,IL)
WRITE (61,810)
94  IF (TY(1).EQ.999999.) GO TO 95
WRITE (61,816)
WRITE (61,810) (TY(I),I=1,IL)

```

\*\*\*\*\*

CHAPTER CCCC \*\*\*\* START OF MAIN LOOP \*\*\*\*

\*\*\*\*\*

```

95  IAGAIN = 0
      II = 1
      IF (ISED.NE.1) GO TO 110
      DO 100 I=1,N
        Y(I) = Y(I)
100  CONTINUE
110  IF (II.GE.IL) STOP
      IF (ITEST.NE.1) GO TO 115
      IF (IAGAIN.EQ.0) WRITE (61,515)
      IF (IAGAIN.EQ.1) WRITE (61,516)
      WRITE (61,810)
      WRITE(61,510) (Y(I),I=1,N)
115  REWIND 4

```

COMPUTATIONS OF GEOMETRIC PROPERTIES OF THE STREAM USING  
SUBROUTINE -- GEOMET.

PROGRAM MOBED

73/171 OPT=1

FTN 4.8+538

86/12/1

```
CCCCCCCCC
C
A          IS FLOW CROSS SECTIONAL AREA (IN M**2).
P          IS THE WETTED PERIMETER (IN METERS).
R          IS HYDRAULIC RADIUS (IN METERS).
B          IS TOP WIDTH (IN METERS).
Q          IS FLOW RATE (IN M**3/SEC).
AYX       IS THE RATE OF CHANGE OF  $\frac{A}{B}$  W.R.T. X - DIRECTION
          WHEN DEPTH IS HELD CONSTANT.
AVY       IS THE AVERAGE DEPTH IN THE REACH.
FR        IS THE FROUDE NUMBER.

CALL GEOMET (A,P,R,B,DERB,DERP,Q,AYX,AVY,FR,IDEEP,RNEH,
S          SABL,SD35,SD65,SSDP,UNIFORM,SBL,UNISIZF)

          COMPUTATION OF FRICTIONAL PARAMETERS FOR THE FLOW USING
          SUBROUTINE -- FRICT.

CONST, EM, AND EN ARE FRICTION PARAMETERS.

ASF       IS THE AVERAGE CHANGE SLOPE OF THE REACH.
C         IS THE FRICTION COEFFICIENT.
IBED     IS A PARAMETRIC ARRAY REPRESENTING THE BED FORM
          CHARACTERISTICS.

CALL FRICT (Y,Z,Q,A,R,CONST,EM,EN,ASF,BSLOPE,IBED,IFRICT,ISED,
S          SASFL)

C
  IF (ISED,NE.1) GO TO 230
  IF (IFRICT,NE.1) GO TO 120
  GO TO 130
120 CONTINUE
  DO 125 I=1,N
  125 CONST(I) = CONST2
130 IF (IAGAIN,EO.1) GO TO 250
  IF (II.LE.2) GO TO 150
  DO 140 I=1,N
    P1(I) = P2(I)
    BT1(I) = BT2(I)
    AR1(I) = AP2(I)
    CAV1(I) = CAV2(I)
140 QST1(I) = QST2(I)
  GO TO 170
150 IF (II.GE.2) GO TO 170

          COMPUTATION OF THE SEDIMENT TRANSPORT RATE AND THE AVERAGE
          SEDIMENTS CONCENTRATIONS USING SUBROUTINE -- SEDI.

CAV       IS THE AVERAGE VOLUPETRIC SEDIMENT CONCENTRATION.
QST       IS THE TOTAL SEDIMENT TRANSPORT RATE (IN M**3/SEC).

CALL SEDI (Q,A,AVY,R,FR,ASF,QST,CAV,C,TA)
DO 160 I=1,N
  P1(I) = P(I)
  BT1(I) = B(I)
  AR1(I) = A(I)
  CAV1(I) = CAV(I)
160 QST1(I) = QST(I)
  GO TO 250
```

PROGRAM NOBED

73/171 CPT=1

FTN 4.8+538

86/12/.

```

170 CALL SEDI (Q,A,AVY,R,FR,ASF,QST,CAV,C,TA)
DO 180 I=1,N
  P2(I) = P(I)
  BT2(I) = B(I)
  AR2(I) = A(I)
  CAV2(I) = CAV(I)
180 QST2(I) = QST(I)
*** *****
CHAPTER DDDD **** LATERAL SEDIMENT INFLOW ****
*** *****
IF(INFLOW.EQ.1) GJ TO 194
DO 193 I=1,N
  QS1(I)=0.00
  QS2(I)=0.00
193 CONTINUE
  GO TO 195
194 DO 191 KK=1,NTOT
  LL=NT(KK)
  DO 190 I=1,N
  IF(I.EQ.NT(KK)) JD TO 192
  QS1(I)=0.00
  QS2(I)=0.00
  GO TO 190
192 QS1(I)=QSED(LL,II)
  QS2(I)=QSED(LL,II+1)
190 CONTINUE
191 CONTINUE
195 CONTINUE

*****
CHAPTER EEEE **** FLOW DEPTH AND BED ELEVATION CORRECTION ****
*****

N1 = N-1
DO 200 I=2,N1
  DELZ(I) = (DELTAT/(4.0*PORS))*(QS1(I)+QS2(I)+0.5*QS1(I-1)+0.5*
  QS2(I-1)+0.5*QS1(I+1)+0.5*QS2(I+1))-(DELTAT/(4.0*PORS*DELTAX))*
  (QST1(I+1)-QST1(I-1)+QST2(I+1)-QST2(I-1))-(1.0/(4.0*PORS))*(2.0
  *AR2(I)*CAV2(I)-2.0*AR1(I)*CAV1(I)+AR2(I-1)*CAV2(I-1)-AR1(I-1)*
  CAV1(I-1)+AR2(I+1)*CAV2(I+1)-AR1(I+1)*CAV1(I+1))
200 CONTINUE
  DELZ(1) = (DELTAT/(4.0*PORS))*(QS1(1)+QS1(2)+QS2(1)+QS2(2))-
  (DELTAT/(2.0*PORS*DELTAX))*(QST1(2)-QST1(1)+QST2(2)-QST2(1))-(1.0/
  (2.0*PORS))*(AR2(1)*CAV2(1)-AR1(1)*CAV1(1)+AR2(2)*CAV2(2)-AR1(2)*
  CAV1(2))
  DELZ(N) = (DELTAT/(4.0*PORS))*(QS1(N)+QS1(N1)+QS2(N)+QS2(N1))-
  (DELTAT/(2.0*PORS*DELTAX))*(QST1(N)-QST1(N1)+QST2(N)-QST2(N1))-(1.0/
  (2.0*PORS))*(AR2(N)*CAV2(N)-AR1(N)*CAV1(N)+AR2(N1)*CAV2(N1)-AR1
  (N1)*CAV1(N1))
  DO 210 I=1,N
  DELZ(I) = 2.0*DELZ(I)/(BT2(I)+BT1(I))
  CALL NEWBED(I,DELZ(I),ASF,QA,R,CONST,EM,EN,UNIFORM)
  CALL BED(I,UNIFORM,UNISIZF)
210 CONTINUE
  IF (ITEST.NE.1) GJ TO 215
  WRITE (61,825)
  WRITE (61,826)
  WRITE (61,830) (QST1(I),QST2(I),CAV1(I),CAV2(I),I=1,N)
  WRITE (61,835)
  WRITE (61,836)
  WRITE (61,840) (DELZ(I),I=1,N)
215 DO 220 I=1,N

```

PROGRAM MOBED

73/171 OPT=1

FTN 4.8+538

86/12/1

```

ZZ(I) = Z(I)+DELZ(I)
Y(I) = Z(I)+Y2(I)-ZZ(I)
C
C
STOP 001 CHECK FOR -VE FLW DEPTHS
C
IF (Y(I),GE,0.0) GO TO 220
WRITE (61,E50)
WRITE (61,E60)
STOP 001
C
C
220 Z(I) = ZZ(I)
IAGAIN = 1
GO TO 110
230 CONTINUE
DO 235 I = 1,N
235 CONST(I) = CONSTR
DO 240 I = 1,N
240 C(I) = (Q(I)/A(I))/SQRT(G*R(I)*ASF)
GO TO 270
C
C
250 CONTINUE
CALL SEED (Q,A,AVY,R,FR,ASF,QST,CAV,C,TA)
DO 250 I=1,N
Y2(I) = Y(I)
P2(I) = P(I)
BT2(I) = B(I)
AP2(I) = A(I)
CAV2(I) = CAV(I)
QST2(I) = QST(I)
260 CONTINUE
C
C
*****
CHAPTER FFFF **** LATERAL INFLW ****
*****
C
C
270 IF (INFLOW.EQ.1) GO TO 280
DO 275 I=1,N
QL1(I)=0.00
QL2(I)=0.00
275 CONTINUE
GO TO 300
280 CALL TRIBUT (II,TQL,NT,NTCT,QL1,QL2)
300 CONTINUE
C
C
*****
CHAPTER GGGG **** DOUBLE SWEEP COEFFICIENTS ****
*****
C
C
310 H = N-1
C
C
DOUBLE SWEEP COEFFICIENTS
DO 320 I=1,H
A1(I) = (B(I)+B(I+1))/(2.*DELTA T)-(2.*THETA/DELTA X)*((Q(I+1)-Q
(I))/(B(I+1)+B(I))*DERB(I+1)+THETA*(QL1(I+1)+QL1(I))/(B(I+1)+
B(I))*DERB(I))
B1(I) = 2.*THETA/DELTA X
C1(I) = (B(I)+B(I+1))/(2.*DELTA T)-(2.*THETA/DELTA X)*((Q(I+1)-Q
(I))/(B(I+1)+B(I))*DERB(I)+THETA*(QL1(I)+QL1(I+1))/(B(I+1)+B
(I))*DERB(I))
D1(I) = -C1(I)
O1(I) = -B1(I)
E1(I) = (2./DELTA X)*(C(I+1)-Q(I))-(QL1(I+1)+QL1(I))-THETA*QL2

```

```

320 $ (I+1)+QL2(I))*1.0
      E1(I) = -E1(I)
      DO 330 I=1,H

```

```

      A2(I) = -(THETA/DELTA X)*Q(I+1)*B(I+1)*(Q(I+1)-Q(I))/(A(I+1)*A(I
      +1))+(G*THETA/(2.0*DELTA X))*((Y(I+1)-Y(I))*B(I+1)+A(I+1)+A(I))+
      (THETA/(2.0*DELTA X))*((2.0*B(I+1)*E(I+1)*Q(I+1)*Q(I+1)/A(I+1))*
      *3)-(DERB(I+1)*Q(I+1)*Q(I+1)/A(I+1)**2))*(Y(I+1)-Y(I))-(B(I+1)*
      Q(I+1)*Q(I+1)/A(I+1)**2)-(B(I)*Q(I)*Q(I)/A(I)**2)-(G*THETA/(2.
      0*DELTA X))*B(I+1)*(Z(I)-Z(I+1))+(G*THETA/2.0)*CONST(I)*(R(I+1
      )/D65(I))*EM(I)*FR(I+1)*EN(I)*B(I+1)*(EM(I)-3.0*EN(I))-(A(I+
      1)*DERP(I+1)*(EM(I)-EN(I))/P(I+1))+E(I+1))+(THETA/2.0)*(2.0*Q(I
      +1)*Q(I+1)*B(I+1)*AYX(I+1)/A(I+1)**3)

```

```

      B2(I) = 1.0/(2.0*DELTAT)+(THETA/DELTA X)*(Q(I+1)/A(I+1)+Q(I)/A(I
      +1)+Q(I+1)-Q(I))/A(I+1))+THETA/(2.0*DELTA X))*2.0*Q(I+1)*B(I+1)*
      (Y(I)-Y(I+1))/(A(I+1)*A(I+1))+((G*THETA/2.0)*CONST(I)*(R(I+1)/D
      65(I))*EM(I)*FR(I+1)*EN(I)*2.0*EN(I)*A(I+1)/Q(I+1))-((THETA/2
      .0)*2.0*Q(I+1)*AYX(I+1)/A(I+1)*A(I+1))

```

```

      C2(I) = -(THETA/DELTA X)*Q(I)*B(I)*(C(I+1)-Q(I))/(A(I)*A(I))+G*
      THETA/(2.0*DELTA X))*B(I)*(Y(I+1)-Y(I))-A(I+1)-A(I))+THETA/(2.
      0*DELTA X))*((Y(I+1)-Y(I))*2.0*B(I)*B(I)*Q(I)*Q(I)/A(I)*A(I)*A
      (I))-Q(I)*Q(I)*DERB(I)/(A(I)*A(I))+B(I+1)*Q(I+1)*Q(I+1)/A(I+1
      )*A(I+1)+B(I)*Q(I)*Q(I)/A(I)*A(I))-G*THETA/(2.0*DELTA X))*B
      (I)*(Z(I)-Z(I+1))+(G*THETA/2.0)*CONST(I)*(R(I)/D65(I))*EM(I)*
      FR(I)*EN(I)*3(I))*EM(I)-3.0*EN(I))-A(I)*DERP(I)*(EM(I)-EN(I)
      )/P(I)+B(I))+THETA/2.0)*2.0*Q(I)*Q(I)*B(I)*AYX(I)/A(I)*A(I)*A
      (I)
      C2(I) = -C2(I)

```

```

      D2(I) = (1.0/(2.0*DELTAT)+(THETA/DELTA X))*((Q(I+1)-Q(I))/A(I)-Q
      (I)/A(I)-Q(I+1)/A(I+1))+THETA/(2.0*DELTA X))*2.0*Q(I)*B(I)*(Y(I
      )-Y(I+1))/(A(I)*A(I))+G*THETA/2.0)*CONST(I)*(R(I)/D65(I))*EM
      (I)*FR(I)*EN(I)*2.0*EN(I)*A(I)/Q(I))-THETA/2.0)*2.0*Q(I)*AYX(I
      )/A(I)*A(I)
      D2(I) = -D2(I)

```

```

      E2(I) = (1.0/DELTA X)*(Q(I+1)/A(I+1)+Q(I)/A(I))*(Q(I+1)-Q(I))+G
      /2.0*DELTA X))*A(I+1)+A(I))*Y(I+1)-Y(I))-1.0/(2.0*DELTA X))*
      (B(I+1)*Q(I+1)*Q(I+1)/A(I+1)*A(I+1))+B(I)*Q(I)*Q(I)/A(I)*A(I)
      ))*(Y(I+1)-Y(I))-G/(2.0*DELTA X))*A(I+1)+A(I))*Z(I)-Z(I+1))+
      (G/2.0)*CONST(I)*(R(I+1)/D65(I))*EM(I)*FR(I+1)*EN(I)*A(I+1)+
      (G/2.0)*CONST(I)*(R(I)/D65(I))*EM(I)*FR(I)*EN(I)*A(I)-0.50*Q
      (I+1)*Q(I+1)*AYX(I+1)/A(I+1)*A(I+1))-0.50*Q(I)*Q(I)*AYX(I)/A
      (I)*A(I)
      E2(I) = -E2(I)

```

```

330 CONTINUE
      IF (ITEST.NE.1) GO TO 340

```

```

      WRITE (61, 865)
      WRITE (61, 866)
      WRITE (61, 870) (A1(I), B1(I), C1(I), D1(I), E1(I), I=1, H)
      WRITE (61, 867)
      WRITE (61, 866)
      WRITE (61, 870) (A2(I), B2(I), C2(I), D2(I), E2(I), I=1, H)

```

```

340 CONTINUE

```

```

*****
CHAPTER HHHH **** UPSTREAM BOUNDARY CONDITIONS ****
*****
K1 = II

```

PROGRAM MOBED

73/171 OPT=1

FTN 4.8+538

86/12/L

CCCCCCCC

CALL USTREAM

\*\*\*\*\*

CHAPTER IIII \*\*\*\* STORAGE ADJUSTMENTS \*\*\*\*

\*\*\*\*\*

DO 350 I=2,N

J = I-1

E(I) = (A1(J)\*C2(J)+D2(J)\*E(J))-A2(J)\*(C1(J)+D1(J)\*E(J))/(B2

\$(J)\*(C1(J)+D1(J)\*E(J))-B1(J)\*(C2(J)+D2(J)\*E(J))

F(I) = ((E2(J)+D2(J)\*F(J))\*(C1(J)+D1(J)\*E(J))-(E1(J)+D1(J)\*F(J)

\$(J)\*(C2(J)+D2(J)\*E(J)))/(B2(J)\*(C1(J)+D1(J)\*E(J))-B1(J)\*(C2(J)+D2

\$(J)\*E(J))

IF (I.NE.IS) GO TO 350

E(I) = E(J)-SA\*/DELTAT

F(I) = G(J)-Q(J+1)+F(J)+E(J)\*(Y(J+1)-Y(J))

350 CONTINUE

IF (ITEST.NE.1) GO TO 355

WRITE (61,875)

WRITE (61,876)

WRITE (61,890) (E(I),F(I),I=2,N)

355

DO 360 I=1,H

L(I) = A1(I)/(C1(I)+D1(I)\*E(I))

M(I) = B1(I)/(C1(I)+D1(I)\*E(I))

K(I) = (E1(I)+J1(I)\*F(I))/(C1(I)+D1(I)\*E(I))

IF (I.NE.IS) GO TO 360

L(I) = 1.0

M(I) = 0.0

K(I) = 0.0-(Y(I+1)-Y(I))

360 CONTINUE

C TEMP

TOTALS = 0.0

IF (ITEST.NE.1) GO TO 365

WRITE (61,895)

WRITE (61,896)

WRITE (61,893) (L(I),M(I),K(I),I=1,H)

CCCCCCCC

\*\*\*\*\*

CHAPTER JJJJ \*\*\*\* OUTPUT \*\*\*\*

\*\*\*\*\*

DOWNSTREAM BOUNDARY CONDITIONS

365 DO 370 I=1,N

T(I) = I\*DELTAT-DELTAT

C TEMP

TOTALS = TOTALS + QST(I)

370

D(I) = I\*DELTAX-DELTAX

TIME = (TI+TP)\*DELTAT-DELTAT

MDDAYS = TIME/(3600.0\*24.0)

DAYS = MDDAYS

REM1 = AMOD(TIME,36400.0)

IHR = REM1/3600.0

HR = IHR

REM2 = AMOD(REM1,3600.0)

MIN = REM2/60.0

AMIN = MIN

REM3 = AMOD(REM2,50.0)

SEC = REM3

380

WRITE (61,950) DAYS,HR,AMIN,SEC

WRITE (61,960)

WRITE (61,970) (D(I),Q(I),Y(I),QST(I),FR(I),Z(I),B(I),A(I),C(I),R

\$(I),I=1,N)

IF (ISED.NE.1) GO TO 440

PROGRAM MOBED

73/171 OPT=1

FTN 4.8+538

86/12/1

```

WRITE (61,980) ASF
WRITE (61,990) BSLOPE
C TEMP
WRITE (61,745) TOTALS/N
WRITE (61,750)
WRITE (61,760) (D(I), (SDP(I,J)*100, J=1,10), D35(I), D65(I), BL
$(I), ABL(I), I=1, N)
WRITE (61,1000)
DO 435 I=1, N
GO TO (390,400,410,420,430), IBED(I)
390 WRITE (61,1010) J(I), CONST(I), EP(I), EN(I), TA(I)
GO TO 435
400 WRITE (61,1020) J(I), CONST(I), EP(I), EN(I), TA(I)
GO TO 435
410 WRITE (61,1030) J(I), CONST(I), EP(I), EN(I), TA(I)
GO TO 435
420 WRITE (61,1040) J(I), CONST(I), EP(I), EN(I), TA(I)
GO TO 435
430 WRITE (61,1050) J(I), CONST(I), EP(I), EN(I), TA(I)
435 CONTINUE
440 CONTINUE
CCCCCCCC
..... REMOVE COMMENTS FOR PLOT FILE (TAPE 6) AT DAY ** OF EACH RUN
D,Z AND AD ARE THE DISTANCE DOWNSTREAM, BED AND WATER LEVELS RESP.
CCCCCCCC
IF (DAYS.NE.***) GO TO 449
REWIND 6
DO 445 I=1, N
AD(I)=Y(I)+Z(I)
445 WRITE (6,446) (D(I), Z(I), AD(I))
446 FORMAT (3F12.6)
ENDFILE 6
CCCCCCCC
449 IF (II.EQ.(IL-1)) GO TO 450
GO TO 480
450 REWIND 8
WRITE (8,485) IL, IP, ITEST
WRITE (8,490) N, INFLOW, IN, INT, IS, ISED, IFRICT, METRIC, XIN
WRITE (8,500) THETA, DELTAX, DELTAT, XLENGH, G, QIN, QSED, SAR
WRITE (8,510) (Y(I), I=1, N)
WRITE (8,520) (Q(I), I=1, N)
WRITE (8,530) (Z(I), I=1, N)
WRITE (8,540) IYEAR, IDEEP, IPRNT
WRITE (8,550) GAM, GAMS, ANU
WRITE (8,560) PORS
WRITE (8,570) (BL(I), I=1, N)
WRITE (8,580) (ABL(I), I=1, N)
WRITE (8,590) (D35(I), I=1, N)
WRITE (8,600) (D65(I), I=1, N)
DO 457 I = 1, N
457 WRITE (8,560) (SDP(I,J), J=1,10)
C
WRITE (8,560) (TQS(I), I=1, IL)
WRITE (8,570) (ITYPEUP, I=1, IL)
IF(((ITYPEUP.EQ.2).AND.(ITYPEDN.EQ.1)).OR.((ITYPEUP.EQ.1).AND.
$(ITYPECN.EQ.2))) GO TO 452
IF(((ITYPEUP.EQ.3).AND.(ITYPEDN.EQ.1)).OR.((ITYPEUP.EQ.1).AND.
$(ITYPECN.EQ.3))) GO TO 454
IF ((ITYPEUP.EQ.3).AND.(ITYPEDN.EQ.3)) GO TO 456
WRITE (8,520) (TQ(I), I=1, IL)
GO TO 456
452 WRITE (8,520) (TQ(I), I=1, IL)
454 WRITE (8,520) (TY(I), I=1, IL)
456 IF(INFLOW.NE.0) WRITE (8,511) (NT(I), I=1, NTOT)
IF(INFLOW.NE.0) WRITE (8,520) ((TQL(I,J), J=1, IL), I=1, NTOT)
IF(INFLOW.NE.0) WRITE (8,520) ((QSED(I,J), J=1, IL), I=1, NTOT)
000 IF ((ISED.NE.1).OR.(IFRICT.NE.1)) WRITE (8,580) CONSTR, (EM(I), EN
$(I), IBED(I), I=1, N)
460 CONTINUE
CCCCCCCC
*****
CHAPTER KKKK **** DOWNSTREAM BOUNDARY CONDITIONS ****

```





```

590 FORMAT (1H1)
600 FORMAT (1H0,*** TEST MODE ASSUMED *** )
610 FORMAT (1H0,#NUMBER OF GRID POINTS ALONG THE RIVER REACH N=#,I5)
620 FORMAT (1H0,#CONTROL PARAMETER FOR TRIBUTORY PRESENCE INFLOW=#,I5)
630 FORMAT (1H0,#NUMBER OF TRIBUTARIES=#,I5)
640 FORMAT (1H0,#CONTROL PARAMETER FOR STORAGE BASIN IS=#,I5)
650 FORMAT (1H0,#CONTROL PARAMETER SIGNIFYING THE NATURE OF FLOW #,
660 $#BOUNDARY USED=#,I5)
670 FORMAT (1H0,#WEIGHTING COEFFICIENT THETA=#,F8.3)
680 FORMAT (1H0,#DISTANCE IN METERS BETWEEN GRID POINTS DELTA X=#,F8.2)
690 FORMAT (1H0,#TIME INCREMENTS IN SECONDS DELTA T=#,F9.1)
700 FORMAT (1H0,#TOTAL LENGTH OF RIVER REACH MODELLED XLENGTH=#,F10.2)
710 FORMAT (1H0,#ACCELERATION DUE TO GRAVITY G=#,F8.4)
740 FORMAT (1H0,#WATER SURFACE AREA OF STORAGE BASIN SAR=#,F8.4)
C
745 EMP
750 FORMAT (1H0,#AVERAGE SEDIMENT TRANSPORT RATE = #,F16.10)
      FORMAT (1H1,#NEW SEDIMENT CHARACTERISTICS#,,/5X,#RANGE#,19X,
      $#PERCENTAGES OF GRAIN SIZES 1 THRU 10 RESPECTIVELY#,22X,#D35#,7X,
      $#D65#,3X,#BED DEPTH#,3X,#ABL#,/)
760 FORMAT (F11.2,2X,10F8.3,1X,2F10.6,F9.3,F9.5)
770 FORMAT (1H0,#THE VALUE OF KINEMATIC VISCOSITY IS=#,F10.6)
780 FORMAT (1H0,#UPSTREAM BOUNDARY TYPE=#,I5)
790 FORMAT (1H0,#DOWNSTREAM BOUNDARY TYPE=#,I5)
800 FORMAT (1H0,#BOUNDARY VALUES#)
810 FORMAT (1H0)
815 FORMAT (3X,#UPSTREAM#,/,3X,#-----#)
816 FORMAT (3X,#DOWNSTREAM#,/,3X,#-----#)
820 FORMAT (1H0)
825 FORMAT (/1H,11X,#QST1#,11X,#QST2#,11X,#CAV1#,11X,#CAV2#,5X,
      $#(I=1,N)#)
826 FORMAT (1H,11X,#-----#,11X,#-----#,11X,#-----#,11X,#-----#)
830 FORMAT (/100(/1H,4F15.2)/)
835 FORMAT (/1H,5X,#(DELZ(I),I=1,N)#)
836 FORMAT (1H,5X,15(#-#))
840 FORMAT (/100(/1H,10F10.3))
850 FORMAT (1H0,#THE CORRECTED FLOW DEPTH GOES NEGATIVE.#,/,
      $# POSSIBLE CAUSE: UNREALISTIC VALUE FOR BED LEVEL CHANGE (DELZ).#)
860 FORMAT (1H, #REMEDY: REDUCE DELTA T AND RERUN#)
865 FORMAT (/1H,16X,#A1#,18X,#B1#,18X,#C1#,18X,#D1#,18X,#E1#,5X,
      $#(I=1,N-1)#)
866 FORMAT (1H,16X,#A2#,18X,#B2#,18X,#C2#,18X,#D2#,18X,#E2#,5X,
      $#(I=1,N-1)#)
868 FORMAT (/1H,15X,#DELY#,16X,#DELQ#,18X,#K#,18X,#F#,18X,#Q#,5X,
      $#(I=1,N-1)#)
869 FORMAT (1H,15X,#-----#,16X,#-----#,18X,#-----#,18X,#-----#)
870 FORMAT (/100(/1H,5F20.5))
875 FORMAT (/1H,8X,#E#,12X,#F#,5X,#(I=2,N)#)
876 FORMAT (1H,8X,#-----#,12X,#-----#)
880 FORMAT (/100(/1H,2F12.3))
885 FORMAT (/1H,9X,#L#,11X,#M#,11X,#K#,5X,#(I=1,N-1)#)
886 FORMAT (1H,9X,#-----#,11X,#-----#,11X,#-----#)
890 FORMAT (/100(/1H,3F12.3))
900 FORMAT (1H1,1X,#UPSTREAM BOUNDARY CONDITIONS#)
910 FORMAT (1H0,1X,#TIME#,6X,#FLOW RATE#)
920 FORMAT (1H,9.0,3X,F10.4)
930 FORMAT (1H0,1X,#DOWNSTREAM BOUNDARY CONDITIONS#)
940 FORMAT (1H0,1X,#TIME#,6X,#FLOW DEPTH#)
950 FORMAT (1H1,1X,#SOLUTION AT TIME T=#,F9.2,#DAYS#,F9.2,#HOURS#,
      $F9.2,#MINUTES#,F9.2,#SECONDS#)
960 FORMAT (1H0,1X,#DISTANCE#,2X,#FLOW RATE#,2X,#FLOW DEPTH#,2X,
      $#SEDIMENT RATE#,2X,#FROUDE N#,2X,#BOTTOM ELEVATION#,2X,#TOP WIDTH#,
      $2X,#FLOW AREA#,2X,#FRICTION FACTOR#,2X,#HYDRAULIC RADIUS#)
970 FORMAT (1H,1X,F8.2,2X,F10.4,1X,F10.4,F14.10,2X,F10.4,4X,F10.6,
      $4X,F10.4,2X,F10.4,4X,F10.4,8X,F10.4)
980 FORMAT (1H0,#AVERAGE ENERGY SLOPE OF THE REACH=#,F8.5)
990 FORMAT (1H0,#AVERAGE SLOPE OF BED=#,F8.5)
1000 FORMAT (/5X,#RANGE#,25X,#BED STATE#,44X,#FRICTION PARAMETERS#,
      $5X,#RATIO GSS/QST#,/.86X,#CONST#,6X,#M#,10X,#N#)
1010 FORMAT (F11.2,9X,#THE BED IS COVERED WITH CUNES OF TYPE 1#,20X,
      $F12.4,3F10.4)
1020 FORMAT (F11.2,9X,#THE BED IS COVERED WITH CUNES OF TYPE 2#,20X,
      $F12.4,3F10.4)
1030 FORMAT (F11.2,6X,#THE BED IS IN A TRANSITION STATE FROM DUNE 1 #,
      $TO FLAT BED#,7X,#12.4,3F10.4)
1040 FORMAT (F11.2,27X,#THE BED IS FLAT#,27X,F12.4,3F10.4)
1050 FORMAT (F12.2,18X,#THE BED IS COVERED WITH ANTI DUNES#,18X,F11.4,

```

PROGRAM MOBED

73/171 GPT=1

FTN 4.8+536

66/12/1

```
$3F10.4)
1050 FORMAT (6F10.6)
1060 FORMAT (1H0,*,CALCULATED FLOW DEPTH IS NEGATIVE*,/,*,REMEDY: *,
*,REDUCE DELTAT AND PERUN*)
1120 FORMAT (1H1,51X,1) (*,*,/,62X,*,REPORT *,/,62X,10(*,*,),
//,50X,19(*,*),2(/,50X,*,*,35X,*,*),/50X,*,*,12X,
*,40DEL MOBED),12X,*,*,/,50X,*,*,1X,*( WITH SEDIMENT *,
*,CHARACTERISTICS ),1X,*,*,2(/,50X,*,*,35X,*,*),/,50X,
19(*,*)
1130 FORMAT (1H1,*,Y(N) IS NEGATIVE *)
END
```

ROUTINE BED

73/171 OPT=1

FTN 4.8+538

66/12/1

SUBROUTINE BED(N,JNIFORM,UNISIZF)

SUBROUTINE TC CALCULATE D35 AND D65 FROM THE GIVEN SEDIMENT SIZE DISTRIBUTION

```

COMMON/B/ GAM,GAMS,D35(61),D65(61),ANU,G,ABL(61),SDP(61,10),BL(61)
$,SDPI(61,10)
COMMON/X/ A(61),P(61),W(61),DEPTH(61)
DIMENSION SPOT(2),PU(2),PL(2),DU(2),DL(2),D(10),UNISIZF(61)
INTEGER UNIFORM(61)
DATA (D(I),I=1,10) /0.000125,0.000250,0.000500,0.00100,0.0020,
0.004,0.008,0.016,0.032,0.064/
DATA SPOT(1),SPOT(2) /0.35,0.65/
DATA (DU(I),I=1,2) /2*0.0/
DATA (DL(I),I=1,2) /2*0.0/

```

```

IF (UNIFORM(N).EQ.1) GO TO 30
PU(1) = 0.0
PU(2) = 0.0
DO 20 L = 1,2
TEMP = 0.0
DO 10 K=1,10
TEMP = TEMP + SDP(N,K)
IF ((TEMP.LT.SPOT(L)).OR.(PU(L).NE.(0))) GO TO 10
PU(L) = TEMP
PL(L) = TEMP - SDP(N,K)

```

STOP 003 CHECK FOR ZERO DENCMINATOR FOR D65,D35 CALCULATION

```

IF (PU(L).EQ.PL(L)) GO TO 50
DU(L) = D(K)
DL(L) = D(K-1)
CONTINUE
IF ((DU(L).LT.0.0).OR.(DL(L).LT.0.0)) GO TO 50
CONTINUE
D35(N) = 10**((0.35-PL(1))/(PU(1)-PL(1))+(ALOG10(DU(1))-ALOG10(
DL(1)))/ALOG10(DL(1)))
D65(N) = 10**((0.65-PL(2))/(PU(2)-PL(2))+(ALOG10(DU(2))-ALOG10(
DL(2)))/ALOG10(DL(2)))

```

```

GO TO 45
CONTINUE
DO 35 I = 1,10
D35(N) = D(I)*UNISIZF(N)
IF (SDP(N,I).GT.0.1) GO TO 40
CONTINUE
D65(N) = D35(N)
CONTINUE
RETURN

```

```

WRITE (61,120) PL(L),PU(L),DL(L),DU(L)
STOP 003
FORMAT (1H, #CALCULATION OF D35 OR D65 WOULD HAVE RESULTED#,
# IN DIVISION BY ZERO IN SUBROUTINE BED OR LOG OF #,
# NEGATIVE NUMBER #, /, # PL = #, F10.5, # PU = #, F10.5,
# DL = #, F10.5, # DU = #, F10.5)

```

END

ROUTINE NEWBED

73/171 CPT=1

FTN 4.8+538

86/12/

```

SUBROUTINE NEWBED(I,DELTAZ,ASFL,Q,A,R,CONST,EM,EN,UNIFORM)
SUBROUTINE TO RECALCULATE THE NEW PERCENTAGES OF EACH BED SIZE
AND THE ADJUSTED ACTIVE BED LAYER DEPTH
COMMON/A/ Y(61),B(61),II,TQS(200)
COMMON/B/ GAM,GAYS,D35(61),D65(61),ANU,G,ABL(61),SDP(61,10),BL(61)
SDPI(61,10)
DIMENSION A(61),R(61)
DIMENSION VCR(10),QP(10),D(10),ASFL(61),Q(61),CONST(61),EM(61),
EN(61)
INTEGER UNIFORM(61)
DATA (VCR(I),I=1,10) /0.0120,0.0120,0.0146,0.0135,0.0306,0.0067,
0.0085,0.0073,0.1377,0.1947/
DATA (D(I),I=1,10) /0.0000,0.0016,0.000375,0.00075,0.0015,
0.0030,0.006,0.012,0.024,0.048/
C
IF(UNIFORM(I).EQ.1) GO TO 100
FR=(Q(I)/A(I))*2/(G*R(I))
ASL=CONST(I)*(R(I)/D65(I))*EM(I)*FR*EN(I)
VS = SQRT(G*Y(I)*ASL)
M = 1
DO 10 J = 1,10
K = 10 - (J-1)
IF (VS.LT.VCR(K)) M=K
CONTINUE
IF (M.EQ.1) GO TO 70
M1 = M - 1
SUM = 0.0
DO 20 J = 1,M1
SUM = SUM + SDP(I,J)
C
PC = 1.0 - SUM
IF (PC.LE.0.00001) GO TO 60
ABL(I) = 2.0*D65(I)
CV = 0.57
SUM1 = 0.0
SUM2 = 0.0
DO 30 J=1,10
XU = ((VCR(J)/VS)**2-1.)/CV
CP(J) = ERF(XU/SQRT(2.0))/2.0 + 0.5
SUM1 = SUM1 + CP(J)*SDP(I,J)
SUM2 = SUM2 + (1.0-QP(J))*SDP(I,J)
CONTINUE
IF(DELTAZ.LT.0.0) GO TO 95
DIN = ABL(I)+DELTAZ*SUM1
IF (DIN.EQ.0.0) GO TO 80
DO 40 J = 1,10
SDP(I,J) = (ABL(I)*SDP(I,J)+DELTAZ*QP(J)*SDP(I,J))/DIN
CONTINUE
GO TO 70
95 CONTINUE
DELTAB=ABS(DELTAZ)
IF(ABL(I).LT.DELTAB) ABL(I)=DELTAB
DIN =DELTAZ*SUM1+ABL(I)-DELTAB
DO 96 J=1,10
SDP(I,J)=(DELTAB*QP(J)*SDP(I,J)+(ABL(I)-DELTAB)*SDP(I,J))/DIN
CONTINUE
GO TO 70
66 DO 66 J=1,10
SDP(I,J)=SDPI(I,J)
CONTINUE
65 BL(I)=BL(I)+DELTAZ
70 IF (BL(I).LE.C.0) GO TO 9C
100 RETURN
80 WRITE (61,120)
STOP 004
90 DELTAZ = 0.0
BL(I) = 0.0
RETURN
120 FORMAT (1F0,2ENGINATOR ZERO IN SUBROUTINE NEWBED#)
END

```

ROUTINE USTREAM 73/171 OPT=1

FTN 4.3+538

86/12/

```
      SUPROUTINE USTREAM
      SUBROUTINE TO ESTABLISH UPSTREAM BOUNDARY DOUBLE
      SWEEP COEFFICIENTS
      COMMON/A/ Y(61),B4(61),II,TQS(200)
      COMMON/C/ ITYPEUP,ITYPEDN,K1,AAU,BBU,CCU,AAD,BBD,CCD
      COMMON/D/ TQ(200),TY(200),E(61),F(61),DELY(61),Q(61)
      COMMON/E/ DELTAX,DELTA T,N
      GO TO (100,110,120), ITYPEUP
100   E(1) = 100000.00
      F(1) = -E(1)*(TY(K1+1)-TY(K1))
      RETURN
110   E(1) = 0.0
      F(1) = TQ(K1+1)-TQ(K1)
      RETURN
120   F(1) = AAU*Y(1)**BBU-Q(1)
      E(1) = AAU*SEU*Y(1)**(BBU-1)
      RETURN
      END
```

VERIFY DETAILS DIAGNOSIS OF PROBLEM

I AN IF STATEMENT MAY BE MORE EFFICIENT THAN A 2 OR 3 BRANCH COMPUTED

ROUTINE DSTREAM 73/171 OPT=1

FTN 4.8+538

86/12/C

```
      SUBROUTINE DSTREAM
C     SUBROUTINE TO ESTABLISH DOWNSTREAM BOUNDARY
C     VALUES OF DELY(N)
C
      COMMON/A/ Y(51),B4(61),II,TQS(200)
      COMMON/C/ ITYPEUP,ITYPEDN,K1,AAU,BBU,CCU,AAD,BBD,CCD
      COMMON/D/ TQ(200),TY(200),E(61),F(61),CELY(61),Q(61)
      COMMON /E/ DELTAX,DELTAT,N
C
      GO TO (100,110,120), ITYPEDN
C
100  DELY(N) = TY(K1+1)-TY(K1)
      RETURN
110  DELY(N) = ((TQ(K1+1)-TQ(K1))-F(N))/E(N)
      RETURN
C
120  S=Y(N)-0.800
      IF(S.GT.0.1775) GO TO 10
      DELY(N)=(Q(N)+F(N)-3.0585*S**3.0889)/(9.4474*S**2.0889-E(N))
      GO TO 20
10   DELY(N)=(Q(N)+F(N)-.754*S**2.27905)/(1.7184*S**1.27905-E(N))
20   CONTINUE
      RETURN
      END
```

VERIFY DETAILS DIAGNOSIS OF PROBLEM

I

AN IF STATEMENT MAY BE MORE EFFICIENT THAN A 2 OR 3 BRANCH COMPUTEC

ROUTINE TRIBUT

73/171 OPT=1

FTN 4.8+538

86/12/1

CCCC

SUBROUTINE TRIBUT(N, II, TQL, NT, NTOT, QL1, QL2)

THIS SUBROUTINE COMPUTES TRIBUTARY-PARAMETERS

DIMENSION TQL(6,200), NT(6), QL1(61), QL2(61)

DO 10 I=1, N

QL1(I)=0.00

QL2(I)=0.00

10 CONTINUE

DO 20 I=1, NTOT

NI=NT(I)

QL1(NI)=TQL(I, II)

20 QL2(NI)=TQL(I, II+1)-TQL(I, II)

CONTINUE

RETURN

END



ROUTINE GEOMET

73/171 OPT=1

FTN 4.8+538

86/12/C

```

SUBROUTINE GEOMET (A,P,R,B,DERB,DERP,Q,AYX,AVY,FR,IDEEP,RNEW,
$ ABL,SD35,SD65,SSDP,UNIFORM,SBL,UNISIZF)
C
C SUBROUTINE TO ESTABLISH THE GEOMETRIC CHARACTERISTICS
C OF THE RIVER SECTION
C
C DIMENSION AR(61),TW(61),WP(61),D(61)
C COMMON/A/Y(61),BW(61),II,TQS(200)
C COMMON/B/ GAM,GAMS,D35(61),D65(61),ANU,G,ABL(61),SDP(61,10),BL(61)
C $ SDPI(61,10)
C COMMON/E/ DELTAX,DELTAT,N
C DIMENSION A(61),P(61),R(61),B(61),DERB(61),DERP(61),Q(61),
C $AYX(61),AVY(61),FR(61)
C DIMENSION AY(61),SABL(61),SD35(61),SD65(61),TSDP(10),SSDP(61,10)
C DIMENSION RNEW(61),ELEV(61),SBL(61),UNISIZF(61)
C INTEGER H,UNIFORM(61),TUNIF
C G = 9.81
C IEND = IDEEP+1
C FEET = 1./(.0254*12.)
C
C YAV = 0.0
C DO 100 I=1,N
C YAV = YAV+Y(I)
100 CONTINUE
C YAV = YAV/N
C
C I1AV = IFIX((YAV*FEET)+1.00001)
C I2AV = I1AV+1
C RNO = 999999.0
C
C DO 150 I=1,N
110 IF (RNEW(I).EQ.RNO) GO TO 130
C
C DO 120 JI=1,IEND
C READ (4) RNO,ELEV(JI),D(JI),AR(JI),WP(JI),TW(JI)
120 CONTINUE
C READ (4) TBL,TABL,TD35,TD65,(TSDP(KK),KK = 1,10),TUNIF,TUNIS
C IF (RNEW(I).EQ.RNO) GO TO 130
C GO TO 110
C
C 130 CONTINUE
C IF (II.NE.1) GO TO 137
C BL(I) = TBL
C ABL(I) = TABL
C D35(I) = TD35
C D65(I) = TD65
C DO 135 KK = 1,10
135 SDPI(I, KK) = TSDP(KK)
C SDP(I, KK) = TSDP(KK)
C UNIFORM(I) = TJNIF
C UNISIZF(I) = TJNIS
137 CONTINUE
C CALL SEEK (AV,AR(I1AV),AR(I2AV),YAV,D(I1AV),D(I2AV))
C
C J = IFIX((Y(I)*FEET)+1.00001)
C K = J+1
C
C STOP 005 FLOW DEPTH EXCEEDING MAX DEPTH CHECK
C
C IF (K.LE.IDEEP) GO TO 140
C WRITE (61,170)
C WRITE (61,180)
C STOP 005
C
C 140 CALL SEEK (A(I),AR(J),AR(K),Y(I),D(J),D(K))
C CALL SEEK (P(I),WP(J),WP(K),Y(I),D(J),D(K))
C CALL SEEK (B(I),TW(J),TW(K),Y(I),D(J),D(K))
C
C DERB(I) = (TW(K)-TW(J))/(D(K)-D(J))
C DERP(I) = DERP(I)
C AY(I) = AV
C
C R(I) = A(I)/P(I)
C AVY(I) = A(I)/B(I)
C FR(I) = (Q(I)*Q(I)/(A(I)*A(I)))/(G*R(I))
C

```

ROUTINE GEOMET

73/171 OPT=1

FTN 4-8+538

86/12/L

```
      IF ((SBL(I),EQ,0.0).OR.(II,NE,1)) GO TO 150
      BL(I) = SBL(I)
      ABL(I) = SABL(I)
      D35(I) = SD35(I)
      D65(I) = SD65(I)
      DO 145 KK = 1,10
145    SDP(I, KK) = SSDP(I, KK)
150  CONTINUE
      H = N-1
      DO 160 I=1, H
160    AYX(I) = (AY(I+1)-AY(I))/DELTAX
      CONTINUE
      AYX(N) = AYX(H)
      RETURN
C 170  FORMAT (1H0, #THE CALCULATED FLOW DEPTH EXCEEDS THE #,
180    #MAXIMUM CHANNEL DEPTHS SPECIFIED.#)
      FORMAT (1H ,#REMEJY# INCREASE THE VALUE OF IDEEP*#)
      END
```

ROUTINE SEEK

73/171 GPT=1

FTN 4.8+538

86/12/1

```
C   SUBROUTINE SEEK (Z,A1,A2,Y,D1,D2)
C   Z = A1+((A2-A1)/(D2-D1))*(Y-D1)
   RETURN
   END
```

ROUTINE FRICT

73/171 OPT=1

FTN 4.8+538

86/12/C

```

SUBROUTINE FRICT (Y,Z,Q,A,R,CONST,EM,EN,ASF,BSLOPE,IBED,IFRICT,
$ISED,ASFL)
C
C SUBROUTINE TO ESTABLISH THE FRICTIONAL PARAMETERS NAMELY
C CONST, M, AND N FOR MOBILE BOUNDARY FLCS
COMMON/B/ GAM,GAMS,D35(61),D65(61),ANU,G,ABL(61),SDP(61,10),BL(61)
$,SCPI(61,10)
COMMON/E/ DELTAX,DELTAT,N
DIMENSION Y(61),Z(61),Q(61),A(61),R(61),EL(61),X(61),CONST(61),
$EM(61),EN(61),ASFL(61)
INTEGER IBED(61)
C
DO 100 I=1,N
EL(I) = Z(I)+Y(I)+(Q(I)*Q(I)/(A(I)*A(I)))/(2.0*G)
X(I) = DELTAX*(I-1)
100 CONTINUE
C COMPUTATION OF LOCAL ENERGY SLOPES
C
N1=N-1
DO 10 I=2,N1
EDIF=(EL(I-1)-EL(I+1))
IF((Q(I).GT.0.0).AND.(EDIF.LT.0.0)) GO TO 220
IF((Q(I).LT.0.0).AND.(EDIF.GT.0.0)) GO TO 220
ASFL(I)=ABS(EDIF)/(2.0*DELTAX)
10 CONTINUE
ASFL(1)=ASFL(2)
ASFL(N)=ASFL(N1)
SUM1 = 0.00
SUM2 = 0.00
SUM3 = 0.00
SUM4 = 0.00
SUM5 = 0.00
DO 110 I=1,N
SUM1 = SUM1+1.0
SUM2 = SUM2+X(I)
SUM3 = SUM3+X(I)*X(I)
SUM4 = SUM4+EL(I)
SUM5 = SUM5+EL(I)*X(I)
110 CONTINUE
S0 = SUM1
S1 = SUM2
S2 = SUM3
T0 = SUM4
T1 = SUM5
A1 = (S0*T1-S1*T0)/(S0*S2-S1*S1)
A0 = (S2*T0-S1*T1)/(S0*S2-S1*S1)
ASF = ABS(A1)
C
SUM4 = 0.00
SUM5 = 0.00
DO 120 I=1,N
SUM4 = SUM4+Z(I)
SUM5 = SUM5+Z(I)*X(I)
120 CONTINUE
T0 = SUM4
T1 = SUM5
A1 = (S0*T1-S1*T0)/(S0*S2-S1*S1)
A0 = (S2*T0-S1*T1)/(S0*S2-S1*S1)
BSLOPE = ABS(A1)
IF((ISED.NE.1).OR.(IFRICT.NE.1)) RETURN
SUM = 0.00
DO 130 I=1,N
SUM = SUM+R(I)
130 CONTINUE
C
DO 190 I=1,N
IF(D65(I).EQ.0.0) GO TO 200
AVR = SUM/N
AYD = GAM*AVR*ASF/(GAMS*D65(I))
AL1 = 0.02*(AVR/D65(I))**0.50
AL2 = 0.02*(AVR/D65(I))**0.56
AL3 = 0.07*(AVR/D65(I))**0.40
IF(AL3.LT.AL2)AL3 = AL2
AL1L = AL1-0.000001
AL1H = AL1+0.000001

```

ROUTINE FRICT

73/171 OPT=1

FTN 4.8+538

86/12/L

```
G
CCCCC TRANSFERING CONTROL TO SET PROPER FRICTION PARAMETERS
CCCCC TEMP
CCCCC CONST(I) = 0.0340
CCCCC EM(I) = 0.0
CCCCC EN(I) = 3.0
CCCCC IBED(I) = 1
CCCCC GO TO 190
IF (AYD.LT.AL1L) GO TO 140
IF ((AYD.GT.AL1L).AND.(AYD.LT.AL1H)) GC TO 150
IF ((AYD.GT.AL1).AND.(AYD.LT.AL2)) GO TO 160
IF ((AYD.GT.AL2).AND.(AYD.LT.AL3)) GO TO 170
IF (AYD.GT.AL3) GO TO 180
140 CONST(I)=(GAM/GAMS)**2/191.0
EM(I)=1.0
EN(I)=3.00
IBED(I)=1
IF(Q(I).LT.0.0) CONST(I)=-CONST(I)
GO TO 190
150 CONST(I)=1.0/91.0
EM(I)=0.00
EN(I)=1.0
IBED(I)=2
IF(Q(I).LT.0.0) CONST(I)=-CONST(I)
GO TO 190
160 CONST(I)=1.0/(53.43*(GAM/GAMS)**0.857)
EM(I)=-0.429
EN(I)=0.143
IBED(I)=3
IF(Q(I).LT.0.0) CONST(I)=-CONST(I)
GO TO 190
170 CONST(I)=1.0/47.5
EM(I)=-0.333
EN(I)=1.00
IBED(I)=4
IF(Q(I).LT.0.0) CONST(I)=-CONST(I)
GO TO 190
180 CONST(I)=(GAM/GAMS)**2/482.00
EM(I)=0.20
EN(I)=3.00
IBED(I)=5
IF(Q(I).LT.0.0) CONST(I)=-CONST(I)
190 CONTINUE
FEIURN
200 WRITE (61,210)
STOP 007
210 FORMAT (1H1,2 ** J65 = 0 ZERO DIVISION IF FRICT*)
220 WRITE (61,230)
STOP 020
230 FORMAT (1H1,2***** ENERGY SLOPE IS NEGATIVE IN FRICT***)
END
```

ROUTINE SEDI

73/171 OPT=1

FTN 4.8+538

86/12/L

SUBROUTINE SEDI (I,A,AVY,R,FR,ASF,QST,CAV,C,TA)

SUBROUTINE TO CALCULATE THE SEDIMENT TRANSPORT RATES AND AVERAGE CONCENTRATION OF SEDIMENT

COMMON/A/ Y(61),B1(61),II,TQS(200)
COMMON/B/ GAM,GAMS,D35(61),D65(61),ANU,G,ABL(61),SOP(61,10),BL(61)
COMMON/E/ DELTAX,DELTAT,N
DIMENSION Q(61),A(61),AVY(61),C(61),CAV(61),QST(61)
DIMENSION R(61),FR(61),ZETA(26),Y1(26),Y2(26),TA(61)
REAL NP,LHS

SEDIMENT CONSTANTS

DO 150 I=1,N

DGP = D35(I)\*(G\*GAMS/(ANU\*ANU\*GAM))\*\*(1.0/3.0)
IF(DGR.LT.1.0)DGR = 1.0
IF(DGR.GE.1.0.AND.DGR.LE.60.0) GO TO 100
RN = 0.0
RA = .17
RM = 1.5
RC = 0.025
GO TO 110
100 RN = 1.-.56\*ALOG10(DGR)
RA = .23/SCRT(DGR)+.14
RM = 9.66/DGR+1.34
RCC = 2.86\*ALOG10(DGR)-(ALOG10(DGR))\*\*2-3.53
RC = 10.0\*\*(RCC)
110 CONTINUE

STOP 006 CHECK FOR -VE HYDRAULIC RADIUS VALUES

IF(R(I).GT.0.0) GO TO 120
WRITE(61,160)
WRITE(61,170) R(I)
STOP 006

120 CONTINUE

CALCULATION OF FALL VELOCITY W

LCHECK = 0
DGR3 = DGR\*\*3
IF(DGR3.LE.500000.0) GO TO 121
CT = 1.825
NP = 0.0
GO TO 125
121 CONTINUE
IF(DGR3.LE.100.0) GO TO 122
CT = 0.1585
NP = 0.2
GO TO 125
122 CT = 0.056
NP = 0.5
125 W = CT\*DGR3\*\*NP\*SQRT(GAMS/GAM\*G\*D35(I))

CALCULATION OF CRITICAL SHEARVELOCITY FOR INITIATION OF SUSPENSION

IF(DGR.GT.10.0) GO TO 90
UCRS=4.0\*W/DGR
GO TO 80
90 UCRS=0.4\*W
80 CONTINUE

CALCULATION OF GRAIN ROUGHNESS SHEAR VELOCITY

T1 = 0.40
VSP = SQRT(G\*ASF\*Y(I))
126 LCHECK = LCHECK + 1
IF(LCHECK.GE.5) GO TO 155

ROUTINE SEDI

73/171 OPT=1

FTN 4.8+538

86/12/1

```

TEMP1 = VSP*2.5*D65(I)/ANU
IF (TEMP1.LT.0.0) GO TO 157
X = ALOG(TEMP1)
BS = (2.5*X+5.5)*EXP(-0.217*X**2)+8.5*(1.0-EXP(-0.217*X**2))
AS = EXP(T1*BS)
HP = VSP**2/G**SF
TEMP1 = AS/2.72*HP/2.5/D65(I)
IF (TEMP1.LT.0.0) GO TO 158
RHS = 1.0/T1*A.LOG(TEMP1)
LHS = Q(I)/A(I)/VSP
T2 = ABS(LHS - RHS)
IF (T2.LT.0.001) GO TO 127
T3 = (LHS + RHS)/2.0
VSP = Q(I)/A(I)/T3
GO TO 126
127 CONTINUE
C
C(I) = (Q(I)/A(I))/SQRT(G*R(I)*A SF)
IF (AVY(I).LT.0.0) GO TO 156
FGR = (C(I)/A(I))*(SQRT(32.0)*ALOG(10(10.0*AVY(I)/D35(I))))**
$ (FN-1.0)/(C(I)**RN*SQRT(G*D35(I)*(GMS/GAM)))
IF (FGR.LE.RA) GO TO 130
GO TO 140
130 FGR = RA
140 ZX = RC*((FGR-RA)/RA)**RM
XX = ZX*((GMS/GAM)+1)*D35(I)*C(I)**RN/AVY(I)
QST(I) = ZX*D35(I)*(Q(I)/A(I))*C(I)**RN
QST(I) = QST(I)*A(I)/AVY(I)
C
C
C
C
CALCULATION OF RATIO QSS/QST
NL = 25
NP1 = NL + 1
NM1 = NL - 1
VS = SQRT(G*ASF*Y(I))
IF (VS.LT.UCRS) GO TO 145
Z = H/VSP/T1
ZA = 2.0*D65(I)/Y(I)
H = (1.0-ZA)/N:
ZETA(1) = ZA
DO 141 J = 2,NP1
ZETA(J) = ZETA(J-1) + H
IF (ZETA(J).LT.0.0) GO TO 159
141 CONTINUE
DO 142 J = 1,NP1
Y1(J) = ((1.0-ZETA(J))/ZETA(J))**Z
Y2(J) = Y1(J)**ALOG(ZETA(J))
SUM1 = 0.0
SUM2 = 0.0
DO 143 J = 2,N1+2
SUM1 = SUM1 + Y1(J)*4.0
143 SUM2 = SUM2 + Y2(J)*4.0
DO 144 J = 3,N1+2
SUM1 = SUM1 + Y1(J)*2.0
144 SUM2 = SUM2 + Y2(J)*2.0
TINT1 = (SUM1+Y1(1)+Y1(NP1))*H/3.0
TINT2 = (SUM2+Y2(1)+Y2(NP1))*t/3.0
TEMP1 = AS*Y(I)/2.72/2.5/D65(I)
IF (TEMP1.LT.0.0) GO TO 154
TA(I) = Y(I)/D65(I)/T1/23.2*(ZA/(1.0-ZA))**Z*(ALOG(TEMP1)
$ *TINT1+TINT2)
TA(I) = (1.0-(1.0/(1.0+TA(I))))
GO TO 146
145 TA(I) = 0.00
146 CAV(I) = QST(I)/Q(I)*TA(I)
150 CONTINUE
C
C
C
C
SPECIFICATION OF THE UPSTREAM BOUNDARY CONDITION FOR
SEDIMENT TRANSPORT RATE
QST(1) = TQS(1)/2650.0
CAV(1) = QST(1)/Q(1)
RETURN
C
154 WRITE (61,200)
STOP 017
155 WRITE (61,180) LHS,RHS
STOP 011

```

ROUTINE SEDI

73/171 OPT=1

FTN 4.8+538

86/12/0

```
156 WRITE (61,190)
      STOP 012
157 WRITE (61,200)
      STOP 013
158 WRITE (61,200)
      STOP 014
159 WRITE (61,200)
      STOP 015
```

```
C
160 FORMAT (1H0,*,THE CALCULATED HYDRAULIC RADIUS IS NEGATIVE*,/,
*,POSSIBLE CAUSE: CORRECTED FLOW DEPTH GOES NEGATIVE.*,/,
*,REMEDY: REDUCE DELTAT AND RERUN.*)
170 FORMAT (1H,5HR(I)=,2X,E20.5)
180 FORMAT (1H1,*,LOOP CHECK IN SEDI HIT 50*,/,*,LHS = *,F15.6,
*,RHS = *,F15.6)
190 FORMAT (1H1,*,AVY(I) WAS NEGATIVE IN SEDI CANNOT TAKE LOG*)
200 FORMAT (1H1,*,NEGATIVE ALOG ARGUMENT IN SEDI, CHECK STOP*)
      END
```



ROUTINE PROFILE 73/171 OPT=1

FTN 4.8+538

86/12/C

```
      SUBROUTINE PROFILE (METRIC, IDEEP, RANGE, NUM, IPRNT, IYEAR, XIN, UNIFORM
      $M, UNISIZF)
C
C   SUBROUTINE TO COMPUTE THE GEOMETRIC CHARACTERISTICS OF
C   THE RIVER REACH FOR EACH GRID LOCATION
      COMMON/Z/X(500), Y(500), XU(500), YU(500)
      COMMON/X/A(61), P(61), W(61), DEPTH(61)
      COMMON/B/ GAM, GAMS, D35(61), D65(61), ANU, G, ABL(61), SDP(61,10), BL(61)
      $ SDPT(61,10)
      DIMENSION ELEV(61), RANGE(61), UNISIZF(61)
      INTEGER UNIFORM(61)
      REAL LOWEST, METRIC
      DATA A(1), DEPTH(1)/0.0001866, 0/

      DATA P(1), W(1)/0.673, 0.60/

      NUM = 0
      SUM = 0.00
C
C 100 CONTINUE
C   THE CRSS SECTIONAL DATA ARE SPECIFIED IN A FORMAT
C   SIMILAR TO HEC-2
      SEENO    AN IDENTIFICATION NUMBER FOR THE SECTION
      NUMST    NUMBER OF POINTS TO DEFINE THE SHAPE OF THE SECTION
      XLCH     DISTANCE BETWEEN ADJACENT SECTIONS (IN METERS OR FEET)
      ABL      ACTIVE BED LAYER
      UNIFORM  BED CONTROL PARAMETER (SEE MAIN PROGRAM)
      UNISIZF  (SEE MAIN PROGRAM) OR FT
      Y        VERTICAL CO-ORDINATE OF A POINT ON THE PERIMETER (IN METERS)
      X        HORIZONTAL CO-ORDINATE OF THE SAME POINT (IN METERS OR FEET)
      READ (1,170) SEENO, NUMST, XLCH, TBL, UNIFORM(NUM+1), UNISIZF(NUM+1)
      IF (EOF(1)) 150, 110
C
C 110 CONTINUE
      LAST = NUMST
      TBL = TBL * METRIC
C
      NUM = NUM + 1
      SUM = SUM + XLCH
      RANGE(NUM) = SUM * METRIC + XIN
      READ (1,180) (X(I), Y(I), I=1, LAST)
C
      LOWEST = 99999.
      DO 120 I=1, LAST
      IF (Y(I) .LT. LOWEST) LOWEST = Y(I)
C 120 CONTINUE
      IM = IDEEP + 1
      ELEV(1) = LOWEST
      DO 130 I=2, IM
      ELEV(I) = LOWEST + FLOAT(I-1) * 0.3048 / METRIC
      CALL COMPARE (LAST, ELEV(I), IEND)
      CALL CRSS (I, IEND, ELEV(I), METRIC)
C
C 130 CONTINUE
      IF (IPRNT.NE.0) WRITE (61,190)
      ELEV(1) = ELEV(1) + METRIC
      DO 140 I=1, IM
      WRITE (4) RANGE(NUM), ELEV(I), DEPTH(I), A(I), P(I), W(I)
      IF (IPRNT.NE.0) WRITE (61,200) RANGE(NUM), ELEV(I), DEPTH(I), A(I)
```

```
140 $ P(I),W(I)
    CONTINUE
    TAPL = 0.0
    READ (1,180) (SDP(NUM,I),I=1,10)
    WRITE (61,210) RANGE(NUM),TBL
    WRITE (61,220) (SDP(NUM,I)*100.0,I=1,10)
    CALL BED(NUM,UNIFORM,UNISIZE)
    WRITE (61,230) D35(NUM),D65(NUM)
    WRITE (4) TBL,TAB,D35(NUM),D65(NUM),(SDP(NUM,I),I=1,10),
    $UNIFORM(NUM),UNISIZE(NUM)
C
    GO TO 100
C
150 RETURN
C
160 FORMAT (1H1)
170 FORMAT (6X,F8.3,I+,4X,2F8.3,I3,F8.3)
180 FORMAT (9X,9F6.3)
190 FORMAT (1H1,8X,RANGE#,5X,ELEVATION#,5X,DEPTH#,15X,AREA#,10X,
    $WEIGHTED PERIMETER#,7X,TOP WIDTH#)
200 FORMAT (5X,F9.2,2=12,3,3F20.2)
210 FORMAT (1H1,///,SEDIMENT CHARACTERISTICS AT RANGE #,F9.2,/,
    $DEPTH OF BED LAYER INITIALLY = #,F10.3)
220 FORMAT (///,PERCENTAGE OF GRAINSIZES 1 THRU 10 RESPECTIVELY#,
    $//,10F10.3)
230 FORMAT (/,25X,D35 = #,F10.6,10X,D65 = #,F10.6)
    END
```

ROUTINE COMPARE 73/171 OPT=1

FTN 4.8+538

86/12/L

```
      SUBROUTINE COMPARE (LAST,ELEV,IEND)
C
C THIS SUBROUTINE COMPARES EACH ELEMENT IN Y WITH ELEV
C IF THE POINT IS UNDERWATER OR A BOUNDARY POINT WHICH MUST
C BE INTERPOLATED IT IS RECORDED IN THE ARRAYS XU AND YU
C WHERE XU IS A DISTANCE ALONG THE CROSSSECTION AND YU IS THE ELEVATION
C OF INTEREST
C
C      COMMON/Z/X(500),Y(500),XU(500),YU(500)
C      LOGICAL ABOVE
C
C      ABOVE = .TRUE.
C      J = 1
C
C      DO 140 I=1, LAST
C      IF(Y(I).EQ.0.0)GO TO 60
C      IF (Y(I)-ELEV) 100,100,130
C
C 100      IF (ABOVE) 110,120
C 110      ABOVE = .FALSE.
C          IF (I.EQ.1) GO TO 120
C          M = I-1
C          N = I
C          CALL FORMULA (M,N,ELEV,J)
C          J = J+1
C
C 120      YU(J) = Y(I)
C          XU(J) = X(I)
C          J = J+1
C          GO TO 140
C
C 130      IF (ABOVE) GO TO 140
C          ABOVE = .TRUE.
C          M = I
C          N = I-1
C          CALL FORMULA (N,M,ELEV,J)
C          J = J+1
C
C          GO TO 140
C 60      Y(I)=Y(I-1)
C          X(I)=X(I-1)
C 140      CONTINUE
C
C          IEND = J-1
C          RETURN
C          END
```

ROUTINE FORMULA 73/171 OPT=1

FTN 4.8+538

86/12/C

```
      SUBROUTINE FORMULA (N,M,ELEV,J)
C
C THIS SUBROUTINE INTERPOLATES TO FIND THE VALUE OF X MID-STATION
C WHERE Y EQUALS ELEV
C
C COMMON/Z/X(500),Y(500),XU(500),YU(500)
C
C XU(J) = X(N)-(((ELEV-Y(N))/(Y(M)-Y(N)))*(X(N)-X(M)))
C YU(J) = ELEV
C
C RETURN
C END
```

ROUTINE CROSS

73/171 OPT=1

FTN 4.8+538

86/12/L

```
      SUBROUTINE CROSS (J,IEND,H,METRIC)
C
C THIS SUBROUTINE CALCULATES THE CROSS-SECTIONAL AREA UNDER WATER,
C THE WETTED-PERIMETER, AND THE WIDTH
C
      COMMON/Z/X(500),Y(500),XU(500),YU(500)
      COMMON/X/A(61),P(61),W(61),DEPTH(61)
      REAL METRIC
C
      HH = H*METRIC
C
      A(J) = 0.
      P(J) = 0.
      W(J) = 0.
C
      IF (IEND.LE.1) GO TO 110
      IEND = IEND-1
C
      DO 100 I=1,IEND
      IF (YU(I).EQ.H.AND.YU(I+1).EQ.H) GO TO 100
C
      X1 = (XU(I+1)-XU(I))*METRIC
      Y1 = (YU(I+1)-YU(I))*METRIC
      YYU = (YU(I)+YU(I+1))*METRIC
C
      A(J) = A(J)+X1*(HH-(YYU/2.))
      P(J) = P(J)+SQRT(X1**2+Y1**2)
      W(J) = W(J)+X1
C
100  CONTINUE
110  DEPTH(J) = FLOAT(J-1)*0.3048
      H = H*METRIC
      RETURN
      END
```

ROUTINE KLIST

73/171 OPT=1

FTN 4.8+530

86/12/L

```
      SUBROUTINE KLIST (R,ILAST,N,NEWR,IPRNT,DELTA)
C     THIS SUBROUTINE ASSIGNS THE CORRECT AREAS TOP-WIDTHS
C     WETTED PERIMETERS TO EACH GRID POINT
C
      DIMENSION R(61),NEWR(61)
      REAL NEWR
      DO 120 L=1,N
        XD = (L-1)*DELTA
        AMIN = 9999.0
        DO 110 J=1,ILAST
          DIFF = XD-R(J)
          IF (ABS(DIFF).LT.AMIN) GO TO 100
          GO TO 110
        100  AMIN = DIFF
              K = J
        110  CONTINUE
              NEWR(L) = R(K)
        120  CONTINUE
C
      IF (IPRNT.NE.0) WRITE (61,130) (NEWR(J),J=1,N)
      RETURN
C
      130  FORMAT (1H1,#RANGE: NUMBERS ASSIGNED#,61(/1H ,F10.2))
      END
```

APPENDIX II

A Sample of Input Data in TAPE8









APPENDIX III

A Sample of Input Data in TAPE1



**APPENDIX IV**

**Sample Output**

\*\*\*\*\*  
\* REPORT \*  
\*\*\*\*\*

```
* * * * *
*
*
*      MODEL MOBED
* ( WITH SEDIMENT CHARACTERISTICS ) *
*
* * * * *
```



NUMBER OF GRID POINTS ALONG THE RIVER PEAC-1 N= 26  
CONTROL PARAMETER FOR TRIBUTORY PRESENCE INFLOW= 0  
NUMBER OF TRIBUTARIES= 0  
CONTROL PARAMETER FOR STORAGE BASIN IS= 0  
CONTROL PARAMETER SIGNIFYING THE NATURE OF FLOW BOUNDARY ISED= 1  
WEIGHTING COEFFICIENT THETA= .667  
DISTANCE IN METERS BETWEEN GRID POINTS DELTAX= .50  
TIME INCREMENTS IN SECONDS DELTAT= 600.0  
TOTAL LENGTH-1 OF RIVER REACH MODELLED XLENGTH= 12.50  
ACCELERATION DUE TO GRAVITY G= 9.8100  
WATER SURFACE AREA OF STORAGE BASIN SAR= 0.0000  
THE VALUE OF KINEMATIC VISCOSITY IS= .000001  
UPSTREAM BOUNDARY TYPE= 2  
DOWNSTREAM BOUNDARY TYPE= 1

BOUNDARY VALUES

UPSTREAM					
-----					
.013	.013	.013	.013	.013	.013
DOWNSTREAM					
-----					
.056	.056	.056	.056	.056	.056



SOLUTION AT TIME T=												
	0.00DAYS	0.00HOURS	0.00MINUTES	0.00SECONDS								
DISTANCE	FLOW RATE	FLOW DEPTH	SEDIMENT RATE	FROUDE N	BOTTOM ELEVATION	TOP WIDTH	FLOW AREA	FRICITION FACTOR	HYDRAULIC RADIUS			
0.00	.0127	.0560	0.0000000000	.3293	1.025000	.6000	.0338	12.8437	.0437			
.50	.0127	.0560	.0000002369	.3293	1.024000	.6000	.0338	12.8437	.0437			
1.00	.0127	.0560	.0000002369	.3293	1.023000	.6000	.0338	12.8437	.0437			
1.50	.0127	.0560	.0000002369	.3293	1.022000	.6000	.0338	12.8437	.0437			
2.00	.0127	.0560	.0000002369	.3293	1.021000	.6000	.0338	12.8437	.0437			
2.50	.0127	.0560	.0000002369	.3293	1.020000	.6000	.0338	12.8437	.0437			
3.00	.0127	.0560	.0000002369	.3293	1.019000	.6000	.0338	12.8437	.0437			
3.50	.0127	.0560	.0000002369	.3293	1.018000	.6000	.0338	12.8437	.0437			
4.00	.0127	.0560	.0000002369	.3293	1.017000	.6000	.0338	12.8437	.0437			
4.50	.0127	.0560	.0000002369	.3293	1.016000	.6000	.0338	12.8437	.0437			
5.00	.0127	.0560	.0000002369	.3293	1.015000	.6000	.0338	12.8437	.0437			
5.50	.0127	.0560	.0000002369	.3293	1.014000	.6000	.0338	12.8437	.0437			
6.00	.0127	.0560	.0000002369	.3293	1.013000	.6000	.0338	12.8437	.0437			
6.50	.0127	.0560	.0000002369	.3293	1.012000	.6000	.0338	12.8437	.0437			
7.00	.0127	.0560	.0000002369	.3293	1.011000	.6000	.0338	12.8437	.0437			
7.50	.0127	.0560	.0000002369	.3293	1.010000	.6000	.0338	12.8437	.0437			
8.00	.0127	.0560	.0000002369	.3293	1.009000	.6000	.0338	12.8437	.0437			
8.50	.0127	.0560	.0000002369	.3293	1.008000	.6000	.0338	12.8437	.0437			
9.00	.0127	.0560	.0000002369	.3293	1.007000	.6000	.0338	12.8437	.0437			
9.50	.0127	.0560	.0000002369	.3293	1.006000	.6000	.0338	12.8437	.0437			
10.00	.0127	.0560	.0000002369	.3293	1.005000	.6000	.0338	12.8437	.0437			
10.50	.0127	.0560	.0000002369	.3293	1.004000	.6000	.0338	12.8437	.0437			
11.00	.0127	.0560	.0000002369	.3293	1.003000	.6000	.0338	12.8437	.0437			
11.50	.0127	.0560	.0000002369	.3293	1.002000	.6000	.0338	12.8437	.0437			
12.00	.0127	.0560	.0000002369	.3293	1.001000	.6000	.0338	12.8437	.0437			
12.50	.0127	.0560	.0000002369	.3293	1.000000	.6000	.0338	12.8437	.0437			

AVERAGE ENERGY SLOPE OF THE REACH= .00200

AVERAGE SLOPE OF BED= .00200

AVERAGE SEDIMENT TRANSPORT RATE = .0000002274

NEW SEDIMENT CHARACTERISTICS

RANGE	PERCENTAGES OF GRAINSIZES 1 THRU 10 RESPECTIVELY										D35	D65	BED DEPTH	ABL	
	0.00	0.00	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000					0.000
0.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
1.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
1.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
2.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
2.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
3.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
3.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
4.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
4.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
5.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
5.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
6.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
6.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
7.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
7.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
8.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
8.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
9.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
9.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
10.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
10.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
11.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
11.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
12.00	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000
12.50	0.000	0.000	7.500	10.500	21.000	25.000	19.000	15.000	2.000	0.000	0.000	0.000	0.000	0.000	0.000

RANGE BED STATE FRICTION PARAMETERS N RATIO QSS/QST

RANGE	CONST	M	N	RATIO QSS/QST
0.00	0.0019	1.0000	3.0000	0.0000
.50	0.0019	1.0000	3.0000	0.0000
1.00	0.0019	1.0000	3.0000	0.0000
1.50	0.0019	1.0000	3.0000	0.0000
2.00	0.0019	1.0000	3.0000	0.0000
2.50	0.0019	1.0000	3.0000	0.0000
3.00	0.0019	1.0000	3.0000	0.0000
3.50	0.0019	1.0000	3.0000	0.0000
4.00	0.0019	1.0000	3.0000	0.0000
4.50	0.0019	1.0000	3.0000	0.0000
5.00	0.0019	1.0000	3.0000	0.0000
5.50	0.0019	1.0000	3.0000	0.0000
6.00	0.0019	1.0000	3.0000	0.0000
6.50	0.0019	1.0000	3.0000	0.0000
7.00	0.0019	1.0000	3.0000	0.0000
7.50	0.0019	1.0000	3.0000	0.0000
8.00	0.0019	1.0000	3.0000	0.0000
8.50	0.0019	1.0000	3.0000	0.0000
9.00	0.0019	1.0000	3.0000	0.0000
9.50	0.0019	1.0000	3.0000	0.0000
10.00	0.0019	1.0000	3.0000	0.0000
10.50	0.0019	1.0000	3.0000	0.0000
11.00	0.0019	1.0000	3.0000	0.0000
11.50	0.0019	1.0000	3.0000	0.0000
12.00	0.0019	1.0000	3.0000	0.0000
12.50	0.0019	1.0000	3.0000	0.0000

DISTANCE	SOLUTION AT TIME T=			0.00-DAYS			0.00-HOURS			10.00-MINUTES			0.00-SECONDS			HYDRAULIC RADIUS
	FLOW RATE	FLOW DEPTH	SEDIMENT RATE	FROUDE N	BOTTOM ELEVATION	TOP WIDTH	FLOW AREA	FRICITION FACTOR								
0.00	.0127	.0542	0.0000000000	.3620	1.023228	.6000	.0327	14.1519	.0425							
.50	.0127	.0533	.0000002561	.3793	1.023115	.6000	.0322	14.4847	.0419							
1.00	.0127	.0525	.0000004649	.3976	1.022004	.6000	.0316	14.6304	.0413							
1.50	.0127	.0525	.0000006633	.3972	1.022004	.6000	.0317	14.6233	.0414							
2.00	.0127	.0525	.0000006614	.3963	1.021005	.6000	.0317	14.8151	.0414							
2.50	.0127	.0526	.0000004593	.3952	1.020006	.6000	.0317	14.8055	.0414							
3.00	.0127	.0526	.0000004567	.3957	1.019007	.6000	.0317	14.7945	.0414							
3.50	.0127	.0526	.0000004536	.3950	1.018008	.6000	.0317	14.7817	.0414							
4.00	.0127	.0527	.0000004501	.3942	1.017010	.6000	.0318	14.7670	.0415							
4.50	.0127	.0527	.0000004459	.3933	1.016011	.6000	.0318	14.7498	.0415							
5.00	.0127	.0523	.0000004411	.3922	1.015013	.6000	.0318	14.7300	.0416							
5.50	.0127	.0529	.0000004355	.3910	1.014015	.6000	.0319	14.7071	.0416							
6.00	.0127	.0529	.0000004290	.3896	1.013016	.6000	.0319	14.6805	.0416							
6.50	.0127	.0530	.0000004214	.3879	1.012020	.6000	.0320	14.6497	.0417							
7.00	.0127	.0531	.0000004127	.3861	1.011023	.6000	.0320	14.6140	.0418							
7.50	.0127	.0532	.0000004028	.3833	1.010027	.6000	.0321	14.5727	.0418							
8.00	.0127	.0533	.0000003913	.3814	1.009031	.6000	.0322	14.5250	.0419							
8.50	.0127	.0535	.0000003792	.3785	1.008035	.6000	.0322	14.4697	.0420							
9.00	.0127	.0537	.0000003633	.3751	1.007039	.6000	.0324	14.4055	.0422							
9.50	.0127	.0539	.0000003465	.3713	1.006045	.6000	.0325	14.3322	.0423							
10.00	.0127	.0541	.0000003275	.3669	1.005050	.6000	.0326	14.2472	.0424							
10.50	.0127	.0544	.0000003062	.3613	1.004056	.6000	.0328	14.1493	.0426							
11.00	.0127	.0547	.0000002827	.3562	1.003061	.6000	.0330	14.0366	.0428							
11.50	.0127	.0550	.0000002567	.3496	1.002067	.6000	.0332	13.9072	.0431							
12.00	.0127	.0554	.0000002285	.3422	1.001073	.6000	.0334	13.7569	.0434							
12.50	.0127	.0559	.0000001984	.3333	1.000075	.6000	.0337	13.5886	.0437							

AVERAGE ENERGY SLOPE OF THE REACH= .00181

AVERAGE SLOPE OF BED= .00155

AVERAGE SEDIMENT TRANSPORT RATE = .00000003671



SOLUTION AT TIME T=		0.00DAYS	0.00HOURS	20.00MINUTES	0.00SECONDS					
DISTANCE	FLOW RATE	FLOW DEPTH	SEDIIMENT RATE	FROUDE N	BOTTOM ELEVATION	TOP WIDTH	FLOW AK-A	FRICITION FACTOR	HYDRAULIC RADIUS	
0.00	.0127	.0569	0.0000000000	.3153	1.022159	.6000	.0343	12.9076	.0444	
.50	.0127	.0562	.0000000316	.3260	1.022110	.6000	.0339	13.1257	.0439	
1.00	.0127	.0547	.0000002119	.3534	1.022536	.6000	.0329	13.6665	.0426	
1.50	.0127	.0542	.0000003340	.3622	1.022011	.6000	.0327	13.8355	.0425	
2.00	.0127	.0542	.0000003330	.3621	1.021013	.6000	.0327	13.8332	.0425	
2.50	.0127	.0542	.0000003319	.3619	1.020015	.6000	.0327	13.8304	.0425	
3.00	.0127	.0542	.0000003307	.3619	1.019017	.6000	.0327	13.8271	.0425	
3.50	.0127	.0542	.0000003292	.3616	1.018020	.6000	.0327	13.8231	.0425	
4.00	.0127	.0542	.0000003276	.3613	1.017024	.6000	.0327	13.8183	.0425	
4.50	.0127	.0542	.0000003257	.3610	1.016027	.6000	.0327	13.8126	.0425	
5.00	.0127	.0542	.0000003234	.3607	1.015032	.6000	.0327	13.8056	.0425	
5.50	.0127	.0542	.0000003202	.3602	1.014037	.6000	.0327	13.7972	.0425	
6.00	.0127	.0542	.0000003177	.3597	1.013043	.6000	.0327	13.7871	.0426	
6.50	.0127	.0543	.0000003141	.3590	1.012050	.6000	.0327	13.7748	.0426	
7.00	.0127	.0543	.0000003098	.3583	1.011056	.6000	.0327	13.7599	.0426	
7.50	.0127	.0544	.0000003048	.3573	1.010067	.6000	.0328	13.7418	.0426	
8.00	.0127	.0544	.0000002999	.3562	1.009077	.6000	.0328	13.7200	.0427	
8.50	.0127	.0545	.0000002919	.3548	1.008088	.6000	.0328	13.6937	.0428	
9.00	.0127	.0546	.0000002838	.3532	1.007100	.6000	.0329	13.6622	.0428	
9.50	.0127	.0547	.0000002744	.3512	1.006114	.6000	.0329	13.6244	.0429	
10.00	.0127	.0543	.0000002637	.3483	1.005126	.6000	.0330	13.5797	.0430	
10.50	.0127	.0549	.0000002514	.3462	1.004143	.6000	.0331	13.5269	.0431	
11.00	.0127	.0551	.0000002377	.3431	1.003154	.6000	.0332	13.4655	.0433	
11.50	.0127	.0553	.0000002226	.3395	1.002173	.6000	.0333	13.3945	.0434	
12.00	.0127	.0555	.0000002063	.3354	1.001187	.6000	.0335	13.3136	.0436	
12.50	.0127	.0556	.0000001891	.3307	1.000193	.6000	.0336	13.2204	.0436	

AVERAGE ENERGY SLOPE OF THE REACH= .00133

AVERAGE SLOPE OF BED= .00150

AVERAGE SEDIMENT TRANSPORT RATE = .00000002691

NEW SEDIMENT CHARACTERISTICS

RANGE	PERCENTAGES OF GRAIN SIZES 1 THRU 10 RESPECTIVELY	D35	065	BED DEPTH	ABL								
0.00	5.104	7.230	15.306	23.846	25.110	20.544	2.752	0.000	0.000	0.01238	.002504	.997	.00548
.50	5.351	8.393	17.337	24.520	22.894	18.142	2.453	0.000	0.000	.001058	.002610	.998	.00475
1.00	7.095	9.355	20.000	24.850	20.060	15.371	2.110	0.000	0.000	.000930	.002222	1.000	.00415
1.50	7.492	10.483	20.310	24.355	19.023	15.020	2.003	0.000	0.000	.000977	.002076	1.000	.00415
2.00	7.490	10.487	20.310	24.355	19.023	15.023	2.003	0.000	0.000	.000878	.002078	1.000	.00415
2.50	7.489	10.484	20.372	24.993	13.031	15.027	2.004	0.000	0.000	.000878	.002079	1.000	.00415
3.00	7.487	10.482	20.933	24.952	19.036	15.031	2.004	0.000	0.000	.000378	.002080	1.000	.00415
3.50	7.484	10.479	20.932	24.951	19.042	15.037	2.005	0.000	0.000	.000878	.002080	1.000	.00415
4.00	7.482	10.475	20.936	24.959	1.043	15.043	2.006	0.000	0.000	.000879	.002082	1.000	.00415
4.50	7.479	10.471	20.949	24.988	19.058	15.050	2.007	0.000	0.000	.000879	.002083	1.000	.00416
5.00	7.475	10.466	20.930	24.956	15.067	15.059	2.003	0.000	0.000	.000879	.002084	1.000	.00416
5.50	7.471	10.461	20.931	24.964	13.073	15.067	2.009	0.000	0.000	.000880	.002086	1.000	.00416
6.00	7.466	10.454	20.913	24.982	13.091	15.077	2.910	0.000	0.000	.000881	.002087	1.000	.00416
6.50	7.461	10.447	20.906	24.980	19.105	15.089	2.012	0.000	0.000	.000881	.002089	1.000	.00416
7.00	7.452	10.438	20.911	24.976	19.121	15.103	2.014	0.000	0.000	.000882	.002092	1.000	.00416
7.50	7.447	10.429	20.975	24.976	19.140	15.113	2.016	0.000	0.000	.000883	.002094	1.000	.00417
8.00	7.439	10.417	20.855	24.975	19.161	15.134	2.018	0.000	0.000	.000884	.002097	1.000	.00417
8.50	7.430	10.405	20.834	24.974	19.184	15.153	2.020	0.000	0.000	.000885	.002100	1.000	.00417
9.00	7.420	10.391	20.810	24.975	19.209	15.172	2.023	0.000	0.000	.000886	.002104	1.000	.00417
9.50	7.408	10.375	20.734	24.977	19.236	15.193	2.025	0.000	0.000	.000886	.002108	1.000	.00418
10.00	7.356	10.359	20.726	24.981	19.265	15.215	2.029	0.000	0.000	.000889	.002112	1.000	.00418
10.50	7.383	10.341	20.726	24.981	19.265	15.215	2.029	0.000	0.000	.000891	.002115	1.000	.00418
11.00	7.365	10.322	20.696	25.017	19.321	15.258	2.034	0.000	0.000	.000893	.002119	1.000	.00418
11.50	7.355	10.303	20.668	25.017	19.367	15.276	2.037	0.000	0.000	.000895	.002123	1.000	.00419
12.00	7.341	10.285	20.638	25.035	19.367	15.291	2.039	0.000	0.000	.000896	.002125	1.000	.00419
12.50	7.331	10.274	20.624	25.066	19.370	15.293	2.035	0.000	0.000	.000897	.002126	1.000	.00419

RANGE	THE BED IS COVERED WITH DUNES	CF	TYPE	CONST	M	N	RATIO QSS/QST
0.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
1.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
1.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
2.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
2.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
3.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
3.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
4.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
4.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
5.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
5.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0013	1.0000	3.0000	0.0000
6.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0013	1.0000	3.0000	0.0000
6.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
7.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
7.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
8.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
8.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0013	1.0000	3.0000	0.0000
9.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
9.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
10.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000
10.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0013	1.0000	3.0000	0.0000
11.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0013	1.0000	3.0000	0.0000
11.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0013	1.0000	3.0000	0.0000
12.00	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0013	1.0000	3.0000	0.0000
12.50	THE BED IS COVERED WITH DUNES	OF	TYPE 1	.0019	1.0000	3.0000	0.0000

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