### LAKE DIEFENBAKER, SASKATCHEWAN:

### A CASE STUDY OF RESERVOIR SEDIMENTATION

by

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#### Executive Summary

In 1965 a field survey program was implemented to monitor general sedimentation patterns, delta formation and bank erosion in what was to become Lake Diefenbaker. The program consisted of establishing 38 range lines (cross sections), spaced so that they would be representative of changes and surveying them on a regular basis. A sampling program was included to collect information on suspended sediment, bed material and water temperatures. The field programs were conducted jointly by the Sediment Survey Section (Ottawa), Water Survey of Canada (Regina) of Environment Canada and the Hydrology Division, Saskatchewan Environment.

After fifteen years of data collection it was agreed that this program should be reviewed. The review focussed on two principal areas:

- To compile, analyze and interpret all the data collected from Lake Diefenbaker and document the sedimentation patterns;
- To evaluate the existing data collection program and identify changes required to ensure an effective and efficient program.

Sedimentation has not significantly affected the reservoir operation, since only 1.4% of the total reservoir capacity has been lost between 1966-1980. The area where sedimentation is significant is in the drawdown reach, which lost 19% of its volume, or a 1% loss in live storage. Bank erosion and slumping are active and estimated to have accounted for the other 0.5% of the total 1.5% loss in live storage. The dead storage was computed to have been reduced by 1.3% over this period.

Delta formation was determined to be affected by a low original bed slope, large water level drawdown and reduced sediment loadings in the latter years. The bed slopes indicate that the delta structure has yet to reach equilibrium.

Bed material sampling showed that deposited densities were considerably lower than empirically derived values, which greatly affected the deposited load estimates. Based on the sampling findings, a good agreement between station derived loads and the calculated deposited loads was obtained.

The following recommendations are made based on the findings of this report:

1. This joint program should continue, for it is providing a worthwhile contribution to understanding delta formation and

reservoir sedimentation in the prairies. The information is proving to be not only of site specific importance, but transferable to other reservoir studies. It also provides the opportunity to train and to build upon the expertise already acquired in undertaking sedimentation studies.

- 2. Due to the low sedimentation rate, the survey interval should be extended from three years to at least five years, the five year interval will insure continuity between surveys. Since most of the deposition is occurring upstream of the Saskatchewan Landing Bridge, and should likely be the case for quite some time, only those upstream ranges should be resurveyed in the future. Special area studies (Elbow, Swift Current Creek Delta) do not have to be resurveyed as they show negligible sedimentation.
- 3. Certain data collection methodologies (HYDAC-200 data, cross section vs. area method of surveying) need to be further evaluated and more comprehensive guidelines established for sampling (bed material, suspended sediment) in reservoirs.
- 4. Since the existing range data were not the most suitable data for evaluating the bank contribution; and since bank erosion and slumping are active, an alternative study using either terrestrial photogrammetry or concentrated cross-sectional work should be considered.
- 5. Numerical modelling of sediment deposition (such as HEC-6, MOBEO) was not in the scope of this report, but it should be considered the next logical step. If undertaken prior to the next survey it may help further streamline the program.

#### <u>Acknowledgements</u>

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#### 1. Introduction

### 1.1 Design and Construction of Lake Diefenbaker

A Royal Commission (1952) was established by the Government of Canada and presented a report in 1952 on the possibilities of impounding the South Saskatchewan River. By 1958, an agreement had been reached between the governments of Canada and Saskatchewan which outlined the basis on which the South Saskatchewan River Project would be planned, constructed and financed.

The design of the project was undertaken by the federal agency, Prairie Farm Rehabilitation Administration (PFRA). Numerous sites were then examined to determine the most suitable location to construct the dam. These early investigations raised concern over the weakness of the geologically young Bearpaw shale, which was the major underlying formation. However, soil tests and an analysis of the geology (Pollock, 1961) of the area provided the information needed in helping to choose the best site location. A hydrologic investigation was then undertaken to provide the basic information needed to design the outlet works (Berry et al., 1961). Based on the information from these preliminary studies two dams needed to be designed (MacKenzie, 1960). The large earth-filled Gardiner Dam impounded the South Saskatchewan River, while the Qu'Appelle River Dam prevented overflow into the Qu'Appelle River system. A complete detailed description of the design and construction of these dams is available in a PFRA (1980) publication.

The construction work which began in 1959 was completed in 1967 and the South Saskatchewan River Project became officially known as Lake Diefenbaker. The total project cost had amounted to approximately 120 million dollars as Lake Diefenbaker became the largest man-made lake in Canada and the Gardiner Dam was recognized as one of the largest earth-filled dams in the world.

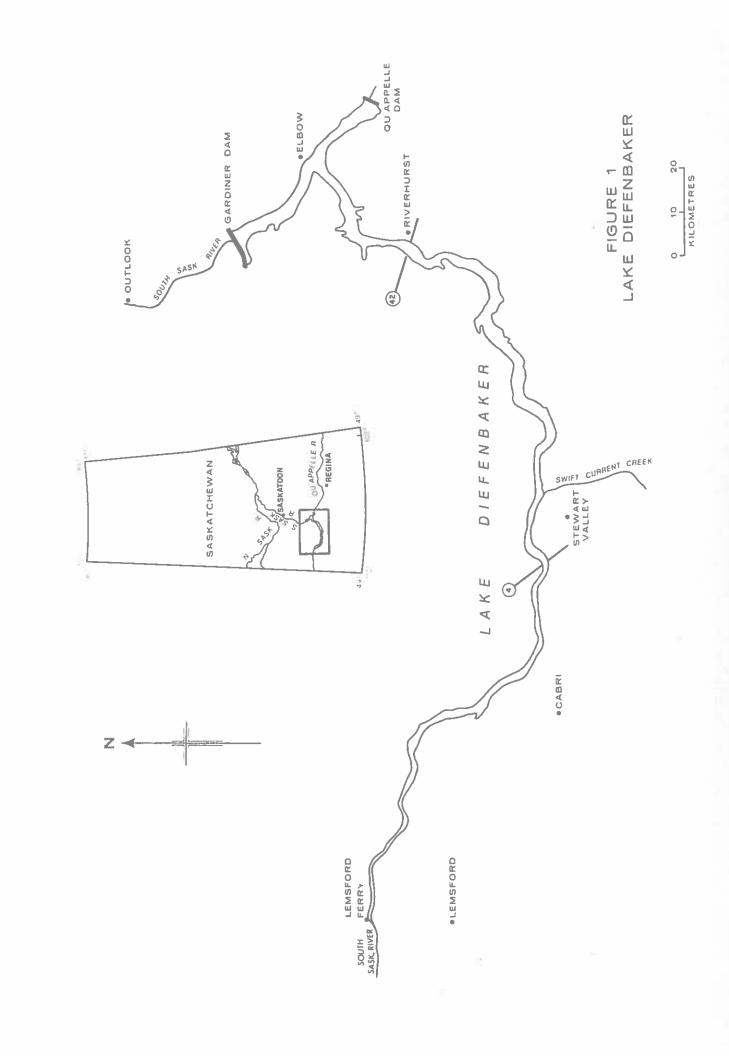
Lake Diefenbaker, which is located in the semi-arid grasslands of southern Saskatchewan (Figure 1), is fed primarily by two tributaries. The major one being the South Saskatchewan River and the other being the smaller Swift Current Creek.

Filling of the reservoir was initiated in 1964, but full supply level (FSL), 556.87 metres above mean sea level, was not reached until 1968. The water level can be drawn down over 11.0 metres to the 545.59 metre elevation. The 225 km-long Lake Diefenbaker has an estimated 9.4 million cubic decametre (dam<sup>3</sup>) storage, of which 4.0 million dam<sup>3</sup> is useable or live storage.

The reservoir effectively impounds the South Saskatchewan River while serving a number of purposes: municipal and industrial use, power generation, irrigation, recreation and flood control.

### 1.2 <u>Pre-reservoir Sedimentation Studies</u>

In 1947, PFRA was concerned that sedimentation of the proposed reservoir could be a problem so a hydrometric station was established at



Outlook, to collect data on sediment transported by the South Saskatchewan River. In 1952, computations revealed that the average annual sediment load would only reduce the proposed total capacity by less than 0.1% per year (PFRA, 1952). Since the value was so low no further work was devoted to sedimentation studies.

In 1959, another consultant was hired to formulate a master plan for the recreational development and utilization of the reservoir. His report identified three sources for concern, which could greatly affect recreational development and for which limited or no data were available. The areas of concern were: deposition of material from eroding banks; sediment transported into the reservoir by Swift Current Creek; and the extent of deposition in the reservoir from the South Saskatchewan River sediment loads (Baker, 1960). This study helped point out the complexities involved in the prediction of sedimentation in a proposed reservoir, and provided the impetus for further studies.

To assess the extent of bank erosion, a specialist in airphoto interpretation and geology was consulted. The basic finding of his study was that the reservoir banks, which were comprised of glacial till and sand, would be highly susceptible to erosion (Mollard, 1961). The report included a topographic map that depicted areas along the proposed shoreline that would be prone to slumping and erosion. However, no quantitative estimates were made because there were insufficient data from which to make such an evaluation. The report stated that to assess the problem properly a study would have to be conducted after the reservoir had been in operation for a few years.

Kuiper (1962) evaluated the processes of sedimentation and forecasted delta formation in the proposed reservoir. Based on suspended sediment data collected at the Outlook station, Kuiper estimated the average annual sediment load to be in the order of 7 400 dam<sup>3</sup>. This provided for a sedimentation rate of only 0.1% yearly, based on the total storage capacity. The report projected the longitudinal profiles of the reservoir after 10, 50 and 100 years, based on the incoming sediment load. This study also had limited data with which to work and necessitated the use of data from existing reservoirs. From this study it became apparent that to effectively monitor and forecast future sedimentation in the reservoir, an ongoing hydrographic data collection program should be implemented.

#### 1.3 Objectives of Reservoir Survey Program

These early studies made it apparent that, while the magnitude of sediment inflows to the reservoir was not a problem, the distribution of the sediment load could pose problems. Even with limited data these studies did provide some valuable insight into sedimentation in what was to be Lake Diefenbaker. However, these studies did make it evident that the processes affecting sedimentation were very complex and that a data collection program would help provide more information on how these processes would interact over time.

So in 1963, the Saskatchewan government implemented a standard reservoir program consisting of profiling established cross sections. This program was established to monitor the progress and configuration of the developing delta, and determine the sedimentation rate in the lake. Due to the fact that the project provided a unique opportunity to study

sedimentation in a newly formed reservoir of such magnitude, and the significance of the reservoir itself, the federal government established the program as an International Hydrological Decade Project in 1964. The project was entitled, "Delta Formation and Sedimentation in Lake Diefenbaker" and from that point on a joint program of sediment and hydrographic data collection was undertaken.

The specific objectives of the reservoir program were defined in the "IHD Project Saskatchewan-2 Progress Report 1964-67" as:

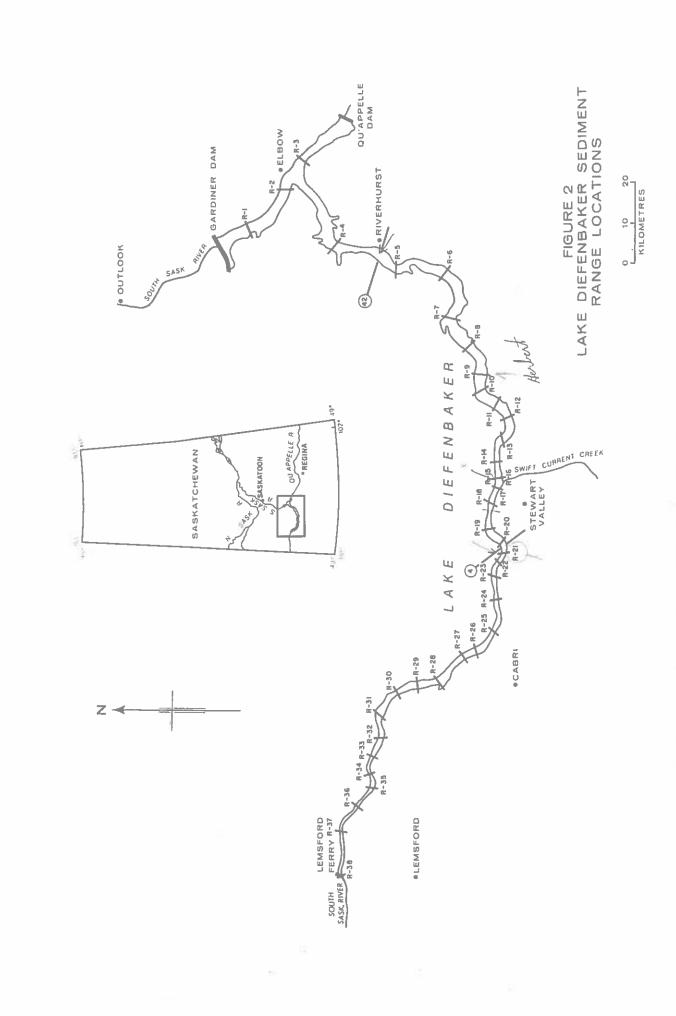
- "1. to provide information that will aid in planning future improvement and developments at Lake Diefenbaker;
- 2. to provide information relative to the effect of aggradation on structures as the Saskatchewan Landing Bridge, ferry approaches, water supply intake works, and recreational and other developments;
- 3. to provide information for operational purposes, such as the effect of sediment accumulation on the reservoir capacity relationship and evaporation losses;
- 4. to provide factual data for possible litigation;
- 5. to provide information that will aid in future planning of other projects."

#### 1.4 <u>Data Collection Programs Involved in the Project</u>

The program involved the permanent establishment of 38 cross sections, more commonly referred to as ranges, spaced in an ascending numerical order from Gardiner Dam to Lemsford Ferry (Figure 2). The location of the ranges were chosen based on the following criteria: that they were representative of the geometry of the reservoir in a particular reach, that they were easily accessible, and that they would reflect anticipated deposition or erosion. Each range was then to be surveyed for cross-sectional area changes at a predetermined interval, so as to monitor the rates of deposition and/or erosion in the reservoir.

Suspended sediment samples were collected at each range to provide information on the sediment transported by incoming flows. This portion of the program was also initiated to help determine if density currents existed as a mode of transport within the reservoir. Temperature data were obtained to determine if stratification of the water existed in the lake and how it might affect sediment transport and deposition of suspended sediment. Bed material sampling provided information on the densities of deposited sediment as well as depositional patterns.

Hydrometric stations where then established by Water Survey of Canada to provide information on the two major tributaries. Data collection at these stations included sediment data, as well as hydrometric data. On the South Saskatchewan River a station was established just upstream of the



reservoir near Lemsford in 1958. In 1965, a station was installed on Swift Current Creek where it drains into Lake Diefenbaker.

The other major source of sediment was determined to originate from the banks of the reservoir, so a program to determine the contribution of this source was implemented. Low level aerial photography was determined to be the best approach to identifying areas undergoing significant bank erosion. At a later date three representative sites were chosen for terrestrial photogrammetry application in an attempt to calculate the volume of bank material being eroded.

#### 1.5 Sedimentation Studies Conducted on Lake Diefenbaker

There have only been a few additional sedimentation studies that have been conducted on Lake Diefenbaker since it was established. The first such study was concerned with the effect of bank erosion on the different storage capacities. Van Everdingen (1968) taking into account the water level drawdown, applied soil and slope stability theories, calculated the extent to which the banks could be eroded. Based on range data from Ranges 4, 5 and 6, it was calculated that the live storage could be increased by as much as 7.4% due to bank erosion. This bank material would then be transported down into the dead storage zone, which would decrease its capacity by 4.0%. These changes were expected to occur within the first ten years of the life of the reservoir. It should be noted that the fact that this area was considered to be indicative of the whole shoreline can be considered questionable assumption. The reach for this study has particularly steep banks which would produce a high, overestimated, rate of bank erosion if applied to the whole reservoir.

In 1969, further research in bank erosion and nearshore sedimentation was undertaken in the vicinity of Elbow. Coakley and Hamblin (1969) after a brief field trip provided some general statements concerning the processes involved. Their findings were that most of the erosion was due to gravity slumping of structurally weakened banks and that most of the material was being transported towards the central portion of the reservoir. Measured nearshore currents were found to be too weak to transport sediment along the reservoir and instead sediment was transported from the banks, laterally, down the steep bank gradients. This also explained why no shoreline depositional features were observed and the general lack of beach formation. No estimates concerning the extent of bank erosion or the rate of sedimentation were made from the study.

An analysis of the hydrographic data collected between 1965 and 1972 on Lake Diefenbaker was conducted by Wiebe and Drennan (1973). The analysis dealt primarily with changes to the longitudinal profile of the reservoir with time. Based on changes in the mean bed elevation, deposition was calculated to have occurred from Range 30 to Range 4. Analysis of the suspended sediment data revealed that 85% of suspended load was deposited by Range 21. The sand portion of the suspended sediment load was found to have settled out of suspension by Range 30. Density of deposited material varied from a high of 1 490 kg/m<sup>3</sup> at Range 33, to a low of 240 kg/m<sup>3</sup> by Range 9. Sand was found to be deposited within the old river channel, while silt and clay were deposited relatively uniformly throughout the cross sections in the upper ranges.

Rasid's (1979) study dealt primarily with the effects that the reservoir would have on the downstream regime, but did discuss sedimentation in the reservoir. An analysis of pre-dam to post-dam sediment loads at Lemsford and Saskatoon revealed that about 97% of the sediment load carried by the South Saskatchewan River was being trapped in Lake Diefenbaker. However considering the length of Lake Diefenbaker, it is highly unlikely that any of the sediment load carried by the South Saskatchewan River passes through the reservoir. The load differences measured in this case can probably be attributed to sediment eroded from the banks and bed between Gardiner Dam and Saskatoon.

## 2. Reservoir Data Collection: Methodology, Techniques and Equipment

#### 2.1 Cross Section (Range Line) Surveys

Through an open control system of surveying, permanent bench marks were located for each of the 38 ranges. A total of four bench marks were installed at each range, two on each bank. The upper bench marks, one located on each side of the reservoir, were to be located so that they would not be affected by unstable ground conditions brought about by the newly established reservoir. The lower bench marks on both sides were located closer to the reservoir shoreline, to minimize the land surveying involved each time the cross sections were surveyed. All the bench marks were then tied into geodetic datum by first order level circuits. The distance across the reservoir, between lower bench marks, was measured by a Tellurometer, an electronic distance measuring device, and a station was assigned to the bench marks. The convention used was that all lower left bench marks were assigned a horizontal station of zero, while the station of the lower right bench marks corresponded to the distance between the two lower bench marks. Bench marks were checked and maintained on an annual basis by personnel from the Hydrology Branch, Saskatchewan Environment.

portion of the cross section by means of a surveyor's chain and level. A point was chosen whenever there was a break in slope or the end of the tape was reached. Proceeding from the bench mark to the water's edge the station and elevation of each point were recorded. This procedure was conducted on

both banks at each range. The water level elevations obtained from each side were then checked to make sure they compared to the nearest tenth of a foot. In some cases a discrepancy could be explained, such as effects from wind build up, but if there were no logical reasons, then the levelling work was redone.

Once the water level elevation had been obtained, the hydrographic data were collected using an echo sounder mounted in a boat. A continuous profile was obtained along the range line. The water depths were extracted whenever there was a break in profile and converted to bed elevations by substracting them from the known water level elevation.

To keep the boat on line, two methods or aids were employed. The first involved the installation of two highly visible range markers on each bank, this provided the visual assistance for the operator to keep the boat on line. The second was to set up a transit on the bench mark and sighting it on the bench mark across the reservoir, if the boat drifted off the line then the instrument man would give directions to the boat operator by means of a two-way radio.

The horizontal distance the boat travelled was determined by triangulation. A measured baseline was laid out perpendicular to the range line, with a transit situated at the end of the baseline. As the boat moved across the reservoir, a signal was given periodically from the boat, an angle was read and recorded, while simultaneously in the boat the echo sounding chart was marked. Another method used occasionally in the early surveys involved the use of a rangefinder. The rangefinder was set up on the range

line and sightings made on the boat as the echo sounder chart was marked.

However, the rangefinder was only calibrated for sighting distances greater than 300 m, which restricted its application at many ranges.

In the upper ranges, where river conditions prevail, different techniques and equipment had to be applied. In these cases a tag line was stretched across the channel and a sounding rod used from a boat or by wading to measure depths at fixed intervals.

In 1970, some testing was undertaken to determine the reproducibility of profiles (IHD Annual Project Report, 1970). Based on different triangulation arrangements, the maximum distance measurement discrepancy was calculated to be 20 m. Boat drifting, and to a lesser extent, instrument readings were found to affect the results. The depth data were found to be precise, as long as the echo sounder had been properly calibrated for temperature and salinity. However, it was determined that if the recorder was not adjusted when there was a change in these conditions, then erroneous readings would be obtained. In fact, most of the 1970 hydrographic data collected from the reservoir was determined to be inaccurate because the echo sounder had not been properly calibrated.

In 1971, a distance positioning system was adopted which consisted of a master and slave Tellurometer, a printer and a synchronizing component was used to measure distances. The slave, which was set up at known position on the range line, would transmit to the master unit in the boat. At a push of a button the distance and depth were recorded simultaneously. The

Tellurometer, which was accurate to  $\pm 1.5$  m proved to be more reliable than the triangulation method for obtaining horizontal distances.

Table 1 indicates the profiles that have been obtained since the beginning of the project. In 1974, the sector method of surveying was incorporated into the program and areas were surveyed instead of cross sections from Range 1 to Range 27. The ranges upstream of Range 27 continued to be surveyed as range lines because they were not easily accessible by the larger boat.

#### 2.2 Area (Sector) Surveys

In 1972, an automated hydrographic system, HYDAC-100, was developed and tested in the reach where Swift Current Creek drains into Lake Diefenbaker. The test results were favorable so the system was adopted for use on Lake Diefenbaker surveys. Instead of collecting a single line of data at each range, as was done before, a collection of lines transecting an area were collected. The area or sector method of surveying at each range was then incorporated into the program in 1974. A detailed description of this system and the survey practices used are available in a technical bulletin by Durette and Zrymiak (1978). In 1978 the system was updated because of technological advances and became known as the HYDAC-200. Since many changes have been made to the prototype, a brief description of the system has been provided in this report.

TABLE 1
CROSS SECTION (RANGE LINE) DATA AVAILABLE

E .	YEAR															
RANGE NO.	1965	1966	1967	1968	1969	1970	1971	1972	1973	1974	1975	1976	1977	1978	1979	1980
1	Х		X	Х			Х			Х						х
2	X				Х		Х							Х		
3	Х						Х									Х
4	Х		Х	Х			Х			Х				Х		Х
5	Х						Х									Х
6	Х		Х	Х				Х			Х					
7		Х					Х							X.		
8		Х			Х		Х						Х			
9			Х					Х			Х					х
10			Х	1			х									
11		Х	Х					Х			Х					
12		Х					х									х
13		Х					Х						Х			Х
14		Х						Х			Х					
15		Х					Х						Х			
16		Х					Х						Х			
17		X	Х								Х			Х		
18		Х					X			Х			Х			Х
19		Х					X						Х			Х
20		X						X			Х			Х		
21		X	Х					Х			Х			Х		
22		Х					Х									Х
23		Х		Х				X			Х			Х		
24		Х						Х			Х					
25		X					Х						Х			
26		X		Х				Х			Х			Х		
27		Х			X			Х			Х			X		
28		Х		Х			Х					1 1	Х	1 -1-		Х
29		Х	_ X		Х									Х		
30		Х	X.			Х		Х			Х	,		Х		
31		Х	X		Х		Х									Х
32		Х	_X			Х		Х			X			Х		Х
33		Х		X				Х			Х			Х		Х
34		Х					Х						Х			Х
35		Х			Х			х			х			Х		X
36		Х						Х			Х	]		Х		Х
37		Х				Х		X			Х			Х		Х
38		X						Х	]	]	Х			X		Х

NOTE: X - CROSS-SECTIONAL DATA

The data collection system shown in the block diagram (Figure 3) can be divided into four sub-systems: positioning, depth sounding, data processing with survey control and data logging. The working relationship of the individual components has also been outlined in the diagram.

The dynamic positioning of the 10 metre-long vessel is accomplished by means of two Tellurometers positioned on shore. The remote Tellurometer locations are fixed by tying them into the network of established survey bench marks. The master Tellurometer on the boat measures the distance to the remote unit under dynamic conditions to a single range accuracy of  $\pm 1.0$  m. This positioning system was modified in 1978 with the installation of the most currently developed MRD 1 Tellurometers. The original MRB 201 Tellurometers were less reliable and had an accuracy of only  $\pm 1.5$  m.

The depth sounding sub-system is considered to be accurate to within 50 mm, up to a depth of 100 m. There are two transducers. The 210 kHz frequency detects low density deposits, while the 33 kHz distinguishes material of higher densities. This particular sub-system has not undergone any component changes since it was originally installed.

The sub-system concerned with data processing and survey control monitors the measuring instruments. The quality of data collection can be controlled through the choice of data acquisition rates and coverage density. This is all accomplished by a software package, consisting of four main programs: a driver; a preplot; an on-line and a data dumping program.

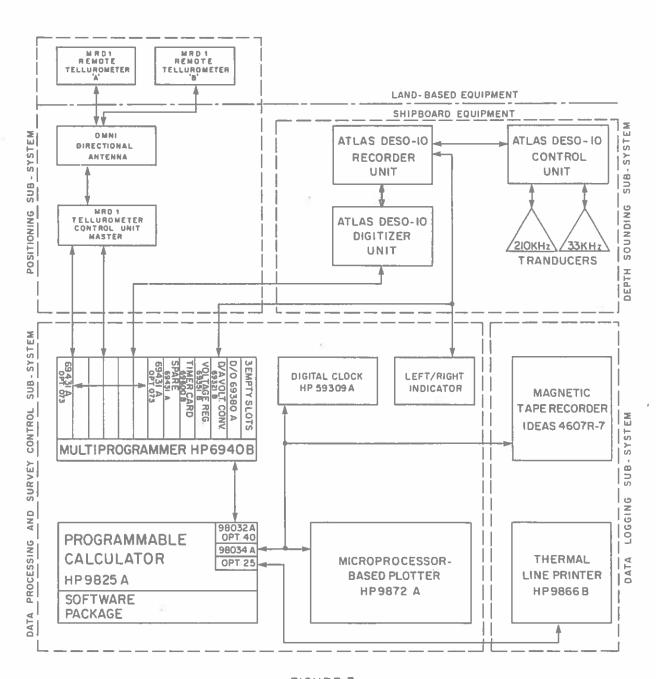


FIGURE 3
HYDAC - 200

A HYDROGRAPHIC DATA ACQUISITION SYSTEM

A point, which is made up of two distances, depth and time, is capable of being collected by the system every two seconds. The addition of an onboard plotter to the system in 1977 greatly improved the coverage, which up until then was not monitored.

The last stage of the data collection system is the storing of the data. The data are stored on two outputs: a seven track magnetic tape and a hardcopy printout. The hardcopy is the backup to the field tape.

The first step in collecting sector data involved land surveying. Profiling was done from the bench mark to the water's edge and the water elevation recorded. The next step was to locate the Tellurometers where they had a good line of sight and reference them to the bench mark network. In most cases a Tellurometer was located on each bank, usually on the range line. This Tellurometer configuration which was used to collect sector data has been found to produce innaccurate range data because of the poor trigonometric arrangement. After the 1978 field season special care was given in locating the Tellurometers to provide accurate range data. The distance between Tellurometers was then measured with an electronic distance measuring device (CA 1000). This distance, which is known as the baseline length, was then fed into the HP 9825A programmable calculator along with a suitable scale. A bearing was then given along with the line interval spacing, and a preplot was produced. On Lake Diefenbaker the line spacing was a standard 50 m apart and approximately 600 m of reservoir length were surveyed for each sector. As a check, the boat transected the baseline and the Tellurometer distances were summed to ensure that the value matched that of the baseline. At this point the driver began surveying by first

collecting data along the old range line, then proceeded into a perimeter run, and the last step was to follow the line spacing to complete the coverage.

The sectors which have been collected since 1974 are shown in Table 2. Cross-sectional data were extracted from most of the sectors as a means of comparing the two survey methods.

In addition to the sector surveys there were also three sites within the reservoir which are significant for studying sedimentation and were incorporated into the program as area surveys. The first included the whole reach from Range 27 to Range 25 as one continuous survey. This reach was considered to be the most indicative in terms of monitoring delta formation in the reservoir. This area was surveyed in 1975 and then again in 1980. The area where Swift Current Creek enters Lake Diefenbaker was surveyed in 1972 and was resurveyed in 1978 for comparison. The purpose for this was to monitor deposition from Swift Current Creek sediment loads. The other area surveyed was at the junction of the two arms, the Gordon Mckenzie and Thompson, near Elbow, and was done in 1974 and has yet to be repeated. This was considered to be an area prone to bank slumping and which could also experience sedimentation that might affect the water intake for the town of Elbow.

To process these data, the Sediment Survey Section developed an automated data reduction and analysis system - HYDRA. This system which

TABLE 2
AREA (SECTORS) DATA AVAILABLE

Щ ·	YEAR															
RANGE NO.	1965	1966	1967	1968	1969	1970	1971	1972	1973	1974	1975	1976	1977	1978	1979	1980
1										S				S		S
2										S				S		S
3										S				S		S
4										S				S		S
5										S				S		S
6											S			S		
7										S				S		
8										S			S			
9											S					S
10													S			
11											S					S
12										S			S			S
13										S			S			S
14											S			S		
15										S			S			S
16										S						S
17											S			S		
18										S			S			S
19										S			S			S
20											S		<u> </u>	S		
21											S			S		
22										S			S			S
23											5			S		
24											S					
25										S			S			
26											S			Ş		
27											S			S		
28																
29																
30																
31																
32																
33										-						
34																
35																
36					_											
37									41							
38																

NOTE: S - SECTORS

South Saskatchewan Delta Area (Range 27 - Range 25) - 1975, 1980 Swift Current Creek Delta Area - 1972, 1978

Elbow Area – 1974

TABLE 3
HYDRA SYSTEM PROCEDURES

PROC.			
NO.	PFN	MEANING	PURPOSE
1.	HD	HYDAC DUMP	To dump and convert the Field Tape Data from ASCII to BCD
2.	TP	TRANS PROCESS	To re-format the DUMPFN Data Records to card images
3.	CS	CONVER SECTOR	To convert the PCATDPFN Data of selected sectors to XYZ Coord.
4.	EP	EDIT POINTPLOT	To produce a TAPE30 Point Plot of the CNTLEDPFN
5.	PP	POINT PLOT	To produce a TAPE30 Point Plot of the CNTLPFN
6.	DR	DUPLICATE REMOVAL	To remove any consecutive duplicate XY Coord. in CNTLPFN
7.	PX	PROFILE	To plot X-section from CNTLPFN
8.	NO	NEWORIGIN	To reset a new origin.
9.	MC	MODULE CREATION	To sort CNTLPFN into modules and select BND for Plotting
10.	MS	MODULE SCREEN	To screen MODPFN so as to reduce linear overkill
11.	CP	CONTOUR PLOT	To produce a contour plot of a MODDECK in the MODPFN
12.	BP	BOUNDARY PLOT	To plot the BND Coord, with ten X-section flags
13.	PC	PROFILE CONTOUR	To plot X-sections from the contour plot
14.	AD	ARRAY DIFFERENCE	To prepare an array of the difference of the overlap of two arrays
15.	AC	ARRAYDIF CONTOUR	To plot a contour map of the array of differences
16.	ВС	BINARY CONTOUR	To plot a contour map using the binary array of contours
17.	NC	NEWBND CONTOUR	To plot a contour map using new BND (Common) Coord. stored on SURIPF
18.	AP	CONTOUR APPLICON	To plot a color contour map using the Applicon Plotter
19.	D3	3 DIMENSION	To plot a three dimension block with BND cut off option
20.	СС	CAPACITY CURVE	To calculate and plot a capacity curve

PFN = Permanent File Name
CNTLPFN = Converted Data (XYZ)
CNTLEDPFN = Data File used to produce
a plot to edit PCATDPFN

MODPFN = CNTL Data split into modules MODDECK = Module Deck Name on MODPFN SURIPF = Survey Indirect Permanent File PCATDPFN = Pre Edited Data from Trans (TP) DUMPPFN = Output Data from Hydac Dump (HD) is comprised of twenty computer routines written in Fortran is outlined in Table 3. The software package used to generate contours is the General Purpose Contour Program (GPCP) which is available at the Computer Science Centre. Department of Energy, Mines and Resources.

Figure 4 illustrates the five basic steps in processing the field data. The first step is to prepare the data into standard formats and to input the necessary information. Each data point is then processed based on the input information and converted to X and Y coordinates and an elevation. The points are then plotted, visually examined and any erroneous data are edited. Once there are no more corrections and the boundary conditions have been set, the data are gridded and run through GPCP to generate contours. For Lake Diefenbaker, the contours were based on a 30 by 30 m array. The capacities were computed by dividing the module into horizontal slices with the thickness of the slice being selectable. Figure 5 illustrates this approach, the mathematical formulation for obtaining the volume of a slice (Equation 1), and subsequently the total capacity (Equation 2).

## 2.3 <u>Suspended Sediment Sampling</u>

After the hydrographic data were collected at each range, suspended sediment samples were collected. Three verticals in each cross section were sampled. They were located at one-sixth, one-half and five-sixths of the distance between the banks. Three samples were obtained from each of the three verticals for a total of nine. In each vertical, a sample was collected near the water surface, one at mid-depth and one near the bed,

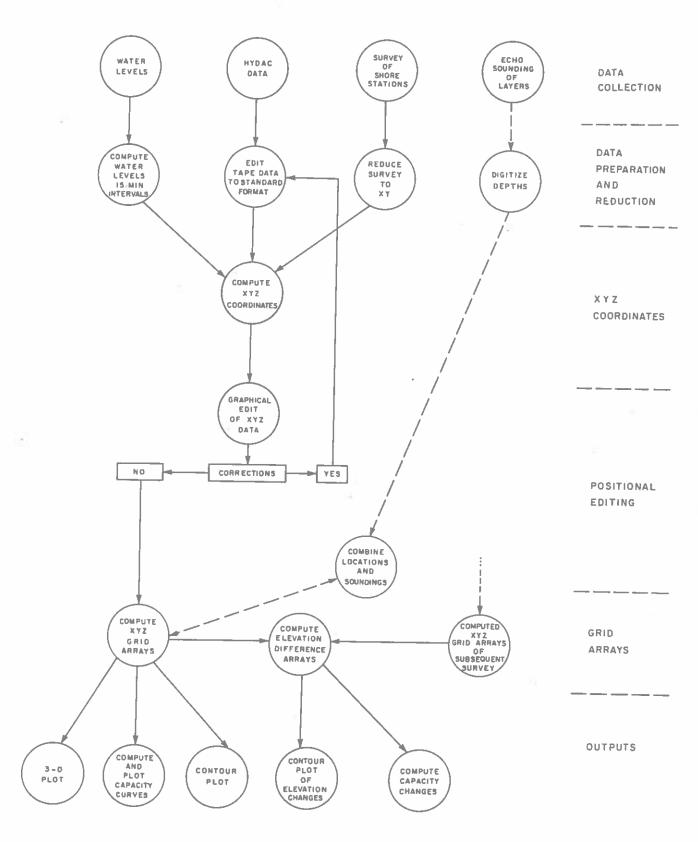


FIGURE 4
HYDRA SYSTEM FLOWCHART
HYDROGRAPHIC DATA REDUCTION AND ANALYSIS

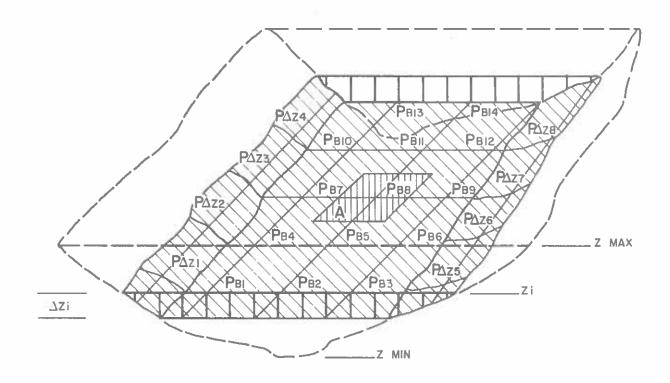


FIGURE 5
ELEVATION-CAPACITY COMPUTATION

$$V\Delta z_{i} = \frac{1}{2} \Delta z_{i} \left( \sum_{Z_{i}-\Delta z_{i}}^{Z_{i}} A P\Delta z_{i} \right) + \Delta z_{i} \left( \sum_{Z \text{ MIN}}^{Z_{i}-\Delta Z_{i}} A PB \right)$$
(1)

and 
$$VTOT = \sum_{Z \text{ MIN}}^{\cdot} V\Delta z_i$$
 (2)

where Vazi — the volume of the horizontal slice under consideration.

 $\Delta z_i$  —elevation increment defined by the user.

A — Area assigned to each elevation point.

Pazi — elevation point residing within the elevation increment under consideration.

PB —elevation points below the elevation  $z_i$ - $\Delta z$ 

VTOT—total volume of water beneath the surveyed area considered.

using an Instantaneous Horizontal suspended sampler. In the upper ranges where there are many channels, a sample was collected from each of them.

These data provided information on the distribution of suspended sediment within a cross section as well as longitudinally in the reservoir. The sediment samples were then analyzed in the laboratory using the evaporation and filtration methods of analysis to determine concentrations; and the bottom withdrawal method to determine particle-size distributions (Guy, 1969).

These sampling procedures, which were implemented in the beginning of the program, have not changed. To collect hydrographic data the most optimum time is when the reservoir is at FSL, therefore ensuring maximum area coverage. However by this time most of the suspended sediment has either settled out of suspension or mixing of the reservoir water has occurred. Since the sediment sampling was undertaken at the same time as the hydrographic surveys it is understandable why this portion of the program has not been very effective. The suspended sediment sampling program was finally discontinued in 1978.

In 1972 sampling was conducted while the reservoir was being filled, and this was the only time suspended sediment samples were of sufficient concentrations to determine particle-size distributions. To obtain particle-size information, concentrations should be in the order of 300 mg/L or have a dry weight of at least 0.5 g.

TABLE 4
SUSPENDED SEDIMENT DATA AVAILABLE

O -		YEAR														
RANGE NO.	1965	1966	1967	1968	1969	1970	1971	1972	1973	1974	1975	1976	1977	1978	1979	1980
1	С	С	С				С			С				C		
2	С	С		С	С		С			С				С		
3		С				C				С				С		
4		Ç	C							С				С		
5		С				С				С				С		
6		С	С								C			С		
7										С				С		
- 8					С					С			С			
9								С			С			С		
10			С				С			С			С			
11		С									С			С		
12					С					С			С			
13				С						С			С			
14								С			С			С		
15							С			С			С			
16										С						
17			С	_				С						С		
18							С				С		С			
19				С					ı.				С			
20					C									С		
21								С			С			С		
22										С			С			
23				С							С			С		
24			С				C				С			С		
25										С			C			
26				С				CP			С			Ċ		
27					C						С			С		
28										C						
29				С	С			CP								
30			С				С	CP								
31					С					С						
32							С									
33								CP								
34										С						
35					С											
36								CP								
37							С									
38																

NOTE: C - CONCENTRATION DATA

P - PARTICLE SIZE DATA

The concentration and particle size data which have been collected over the years are summarized in Table 4.

#### 2.4 Bed Material Sampling

A bed material sample was collected at each of the three verticals used to obtain the suspended sediment samples. In the upper ranges where there are multiple channels, samples were collected in each channel. In most cases samples were collected using a US BM 54 bed material sampler and occasionally the Canadian Drag Bucket (Lane) bed material sampler. As sedimentation occurred, the bed material composition became comprised mostly of loosely deposited silts and clays, making this sampling equipment no longer effective. In 1971, the Phleger 840-A Bottom Corer sampler was introduced into the program for sampling these very low density deposits.

Particle-size distributions were measured by means of sieve analysis for particles larger than 62 mm, and hydrometer analysis for particles less than 62 mm (Guy, 1969). The use of the Phleger Corer made it possible to determine densities. This was done measuring the core length and deriving the volume of the sample. The dry weight of the whole sample when divided by the volume provided the in situ density of the deposited material.

The bed material samples that have been collected are catalogued in Table 5, along with type of information supplied.

TABLE 5
BED MATERIAL DATA AVAILABLE

Щ.								YEA	\R							
RANGE NO.	1965	1966	1967	1968	1969	1970	1971	1972	1973	1974	1975	1976	1977	1978	1979	1980
1		P	Р				Р			PD				p		Ph
2		P		P	P		P			PD				P		PD
3		P				P								P		PD
4		P	P							P				P		
5		P				P				Р				PD		PD
6		P	P								Р			PD		
7							P			P			PD	PD		
8		P			Р			22		P						
9			P			P		PD			Р			PD		
10			P							P			PD			
11		P	P								P					PD
12					P					P			PD			PD
13				Р			Р			Р			PD			PD
14								PD			P			PD		
15										P			PD			PD
16							P			P						PD
17			P					PD			Р			PD		
18						P				P			P			PD
19				P			Р			P			PD			PD
20					P						р			PD		
21			P				35	P			P			PD		
22							P			P			PD			PD
23				P							P			PD		
24						P					P			PD		
25		H)					P			P			PD			
26			P	P				PD			P			PD		
27					P						P			PD		
28			P				Р			P			P			P
29				P	P			P						P		P
30						P		P			P			P		P
31					P		P			P			P			
32			P			Р					P			P		P
33				P							P			P		P
34							P			P			P			P
35					P			P			P			Р		P
36			P								P			P		P
37						P					P					P
38											P					P

NOTE: P - PARTICLE SIZE DATA

D - DENSITY DATA

South Saskatchewan Delta Area (Range 27 - Range 25) - 1975 27 Samples (PD)

- 1980 27 Samples (PD)

Swift Current Creek Delta Area - 1972 37 Samples (PD)

- 1978 13 Samples (PD)

Elbow Area - 1974 31 Samples (PD)

### 2.5 Water Temperature Data Collection

Originally, water temperature data were collected only at the same nine points as the suspended sediment samples. In 1968, temperature readings were obtained at 1.5 m intervals from the surface to a depth of approximately 18 m, below that depth was every 3.0 m. An electronic thermometer, tied to a 60 m cable, was used for data collection. In 1974 the thermometer malfunctioned part way through the field season and temperature data ceased to be collected. Table 6 depicts when and where temperature data had been collected on take Diefenbaker.

Data on surface water temperatures and evaporation rates have been part of an ongoing program with the Atmospheric Environment Service, Environment Canada since 1972 (Environment Canada, 1977). Four sites on the reservoir have been involved in this data collection program.

TABLE 6
TEMPERATURE DATA AVAILABLE

B ·		YEAR														
RANGE NO.	1965	1966	1967	1968	1969	1970	1971	1972	1973	1974	1975	1976	1977	1978	1979	1980
1		Т	Т				Т									
2		T		T	T		T									
3		T				T										
4		T	T													
5		T				Т										
6		T	T													
7							T									
8					T											
9								T								
10			Т			T										
11		Т														
12					T											
13				Т			T	T						55	72	
14																
15						T				Т						
16										T						
17			Т					T								
18						T				T						
19				T			T			T						
20					T											
21								T								
22							Т			T						
23				T												
24			T			Т										- 1
25							T									
26				Т				T								
27					T											
28							T									
29				T	T											
30			T			T										
31					T											
32						Т										
33				T			T									
34							T									
35																
36					T											
37						T			-							
38																

NOTE: T - TEMPERATURE DATA

#### 3. South Saskatchewan River and Swift Current Creek

#### 3.1 <u>Hydrometric Stations</u>

As mentioned earlier there are two tributaries that drain into Lake Diefenbaker, the major one being the South Saskatchewan River, and the other one being Swift Current Creek. Flow and sediment data are available from hydrometric stations that have been established on each of the streams (Table 7).

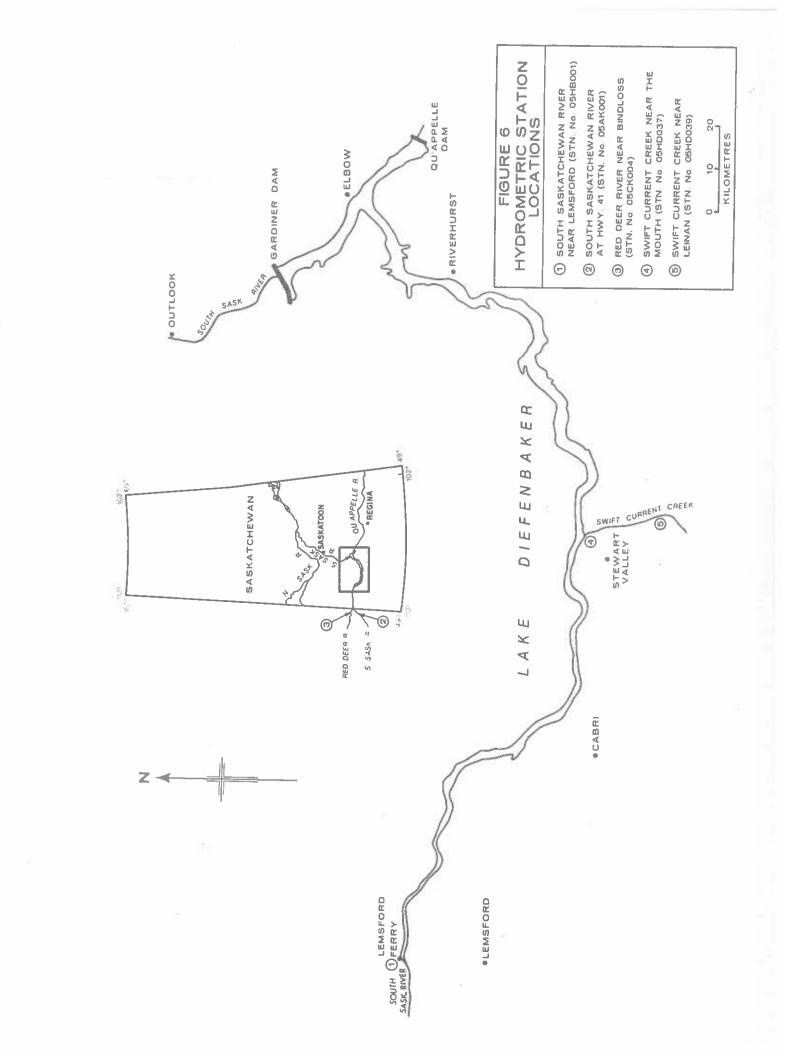
A hydrometric station was established on the South Saskatchewan River at Lemsford Ferry (STN. NO. 05HB001) in 1958, to collect discharge data, and by 1961 sediment data were being collected at the station as well (Figure 6). However, the station was discontinued in 1970 due to maintenance problems and also because there was concern about the accuracy of data due to backwater effects. A station located further upstream near Hwy 41 (STN. No. 05AK001) and one on the Red Deer near Bindloss (STN. No. 05CK004) provided continuing data after the Lemsford station was closed (Figure 6).

In 1965, a station was set up near the mouth of Swift Current Creek (STN. NO. 05HD037). This station after eight years of operation was moved, due to ice associated problems, to a more ideal location further upstream near Leinan (STN. No. 05HD039) in 1973 (Figure 6), ensuring an unbroken record of data.

TABLE 7
Hydrometric and Sediment Station Data Available

Station	Hydrometric Data Years of Record	
South Saskatchewan River at Hwy. 41 (STN. No. 05AK001)	1966-*	1966-*
South Saskatchewan River near Lemsford (STN. No. 05HB001)	1959-1970, 1975	1961-1970, 1975
South Saskatchewan River near Outlook (STN. No. 05HF001)	1947-1966	1948-1961
South Saskatchewan River at Saskatoon (STN. No. 05HG001)	1911-*	1961-1971
Red Deer River near Bindloss (STN. No. 05CK004)	1960-*	1966-*
Swift Current Creek near the Mouth (STN. No. 05HD037)	1965-1972	1965-1972
Swift Current Creek at Leinan (STN. No. 05HD039)	1973-1981	1973-1981
Lake Diefenbaker at Gardiner Dam (STN. No. 05HF003)	1966-*W	
Lake Diefenbaker at Saskatchewan Landing (STN. No. 05HC004)	1968-*W	

<sup>\* -</sup> Still Operating W - Water Levels Only



#### 3.2 <u>South Saskatchewan River Hydrology</u>

The South Saskatchewan River drains an area of approximately 120 000 km<sup>2</sup> as it enters Lake Diefenbaker. In terms of the contribution to the flow, this huge drainage basin can be divided into three distinct parts (Berry et al., 1961). The first part of the basin comprises the eastern slopes of the Rockies, while the second part is made up of the foothills east of the mountains. These two parts combined, contribute 92% of the total flow carried by the South Saskatchewan River. The last part of the drainage basin is made up of the prairies, which is the largest in area, but contributes little to the flow. Some reasons why the prairie contribution is so low are: high moisture deficiency, poorly developed drainage and the fact that much of the snowmelt runoff goes into depressional storage (Stichling and Blackwell, 1957).

Flow duration is important in defining the river flow regime and understanding its capacity to transport sediment. With this in mind a flow duration table (Appendix A) was constructed from all the Lemsford data. The maximum daily discharge at Lemsford was 2 890 m $^3$ /s, while the minimum was 23.2 m $^3$ /s. As Figure 7 reveals, the median flow was 146 m $^3$ /s, discharges greater than 579 m $^3$ /s were exceeded 10% of the time, and discharges over 1 490 m $^3$ /s 1% of the time.

Monthly discharges for the period of record show June as the month contributing the greatest amount of flow, with the monthly flow accounting

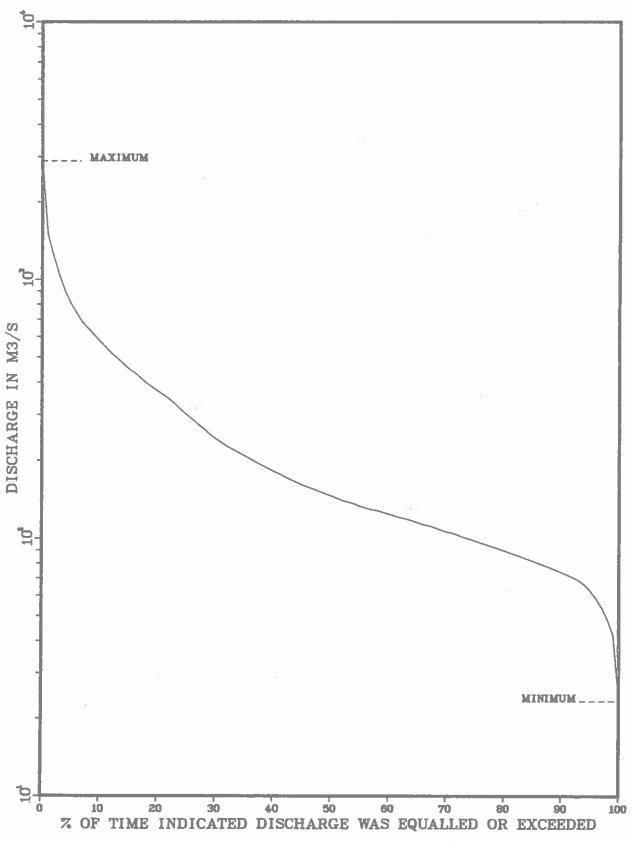


Figure 7
DURATION CURVE OF DAILY
DISCHARGES AT LEMSFORD BASED
ON AVAILABLE DISCHARGE DATA

for 25% of the total annual flow (Figure 8). It is also the month in which there is the greatest variation in discharges. In most years, the mountain runoff reachs the Lemsford site in June, but occasionally the peak discharge occurrs in early July.

Through correlation analysis it was possible to construct the flows at the Lemsford site based on data from the hydrometric stations located further upstream. Correlation analysis (Figure 9) of the total monthly discharges at Lemsford, and the combined flows for the Bindloss and HWY. 41 stations, available in Appendix B, revealed a high correlation. This correlation equation was then used to reconstruct total monthly discharges at Lemsford, to fill in the missing period from 1971-1980 (Appendix C).

Therefore, using the correlation equation for computation of the total annual flows it was possible to extend the record to 1980. During this 22-year period (Table 8), the South Saskatchewan River had a mean annual flow of 7 679 029 dam<sup>3</sup>. The highest year was 1965, with a discharge of 12 007 872 dam<sup>3</sup>, while the lowest year was 1977, with only 3 289 421 dam<sup>3</sup> of flow. Based on the average inflow, the reservoir has a total capacity-inflow ratio of 1.22 and can be considered as a hold-over storage reservoir (Brune, 1953).

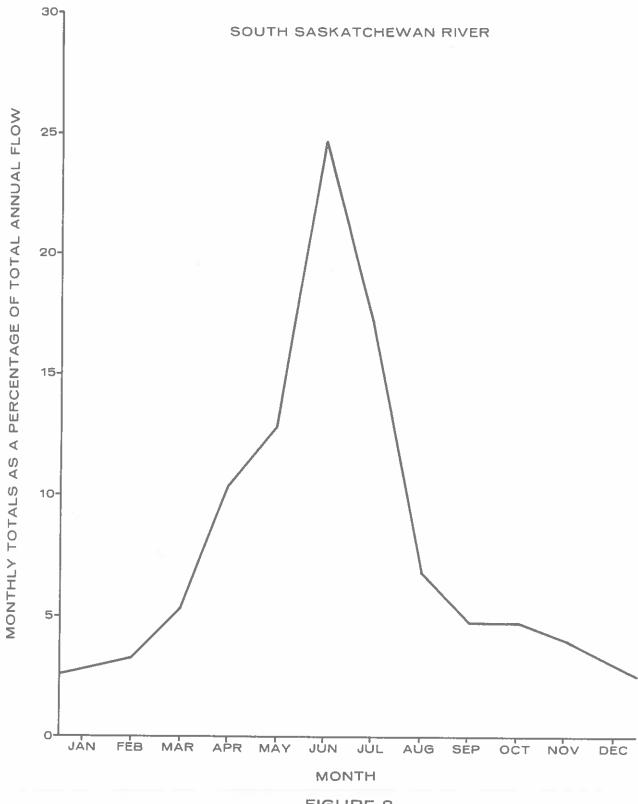
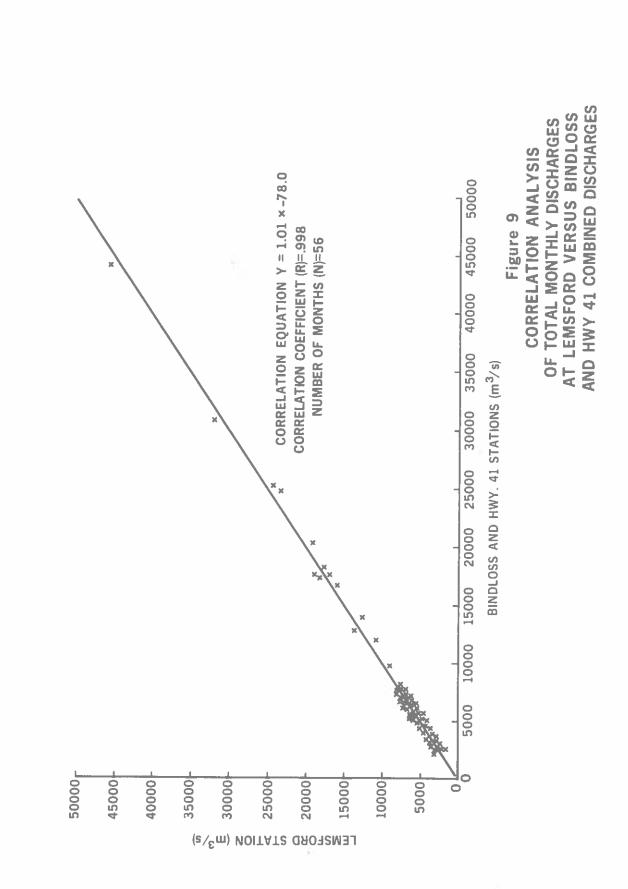


FIGURE 8
MONTHLY CONTRIBUTIONS TO TOTAL ANNUAL FLOW
BASED ON THE LEMSFORD STATION DATA



Total Annual Flows (dam<sup>3</sup>) for the Lemsford Station

Table 8

Year	Tota	1 Ani	nual Flow	Year	Total	Ann	ual Flow
1959	8	153	435	1970	6	943	363
1960	6	710	240	1971	8	075	635
1961	5	723	440	1972	10	229	069
1962	5	723	440	1973	6	328	973
1963	6	615	996	1974	9	713	347
1964	7	889	789	1975	8	501	760
1965	12	007	872	1976	6	754	666
1966	8	979	466	1977	3	289	421
1967	11	346	394	1978	8	014	896
1968	6	541	776	1979	4	919	962
1969	10	235	462	1980	6	240	240

MEAN (22 YEARS) 7 679 029

Source: Environment Canada, Surface Water Data Publications

It was determined that total annual flow would be a better parameter than the peak flood for the frequency study, because the South Saskatchewan River is highly regulated. Regulation of discharges tend to affect the peak flood more so than it does the total annual flow values. Frequency analysis is dependent on the number of years of data, and the more years of data, the more representative the distribution will be. Therefore, to increase the

number of years of record, flows at Saskatoon (STN. NO. 05HG001) were analyzed. Total annual flows at Saskatoon for the period prior to filling Lake Diefenbaker, which began in late 1964, were correlated with those at Lemsford (Table 9). There existed basically a one to one relationship which meant that the Saskatoon data, which extends back to 1912 could be used in frequency analysis of the South Saskatchewan River near Lemsford.

Total Annual Flows ( $\operatorname{dam}^3$ ) at Saskatoon and Lemsford

Table 9

	Saskatoon	Lemsford
Year	Total Annual Flow	Total Annual Flow
1959	8 153 435	8 293 968
1960	6 710 240	6 906 384
1961	5 723 440	5 644 944
1962	5 734 440	5 865 696
1963	6 615 994	6 622 560
1964	7 889 789	7 694 784
	Correlation equation	$y = 1.0 \times -13 336$
	Correlation coefficient	(R) = .979
	Number of years	(N) = 6

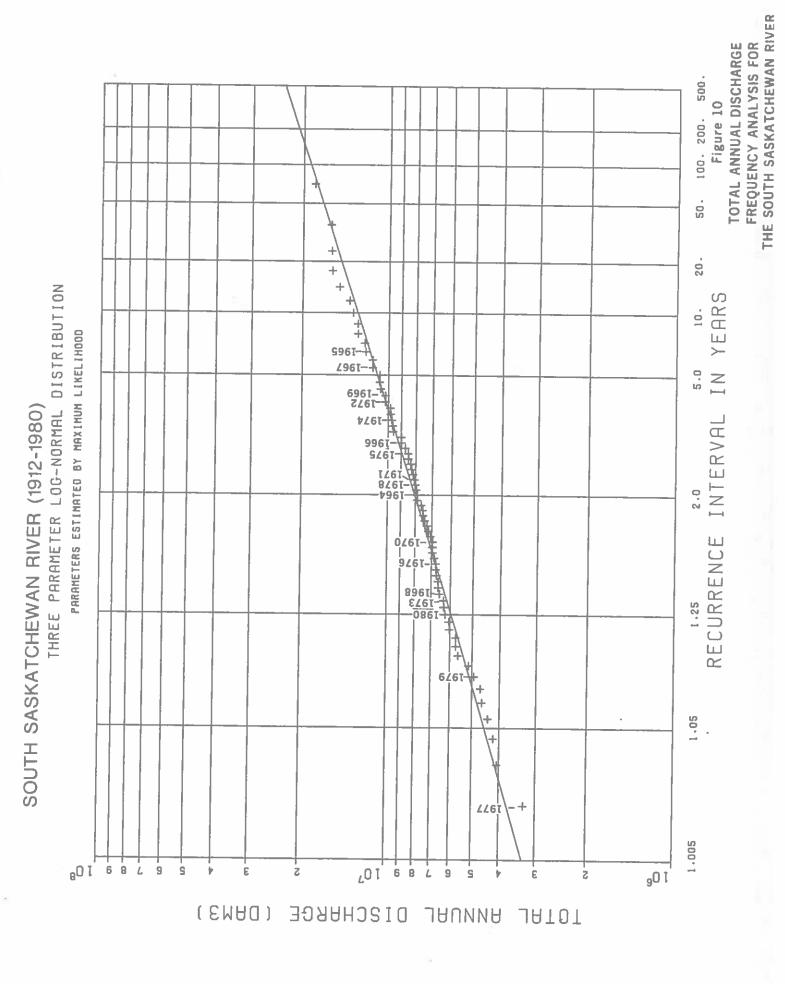
Source: Environment Canada, Surface Water Data Publications

Of the five more commonly used frequency distributions (Condie et al. 1981), the one that produced the best fit, based on coefficients of skewness, kurtosis and the standard error, was the three parameter log-normal distribution (Appendix C). Figure 10, depicts just how well the distribution fits the data. Since filling of the reservoir began in 1964, the greatest return period was 6.364 years, for the 1965 flow. The mean annual flow which corresponds to a return period of 2.333 years, was 8 262 432 dam<sup>3</sup>. For a 100-year return period the flow would be in the order of 18 900 000 dam<sup>3</sup>, while a 500-year return period would have to produce 23 300 000 dam<sup>3</sup> of flow.

# 3.3 <u>Characteristics of the Sediment Loads Transported by the South</u> Saskatchewan River

Suspended sediment sampling records for Lemsford showed that the minimum daily concentration was recorded to be 1 mg/L, while the maximum reached a high of 7 200 mg/L. The median concentration was less than 100 mg/L.

A mean annual hydrograph and sediment curve, based on nine years of data, temporally depicts the sediment transport regime (Figure 11). In any one year there are three periods when sediment transport is significant. The first time is in early spring, when snowmelt generates local runoff. The second time, and the most significant period, is when the mountain runoff reaches the site. The third, which is in September, appears not to be



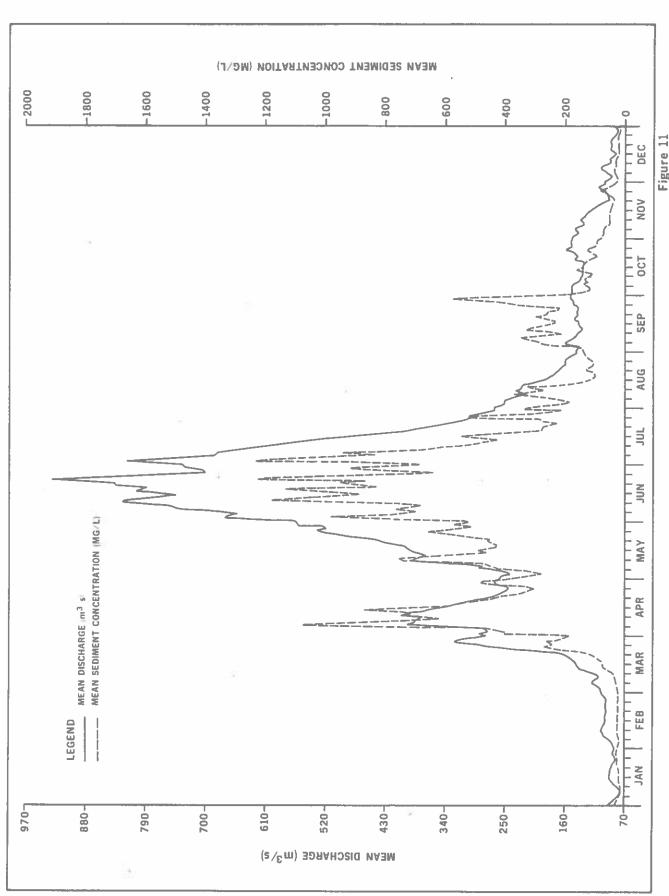


Figure 11
MEAN ANNUAL HYDROGRAPH AND
SEDIMENT CURVE FOR THE LEMSFORD STATION
BASED ON 1962 - 1970 DATA

associated with increased discharge. This increase in concentration is believed to be caused by high concentrations in the water being drained from the irrigation ditches.

Particle-size distributions are not only important in understanding the composition of the suspended sediment loads but more so how it will relate to hydraulics. An examination of the particle-size distribution from the 195 depth integrated samples (Appendix D), revealed that on average, clay made up 38%, silt 41% and sand 28% of the suspended sediment load.

To illustrate the relationship between discharge and suspended sediment, the concentrations from the depth integrating samples were plotted against discharge (Figure 12). The total concentrations showed considerable scatter – in most cases, up to an order of one magnitude. The sand component appears to be well defined, especially when discharges were greater than 400 m $^3$ /s. Most of the samples were collected from the upper median of flow ( $Q_{50}$ ) because of sample size requirements. This in turn provided good definition of the upper limits of sediment transport for the South Saskatchewan River.

Wash load is commonly defined as that part of the transported sediment load which is comprised of particles not found in significant quantities in the bed (Vanoni, 1975; Tywoniuk, 1972). An analysis of the results of the 49 bed material samples collected from the Lemsford site (Appendix F), indicated that on average silt and clay comprised less than 5%

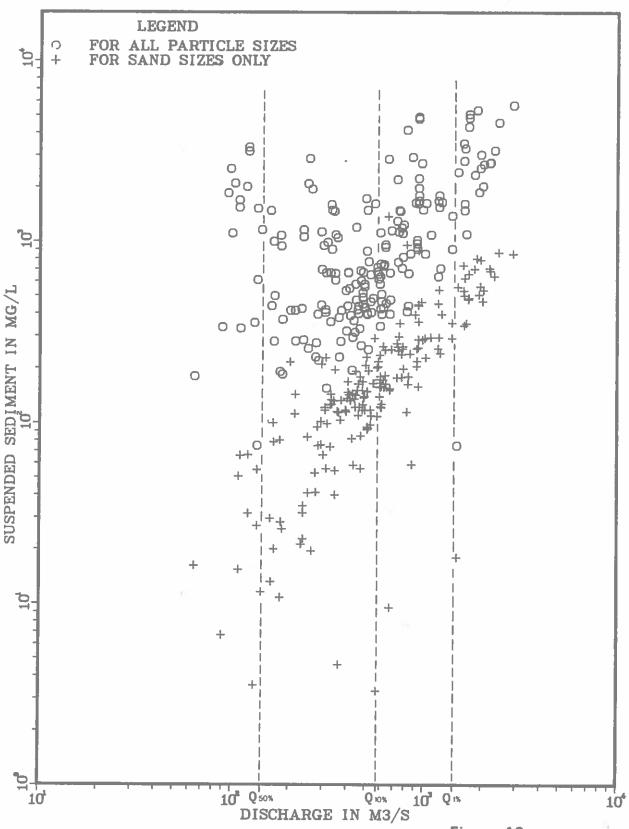


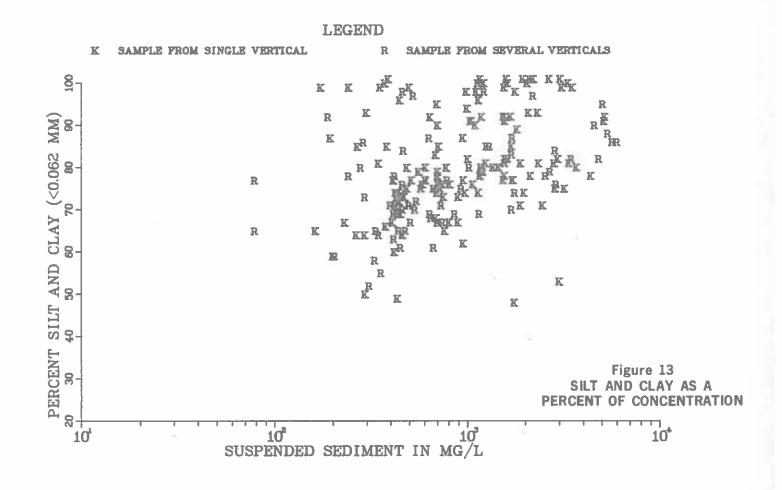
Figure 12
CONCENTRATION RELATIONSHIPS
TO DISCHARGE

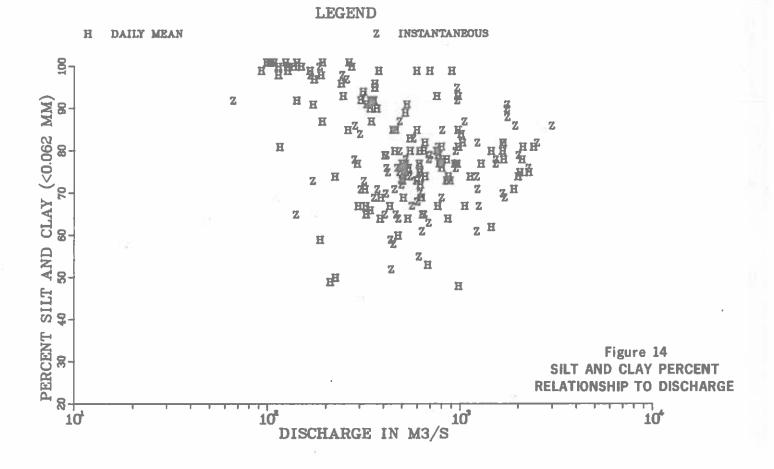
of the bed. Therefore, based on this definition, silt and clay can be considered to be the wash load component. Wash load has been found to be controlled by basin supply rather than hydraulics (Kellerhals et al., 1974), therefore explaining the poor relationship to discharge (Figure 12).

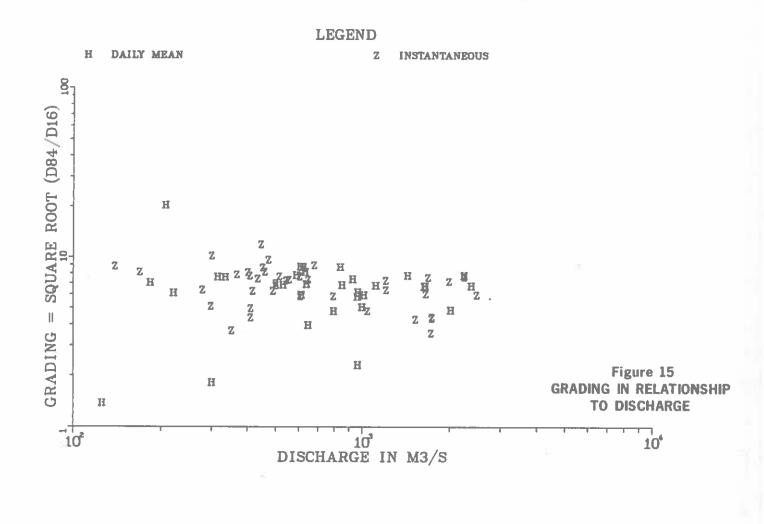
Wash load was found to make up a greater portion of the load as concentration increased (Figure 13) suggesting there is an adequate supply of sediment from the basin. However, as discharges increased, there was marked declines in the wash load contribution (Figure 14). This is probably because the stream's energy is being expended on bed material transport, which in the case of the South Saskatchewan River is primarily sand. Therefore, in proportion the wash load contribution would be reduced.

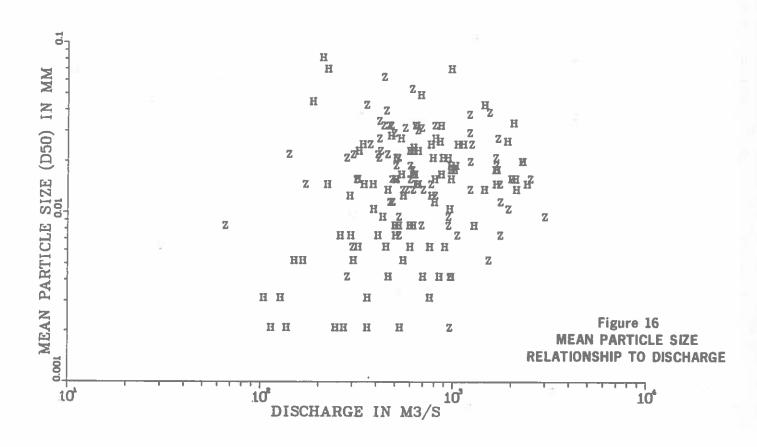
River, samples' gradings were plotted against discharge (Figure 15). From this it appears the suspended load is widely graded, as would be expected from a river with a high wash load component. Furthermore, gradings appeared to be consistent even at higher discharges. The mean particle size shows the variability attributed to the wash load component (Figure 16). As expected there is no trend over the range of discharges. The  $D_{50s}$  were all in the coarse clay to very fine sand range.

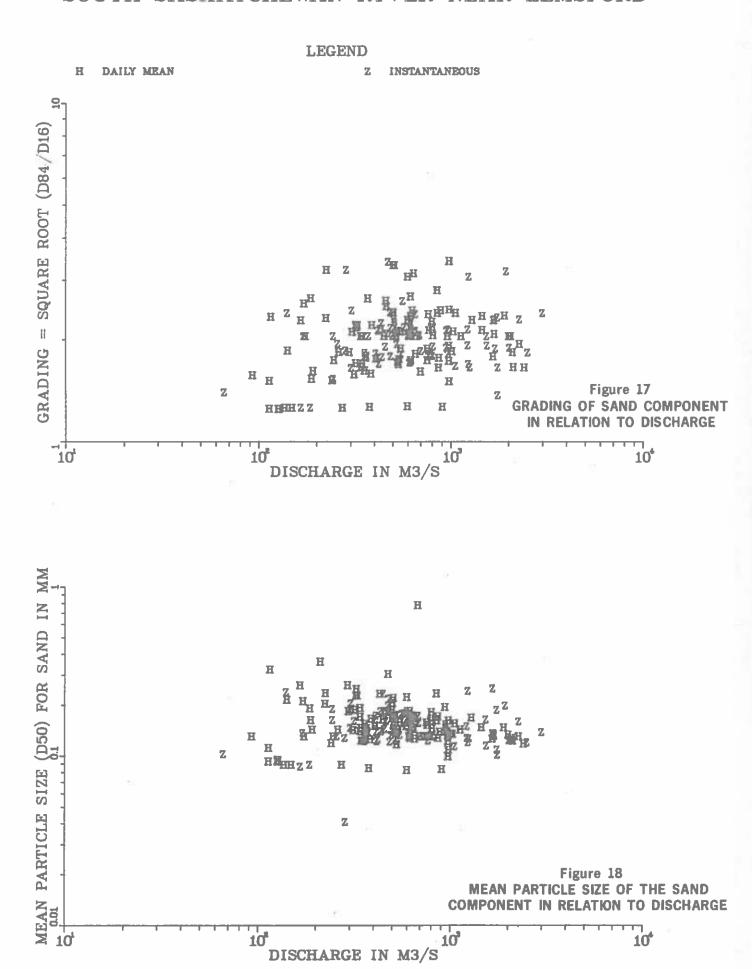
Further analysis of the sand component of suspended sediment load indicated that it was narrowly graded, with no apparent change with discharge (Figure 17). The mean particle size appears to be somewhat constant over the range of discharges, with most of the  $D_{50s}$  being in the very fine to fine sand range (Figure 18).











Bed material sampling results (Appendix E) revealed that on average silt and clay comprised 5% of the sample, sand 87% and gravel the remaining 8%. The mean particle size of the bed material in most cases was found to be within the fine to medium sand range. The gradings show that the bed has remained fairly homogeneous over the years of sampling.

There has never been any bed load sampling conducted on the South Saskatchewan River. To date the only successful bed load sampling has been restricted to streams with beds of coarse sand and gravel (Nordin, 1981). The South Saskatchewan River bed is made up of fine sands; below what is considered optimum for sampling. Bed load in most cases has been found to make up between 5-20% of the total sediment load (Tywoniuk, 1972). Kuiper (1962), estimated using the Einstein method, that bed load only made up 5% of the total load transported by the South Saskatchewan River. Since, in this study, the suspended sediment loads have only been extrapolated, adding an arbitary bed load contribution would not significantly affect the results.

A comparison of the monthly suspended sediment loads at Lemsford to the combined loads of Bindloss and Hwy. 41 (Appendix F) provided a strong correlation (Figure 19). This correlation equation was then used to reconstruct the monthly loads at Lemsford and continue the period of record to 1980.

During these 19 years, the South Saskatchewan River transported an average suspended sediment load of 5 403 249 tonnes. The maximum annual sediment load transported was 11 438 266 tonnes in 1967, the minimum load was computed to have been 633 963 tonnes in 1977 (Table 10).

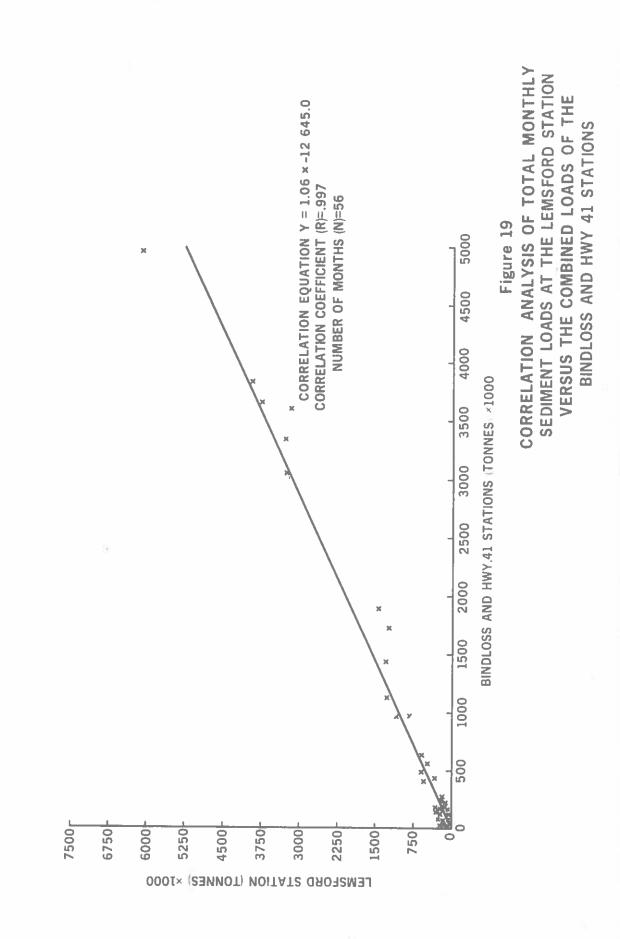


Table 10

Total Annual Suspended Sediment Loads (Tonnes) for the Lemsford Station

Year	Annual Suspended Sediment Load	Year	Annual Suspended Sediment Load
1962	1 643 330	1972	6 560 327
1963	4 433 330	1973	2 312 512
1964	6 989 409	1974	9 793 671
1965	8 818 661	1975	8 974 955
1966	4 738 083	1976	2 188 859
1967	11 438 266	1977	633 963
1968	2 410 830	1978	4 204 436
1969	9 576 518	1979	1 443 823
1970	5 111 928	1980	4 155 063
1971	7 233 764	MEAN (19 YEARS)	5 403 249

Source: Environment Canada, Sediment Data Publications

#### 3.4 Swift Current Creek Hydrology

Swift Current Creek drains an area of 3 910 km<sup>2</sup> as it discharges into Lake Diefenbaker. The creek has been regulated since 1942, with the establishment of Reid Lake. The station, Swift Current Creek near the Mouth (STN. NO. 05HD037) operated from 1965 to 1972, but was moved eight kilometres upstream to Leinan (STN. NO. 05HD039) in 1973, reducing the gross drainage area to 3 720 km<sup>2</sup>. Initially, the station was operated on a eight-month

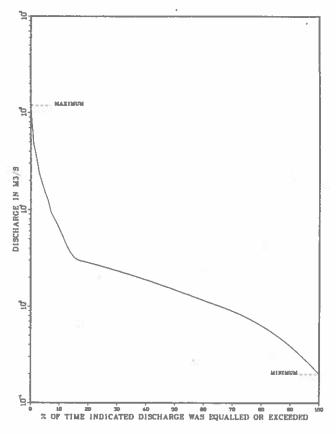
basis - March to October, but since 1973, discharge data have been collected year round. The sediment data collection program has always been operated only during the open-water period.

The winter months do not account for much of the total annual flow.

In fact, based on Leinan data, less than 12% of the total flow was contributed from this four-month period. Since the eight-month period was common to both sites, only that period has been used for comparative purposes.

During the period from 1966 to 1978, a maximum daily discharge of 119 m<sup>3</sup>/s and a minimum of .027 m<sup>3</sup>/s were recorded for Swift Current Creek. As Figure 20 shows, discharges were significantly less during the period from 1973 to 1978. Flow duration tabulations for the two sites (Appendix G), revealed that the flow regime has changed significantly. A comparison of the median flows from the two periods revealed a reduction of almost 60% in the discharge. This reduction in flow is not considered to be man-induced but just a drier period of record for the prairies. The period from 1973 to 1978 is important for this study, since the hydrographic survey of the mouth region was conducted in 1972 and again in 1978.

Table 11 lists the total annual flows for the 13-year period, of which the maximum annual flow was 121 046 dam $^3$  in 1967, and the minimum was 10 195 dam $^3$  in 1973. The mean annual flow was calculated to have been 58 606 dam $^3$ .



SWIFT CURRENT CREEK NEAR LEINAN

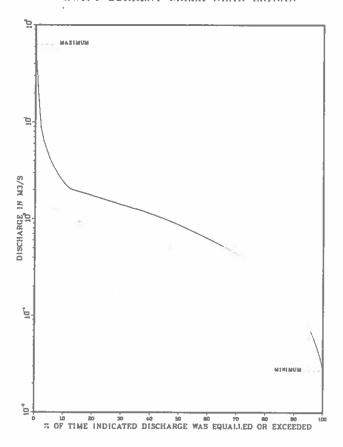


Figure 20
DURATION CURVES OF DAILY
DISCHARGES FOR THE TWO STATIONS

Table 11

Total /	Annua l	Flows	(dam <sup>3</sup> )	for	Swift	Current	Creek
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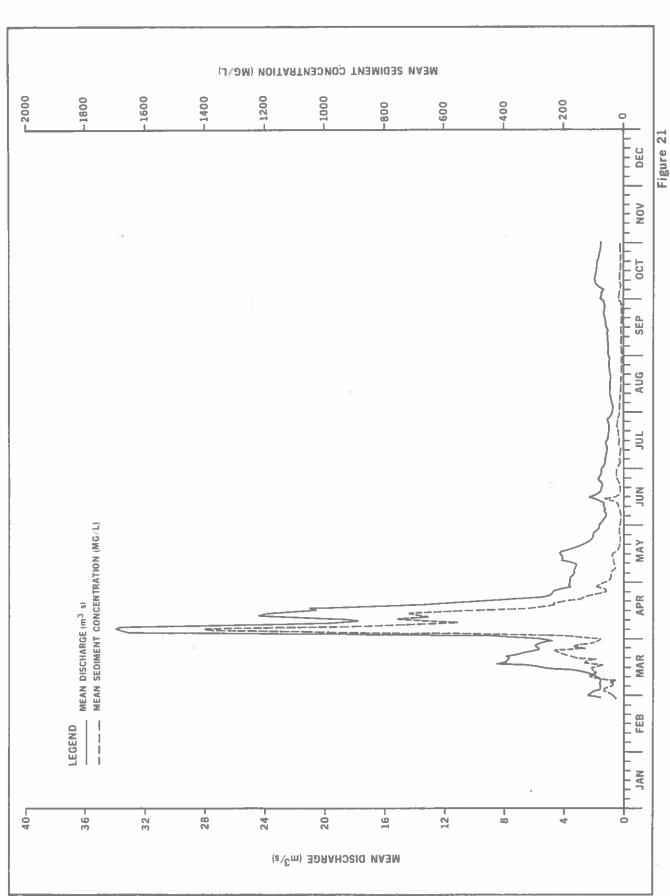
Year	Total Annual Flow	Year	Total Annual Flow
1966	77 501	1973	10 195
1967	121 046	1974	44 323
1968	37 843	1975	44 852
1969	80 265	1976	66 701
1970	119 318	1977	13 910
1971	99 446	1978	21 859
1972	24 883	MEAN (13 YEARS)	58 606

Source: Environment Canada, Surface Water Data Publications

# 3.5 <u>Characteristics of the Sediment Loads Transported by Swift Current</u> <u>Creek</u>

Based on findings from the suspended sediment sampling program on Swift Current Creek the minimum daily concentration was 1 mg/L, while the maximum was 4 340 mg/L. For Leinan, the median concentration was determined to be 18 mg/L.

During the period from 1965-1972, there was a distinct freshet flow accounting for most of the sediment transport (Figure 21). More than 90% of the total suspended load carried by the creek was during the month of April.



MEAN ANNUAL HYDROGRAPH AND SEDIMENT CURVE FOR THE SWIFT CURRENT NEAR THE MOUTH STATION DATA COLLECTED FROM 1965 · 1972

However, from 1973-1978 the sediment loads were significantly less, due to the dramatic decrease in the magnitude of the freshet flows (Figure 22). The mean peak concentration was reduced to 25% of what it had been for the prior eight years.

There were 77 depth integrating samples analyzed for particle size (Appendix H) for the station near the mouth. The results show that on average a sample would be made up of 19% clay, 53% silt and 28% sand. Unfortunately, only one sample was collected and analyzed for its particle-size distribution at the Leinan site. Almost all the samples were collected in the upper 10% of the flow range. Total concentration varied an order of magnitude, while the sand component showed even greater variation (Figure 23). This is considered common for flashy streams such as Swift Current Creek. The silt and clay fraction showed a marked decline with increasing discharge, which suggests that availability of sediment from the basin is limited (Figure 24). There was a distinct decline in the silt/clay to discharge relationship with increasing discharge (Figure 25). This was similar to what occurred on the South Saskatchewan River.

The gradings show (Figure 26) the trend is towards the suspended sediment becoming more widely graded with increasing discharge. The mean particle size was found to increase significantly from fine silt to the very fine sand class (Figure 27). The sand component was found to be relatively homogeneous over the flow range (Figure 28). The mean particle size of the

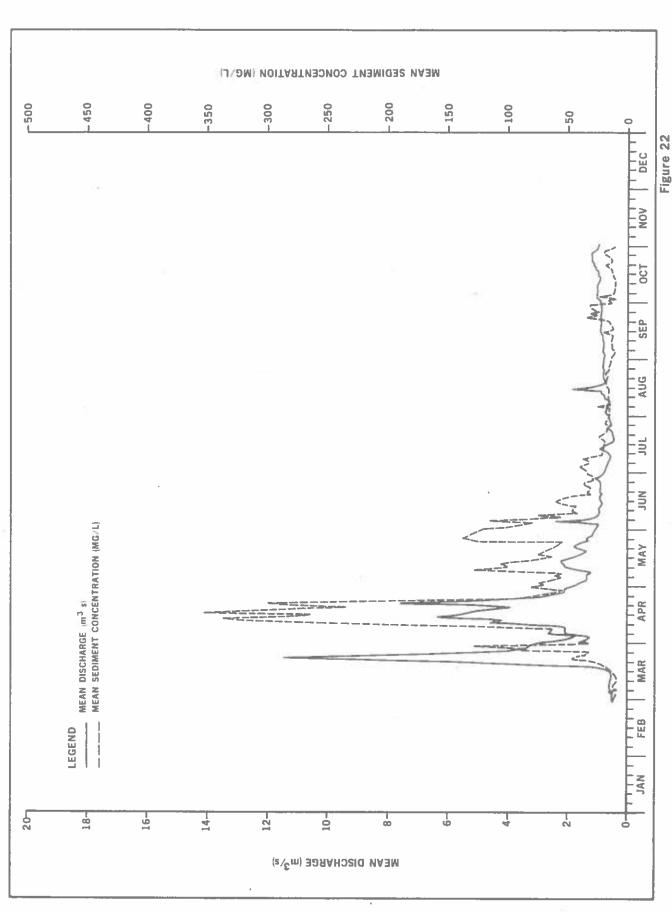
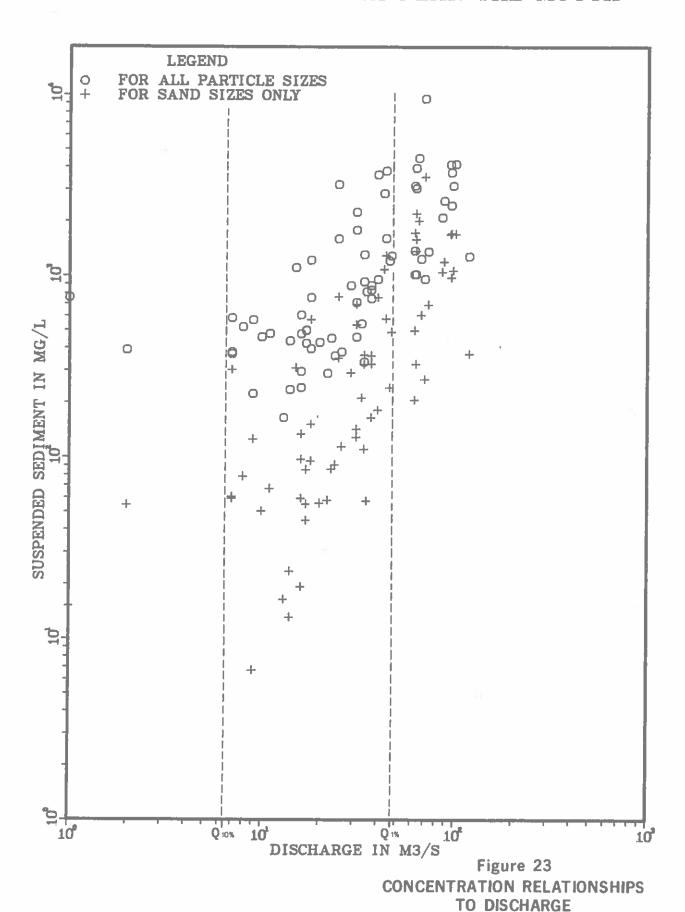
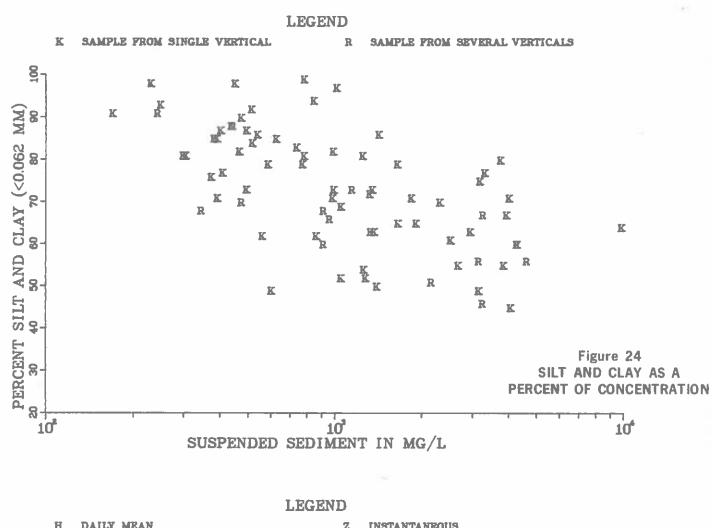


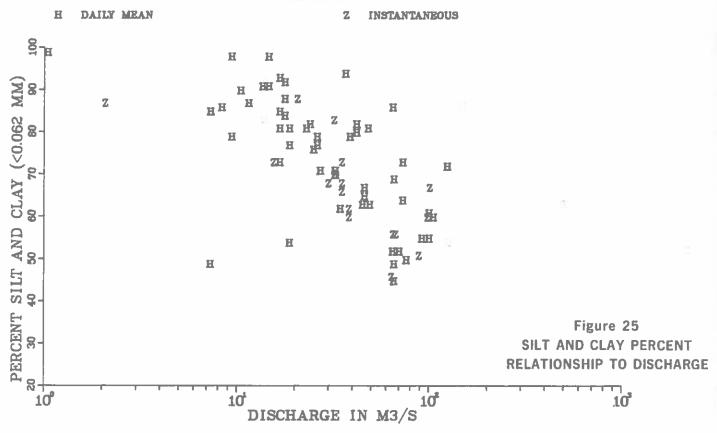
Figure 22
MEAN ANNUAL HYDROGRAPH AND
SEDIMENT CURVE FOR THE LEINAN STATION
DATA COLLECTED FROM 1973 · 1978

# SWIFT CURRENT CREEK NEAR THE MOUTH

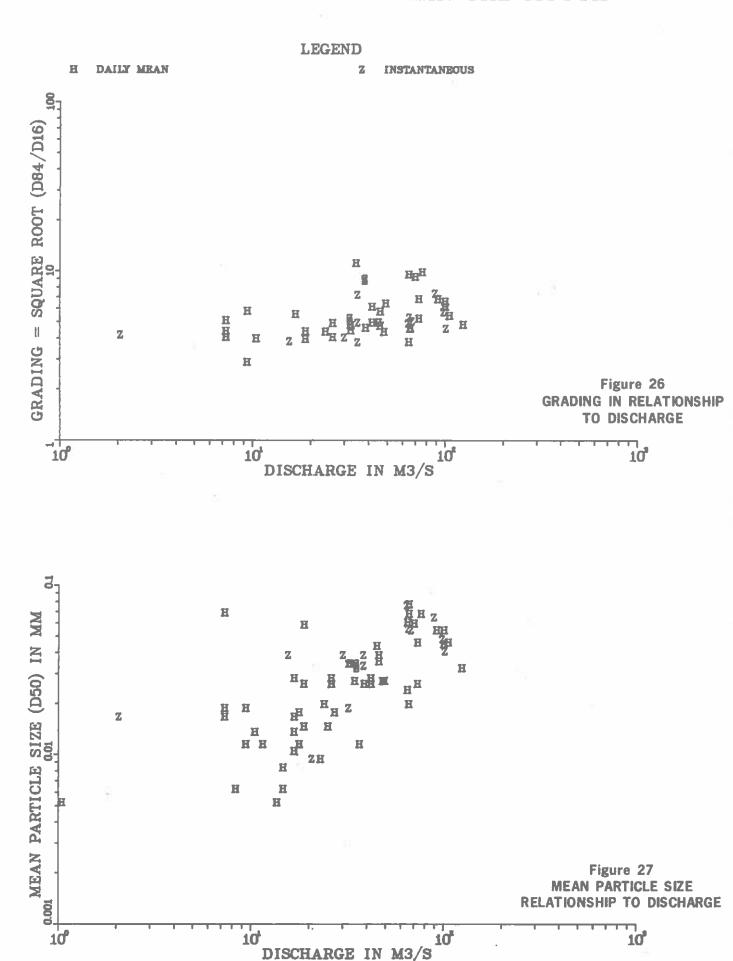


# SWIFT CURRENT CREEK NEAR THE MOUTH

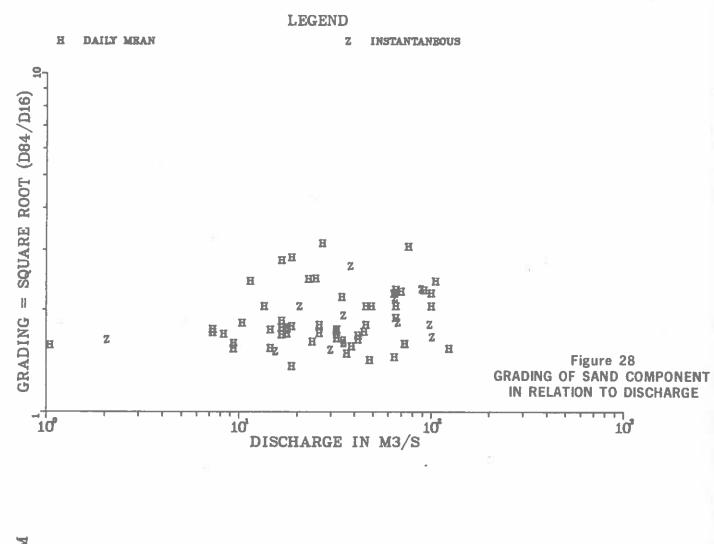


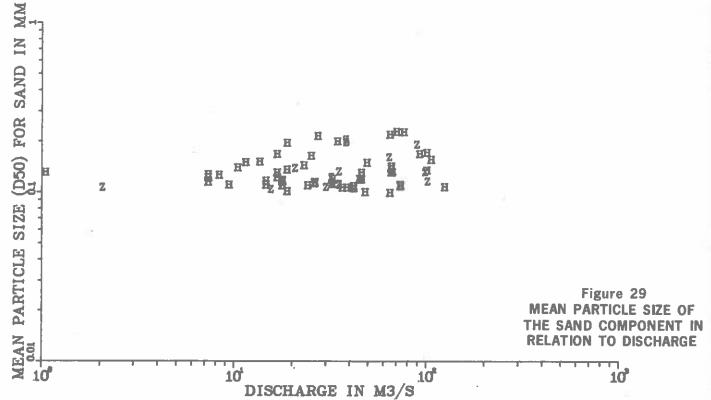


# SWIFT CURRENT CREEK NEAR THE MOUTH



# SWIFT CURRENT CREEK NEAR THE MOUTH





sand fraction did not vary much with discharge and all samples were within the very fine to fine sand range (Figure 29).

Bed material was sampled only once on Swift Current Creek and that was at the mouth. This one sample showed the bed to be comprised totally of sands in the very fine to medium range.

Table 12 indicates the annual suspended sediment load transported by Swift Current Creek during the open-water period. The greatest load was 152 728 tonnes in 1967, and the smallest was 243 tonnes in 1973. On average, the creek transported 40 969 tonnes a year. In comparison to the South Saskatchewan River's contribution, it is insignificant, but it is important as a local input.

Table 12
Total Annual Suspended Sediment Load (Tonnes) for Swift Current Creek

Year	Annual Suspended Sediment Load	Year	Annual Suspended Sediment Load
1966	27 685	1973	243
1967	152 728	1974	19 058
1968	2 119	1975	13 594
1969	84 790	1976	28 713
1970	104 209	1977	1 277
1971	96 011	1978	735
1972	1 433	MEAN (13 YEARS)	40 969

Source: Environment Canada, Sediment Data Publications

# 4. Data Analyses and Results

# 4.1 <u>Introduction</u>

# 4.1.1 <u>Definition of Reaches</u>

In this analysis the 38 ranges have been divided into three distinct reaches. These reaches have been defined on the basis of the original minimum bed elevations and reservoir water level operations.

The first reach, referred to as the upstream reach, extends from Range 38 downstream to Range 32. Range 32 marks the location where the bed elevation approximates the FSL elevation of 556.87 m. Some ranges upstream of Range 32 have minimum bed elevations that are below the FSL elevation, however, this only represents a very small portion of the cross-sectional area of the range. Range 32 marks the point where this becomes a more significant portion of the cross-sectional area. In this reach, the South Saskatchewan River still behaves as a river but is influenced by the backwater effects from the reservoir.

The next reach spans from Range 32 to Range 26 and has been termed the drawdown reach. It is in this reach that conditions change from river to reservoir depending on the reservoir water level. Reservoir studies (US Bureau of Reclamation, 1977) have shown that this is where the major portion of the sediment load transported by the river is deposited. In this reach, the bed elevations are less than that of the FSL elevation, but greater than

the minimum operating water level elevation of 545.59 m. The capacity in this reach is live storage capacity and therefore significant in that any changes will directly affect reservoir operations.

Range 26 marks the approximate location where reservoir conditions exist year round and therefore all the ranges downstream are considered as in the reservoir section. This point also marks the beginning of dead storage capacity, which is usually not considered important in terms of the reservoir operation. However, once the dead storage capacity has been depleted, live storage becomes affected, reducing the useful life of the reservoir.

# 4.1.2 Data Restrictions

As a massive quantity of hydrographic data has been used in this study it is not feasible to make it all available in standard formats. For instance, for every time a range was surveyed the data have been depicted in three formats: a cross-sectional profile, profile sheet and an elevation - area table. Proper representation of the sector data requires a contour plot for each year, elevation-capacity tables and a contour-difference plot between the different years. Since these data are available on request, only the data summaries are contained within this report.

As mentioned earlier both cross-sectional as well as sector (area) data were collected at the ranges downstream of Range 27. This was done primarily to evaluate and compare these two different data collection

strategies. However, with the data that have been collected to date, it was not possible to properly compare these two approaches. Some reasons for this are the quality of the early sector surveys were poor, and much of the sector data have been collected where there has been neglible changes, which in turn has significantly reduced the number of ranges for comparison purposes:

Therefore, further data will be required to properly evaluate and compare these two different data collection strategies.

Since cross-sectional data have been collected from the beginning of the program and throughout the reservoir over the years, this form of data served as the primary data source used to assess sedimentation. However, it should be noted that at the wider ranges, those below Range 9, the cross section findings were found to be questionable. This can be attributed to the fact that at these wider ranges it was increasingly difficult to stay on the baseline. Therefore, this analysis was restricted to data only as far downstream as Range 9. As this analysis will show this did not affect the purpose of this study, since negligible sedimentation was noted for quite a distance upstream of Range 9.

Background information which is important for the analysis of the reservoir as a whole, as well as for particular reaches, is provided in Table 13. The table contains thalweg distance between ranges, reference elevations used in the measurement of the baseline for each range, and the representative live, dead and total storage volumes for each range. These volume figures were derived by averaging the cross-sectional area of two

TABLE 13
Distance and Volume Information for Ranges

Location	Thalweg Distance (km)	Range No.	Reference Elevation (m)	Baselir Length (m)		Dead Storag (dam <sup>3</sup>	e S	Total torage (dam <sup>3</sup> )
38-37	12.0	38	563.0	121.9	_	-		
37-36	7.6	37	561.0	156.3	_	-		
36-35	6.0	36	560.0	323.8	_	_		
35-34	3.1	35	558.5	210.0	-	-		
34-33	5.1	34	558.0	375.0	_	_		
33-32	5.1	33	557.0	234.0	_	-		
32-31	6.4	32	556.5	453.7	3 520			520
31-30	7.1	31	556.5	718.8	14 353	-		353
30-29	5.8	30	556.5	1 725.0	27 694	-		694
29-28	4.9	29	556.5	1 593.8	36 158	w-		158
28-27	8.9	28	556.5	1 675.0	51 465	-	51	465
27-26	3.5	27	566.5	589.1	44 186	-	44	186
26-25	6.9	26	556.5	871.3	40 140	835		975
25-24	8.0	25	566.5	770.0	53 078	3 832		910
24-23	6.0	24	566.5	496.9	40 491	6 513	47	004
23-22	3.6	23	556.5	715.0	27 373	6 093	33	466
22-21	2.9	22	556.5	840.0	21 976	5 156 5 419	27	133
21-20 20-19	2.4 5.5	21 20	556.5	946.9 1 012.5	91 940 40 177		25	358 025
19-18	6.2	20 19	556.5 556.5	1 162.5	40 177 64 281	10 847 21 516	51 85	797
18-17	4.4	18	556.5	822.5	56 405	24 898	81	303
17-15	2.2	17	556.5	1 040.6	35 135	17 809	52	944
15-14	4.7	15	556.5	975.0	46 966	29 787		753
14-13	5.8	14	556.5	1 556.3	82 770	57 312		
13-12	6.2	13	556.5	1 375.0	107 056	77 327	184	383
12-11	4.4	12		2 156.3	111 855	85 905	197	560
11-10	4.9	ii	556.5	2 406.3	120 322	101 066	221	388
10-9	5.1	10	556.5	2 400.0	131 055	125 644		670
9-8	7.8	9	556.5	1 828.1	151 833	170 345		183
8-7	9.7	8	556.5	1 893.8	205 829	240 939		768
7-6	13.8	7	556.5	2 080.0	287 593	356 777	644	370
6-5	13.0	6		2 137.5	323 477	453 430	776	908
5-4	16.6	5	556.5	2 000.0	331 272	580 908	911	370
4-2	24.6	4	556.5	1 625.0	ESTIMAT	ED REMAINING	VOLUME	
2-1	13.9	2		1 968.8	1 533 812	2 970 124	4 503	936
1-DAM	8.3	1		3 075.0				
Total	262.4				4 010 000	5 350 000	9 360	000

consecutive ranges and multiplying it by the distance between the two ranges. These values then were summed to provide a total, and each individually ratioed to the total. The ratios were applied to the original established capacity values, obtained through mapping, to derive the volumes between each set of ranges. The representative volumes were obtained by taking half the volume to the immediate upper range and half the volume to the next lower range. The baseline length for all the ranges downstream and including Range 32 were measured at the reference elevation of 556.5 m, which approximated the FSL elevation. For the ranges upstream of Range 32, the reference elevation was based on the minimum water level elevation obtained from all the surveys of a particular range, ensuring the cross-sectional changes are comparable for all years.

The hydrographic surveys since 1972 have been on a three-year cycle, so this interval was used for this analysis. Therefore changes from pre-reservoir to 1972 were studied, 1972 to 1975, 1975 to 1978 and from 1978 to 1980, which was the last survey conducted.

#### 4.2 Upstream Reach

The original bed slope was derived by straight line interpolation between the minimum bed elevation at Range 38 and that of Range 32. The bed changes were derived from differences in the mean depth (cross-sectional area divided by the baseline length).

The upstream reach is normally an aggrading reach as river velocities are reduced due to backwater effects (Vanoni, 1975). However, as the 1972 survey results revealed, the 39 km-long reach had been severely scoured (Figure 30). In this reach, the South Saskatchewan River meanders and there are many islands and migrating sandbars that provide anchorage for ice jams. Two major ice jams have been recorded to have occurred prior to 1972; the first happened on April 7, 1969 and the second April 10, 1971 (Water Survey of Canada Files). The 1969 ice jam was the most significant and was responsible for destroying the Lemsford hydrometric station, located immediately upstream of Range 38. To put a perspective on the magnitude of the ice jam, the highest stage recorded at this site was 6.03 m for the 1975 flood, whereas the stage recorded from the ice jam was 9.36 m. Therefore, it is likely that upon breaking, the water and ice backed up behind the jam were responsible for the scouring noted along the reach.

As can be seen from Table 14, Range 37 had been scoured the most, with 1.3 m of sediment being removed from the channel. All ranges were found to have experienced some degradation and an estimated 4 500 dam<sup>3</sup> of sediment were estimated to have been eroded from this reach by 1972.

By 1975, infilling had taken place along most of the reach.

Range 38 findings indicated that further minor degradation had occurred as the river adjusted for the severe change in slope along that section.

Cross-sectional changes for Range 33 showed further scour, contrary to the findings from the other ranges in this section. However, Range 33 is a very narrow cross section and probably was affected by local scour.

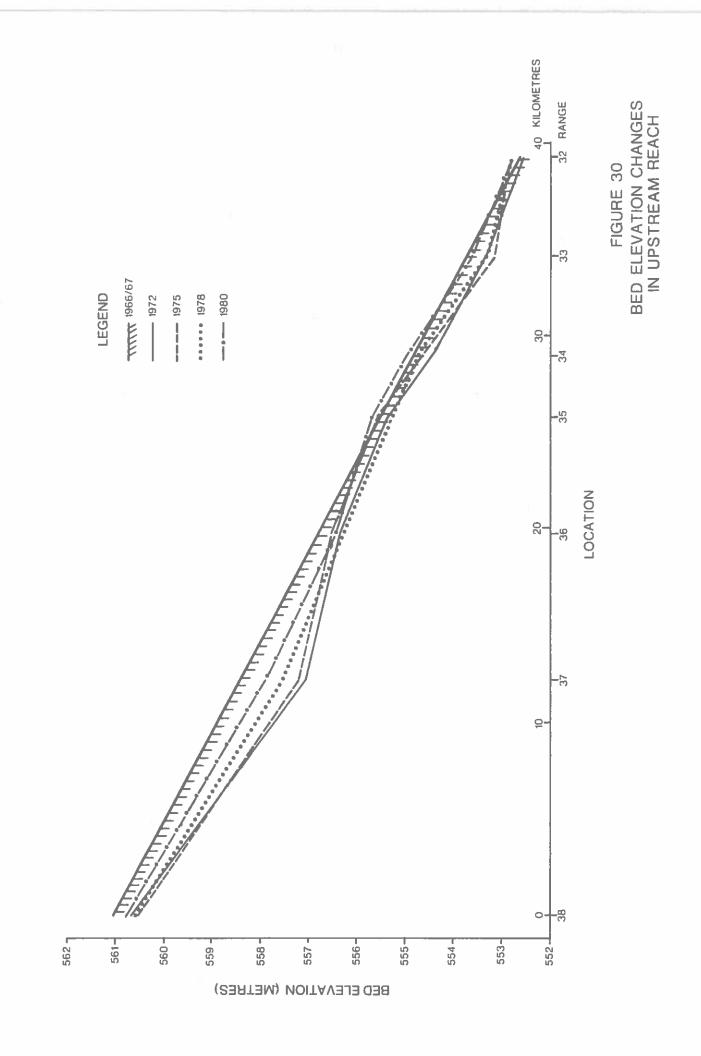


TABLE 14

560.75 556.49 555.65 552,78 557.91 555.01 553.61 -0.30 -0.56 -0.33 +0.13 +0.15 -0.15 +0.12 1980 1979 560.52 557.60 556.38 555,35 553.39 -0.53 -0.87 -0.44 -0.17 1978 554.82 552.78 -0.04 +0.12 1977 Mean Bed Changes (Metres) and Elevations (Metres) for Upstream Ranges 1976 557.38 560,56 556,56 555,56 553.22 552.77 -0.49 -1.09 -0.26 +0.04 +0.11 -0.54 1975 1974 1973 557.15 556.42 555.43 560.65 553.41 552.60 -1.32 -0.40 -0.09 -0.40 -0.35 -0.06 1972 554.44 -0.42 1971 557.28 552.63 -1.19 -0.03 1970 555.52 1969 553.50 -0.26 1968 552.66 1967 553.76 50.199 558.47 556.82 555,52 554.86 1966 38 Mean Bed Change 37 Mean Bed Change 36 Mean Bed Change 35 Mean Bed Change 34 Mean Bed Change 33 Mean Bed Change 32 Mean Bed Change Year Elevation Elevation Elevation Elevation Elevation Elevation Elevation

The 1978 survey results show further infilling along most of the reach with Range 38 still continuing to show degradation. Also, Range 36 and Range 35 were degrading as the river continued to make adjustments to the ice scour effects.

By 1980 deposition had been recorded at all the ranges, and conditions began to represent those that were there prior to the ice jams.

The original average slope for the reach based on the earliest surveys was 0.00022, and by 1972 it had been reduced to 0.00021. The slope recorded from the 1975, 1978 and 1980 data was slightly lower at 0.00020. Based on these findings it appears that the long term trend should be one of further reduction in bed slope as sediment continues to be deposited in the lower ranges.

# 4.3 Drawdown Reach

This reach (Range 32-Range 26) extends for 37 km and has an estimated volume of 218 300 dam<sup>3</sup>, representing about 5.4% of the total live storage volume. In this reach the river flows into the reservoir and velocities are reduced greatly causing the major portion of the sediment load to be deposited.

The reservoir water level at the time of the freshet is a good indicator as to where the sediment loads will begin to be deposited out of suspension (Table 15). As the reservoir water levels indicate, sedimentation during most years would begin to occur in the upper portion of this reach and

Reservoir Water Level Elevations at Peak Discharge Time

TABLE 15

Date	Peak Discharge	Return Period	Elevation
	$(m^3/s)$	(Yr)	(m)
June 23, 1965	1 951	4.25	525.2
June 8, 1966	1 184	1.70	526.1
June 4, 1967	2 432	8.50	541.0
June 16, 1968	897	1.31	550.1
July 3, 1969	2 178	5.67	555.7
June 21, 1970	1 824	3.40	553.8
June 11, 1971	1 388	2.13	553.5
June 7, 1972	1 472	2.43	552.9
June 3, 1973	779	1.33	552.9
June 23, 1974	1 501	2.83	554.5
June 24, 1975	2 890	17.00	555.1
August 14, 1976	906	1.42	551.4
June 15, 1977	268	1.06	551.3
June 15, 1978	1 076	1.55	553.8
June 2, 1979	825	1.21	553.9
June 1, 1980	1 260	1.89	552.3

Source: Saskatchewan Environment Records

most of the reach would be exposed to deposition. Prior to 1968 most of the sediment loads were transported and deposited at the lower ranges in the reservoir. Therefore, the period from 1968 to 1978 basically marks the first ten years the reservoir has been fully operational.

As shown in Figure 31, this reach had been continually aggrading over the years. By 1972, significant accumulation had been recorded for the ranges downstream of Range 30, whereas upstream changes were negligible (Table 16). Range 26 was shown to have the greatest change with 0.87 m of deposition. Both Range 27 and 28 also had significant accumulation with 0.62 and 0.61 m respectively.

The 1975 survey data revealed the same trend with sedimentation occurring throughout the whole reach and the most significant accumulation rates being recorded at the lower ranges. The data shows that the most deposition had occurred at Range 27 with 1.45 m being infilled.

After ten years of reservoir operation significant changes were measured. Range 27 still showed that it was the point of most accumulation within this reach followed closely by Range 26.

The 1980 data, although somewhat limited, does indicate further deposition throughout the reach. Therefore, now that reservoir operations are more stable from year to year, this general pattern of deposition can be expected to continue.

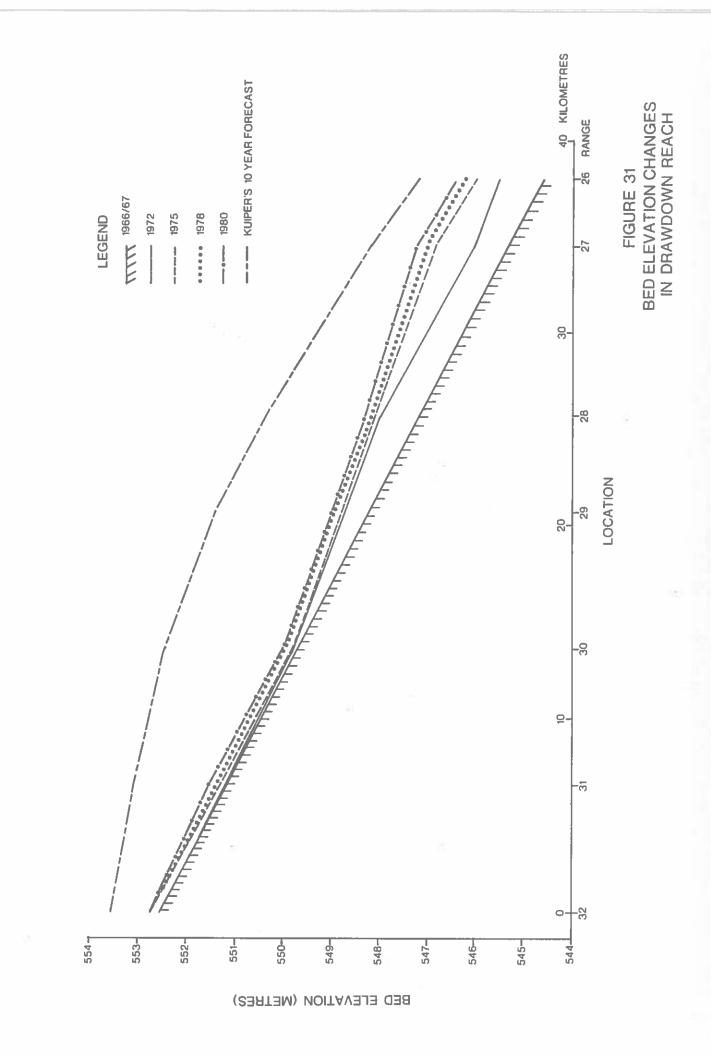


TABLE 16

Mean Bed Changes (Metres) and Elevations (Metres) for Drawdown Ranges

	>		2000	1001		מון מכן כל	) cafinging	ייברו בא) י	מווח רובאנ	יי פווסנוסג	metres) r	ur urawau	real Dea Clauges (Meries) and Lievacions (Meries) for Ordwoomin Kanges					
Year 1966		961	.0	1961	1968	1969	1970	1971	1972	1973	1974	1975	1976	1977	1978	1979	1980	Kuiper's Forecast
Range																		
32 Mean Bed Change	ge						-0.03		-0.06			+0.11			+0.12		+0.12	+0.93
Elevation				552,66			552.63		552.60			552.77			552,78		552.78	553,59
31 Mean Bed Change	ge					+0.03		-0.02									+0.24	+1.88
Elevation				551.27		551.30		551.25									551.51	553,15
30 Mean Bed Change	ge						+0.01		+0.02			+O° 0e			+0.24			+2.81
Elevation				549.71			549.72		549.73			549.77			549.95			552.52
29 Mean Bed Change	ge					+0.28									+0.56			+3.05
Elevation				548.43		548.71									548.99			551,48
28 Mean Bed Change	ge				+0-31			+0.61						+0.85			+O* 66	+3.05
Elevation 54	20	rg.	547.36		547.67			547.97						548.21			548.35	550.41
27 Mean Bed Change	ge					19.0+			+0.62			+1.45			+1.68			+2.81
Elevation 5	ιÒ	LÔ.	545.41			546.02			546.03			546.86			547.09			548.22
26 Mean Bed Change	ge				+0.28				+0.87			+1.41			+1.60			+2.59
Elevation 5	.co	LO .	544.65		544.93				545.52			546.06			546.25			547.24

A comparison with Kuiper's (1962) forecast shows that Kuiper overestimated deposition in this reach. There are two reasons why the estimates were so high: the first, was that they were based on sediment loads that were significantly larger; and second, it was assumed the water level would always be at FSL at the time of the freshet. The differences help illustrate some weaknesses in forecasting sedimentation with limited data.

To determine volumetric changes in this reach the percentage change in the cross-sectional area of each range was applied to the representative volume (Table 17). From 1966 to 1972 an estimated 15 400 dam<sup>3</sup> of storage had been lost due to sedimentation. That is a loss of 7.1% to this particular reach, while in terms of the total live storage of the reservoir it amounts to only 0.4%. Between 1972 and 1975 a further 12 500 dam<sup>3</sup> of sediment had accumulated amounting to a further 5.7% loss to reach and 0.3% to the total live storage. The next three years reduced the storage by 7 600 dam<sup>3</sup>, or a 3.5% loss to the reach and 0.2% over all. From 1978 to 1980 an estimated 5 000 dam<sup>3</sup> had been further lost, or 2.6% to the reach and 0.1% to the total live storage. By 1980 there had been a total loss of 18.9% in storage to the drawdown reach, which is about 1.0% of the total live storage and just less than 0.5% of the total reservoir capacity.

The original slope for this reach was 0.00022, and by 1972 it had been reduced to 0.00019. As sedimentation continued, the slope was further reduced to 0.00018 in 1975, and has remained the same to 1980. This gradual reduction in the slope with time should continue as the major portion of sediment load is deposited in the lower portion of the reach.

TABLE 17

Ranges
Drawdown
for
Changes
Volumetric
and
Percentage

	Volume Change (dam <sup>3</sup> )	-486	-1 091	-582	-1 374	-1 801	-4 419	-2, 745	-12 500	+14	-359	-1 689	-1 302	-926	-884	-533	-5 700
	Percentage Change	-13.8	-7.6	-2.1	-3.8	e. E.	-10.0	-6.7		+0.4	-2.5	-6.1	-3.6	8 4 1	-2.0	e	
awdown Ranges	Year S	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975		1978-1980	1978-1980	1978-1980	1978-1980	1978-1980	1978-1980	1978-1980	
: Changes for Ora	Volume Change (dam³)	+158	+158	-249	-3 327	-4 632	-3 358	-4 192	-15 400	-46	-545	-2 049	-1 482	-1 287	-1 326	-901	-7 600
Percentage and Volumetric Changes for Drawdown Ranges	Percentage Change	+4.5	+1.1	6.0-	2.6-	0.6-	-7.6	-10.2		-1-3	3,00	-7.4	4.1	-2.5	-3.0	-2.2	
Percent	Years	1967-1972	1967-1972	1967-1972	1967-1972	1966-1972	1966-1972	1966-1972		1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	
	Original Volume (dam3)	3 520	14 353	27 694	36 158	51 465	44 186	40 975	218 300	3 520	14 353	27 694	36 158	51 465	44 186	40 975	218 300
	Range	32	31	30	59	28	27	56	Total	32	31	30	59	28	27	26	Total

# 4.4 Reservoir

## 4.4.1 <u>Total Storage Changes</u>

As the longitudinal profile (Figure 32) shows, by 1972 there had been deposition along the whole reservoir reach. These deposition rates show a general decline with increased distance from the source, however there are some areas that do not follow this trend (Table 18). Prior to 1968 the reservoir was being filled and therefore the depositional patterns are somewhat distorted. Also structures such as the Saskatchewan Landing Bridge affected the sedimentation pattern as noted by the changes at Range 20. While channel morphology at the individual ranges have also affected the measured sedimentation rates. An example is Range 18 which is a very narrow cross section with steep banks susceptible to slumping. This high bank material contribution is what is responsible for this high sedimentation rate. Range 15 is an example of a range which is affected by the local input of Swift Current Creek.

Changes between 1972 and 1975 indicate the sedimentation pattern due to a more regular schedule of reservoir operations. The hydrographic survey results show that during this period significant changes were only measured at the ranges upstream of Range 20.

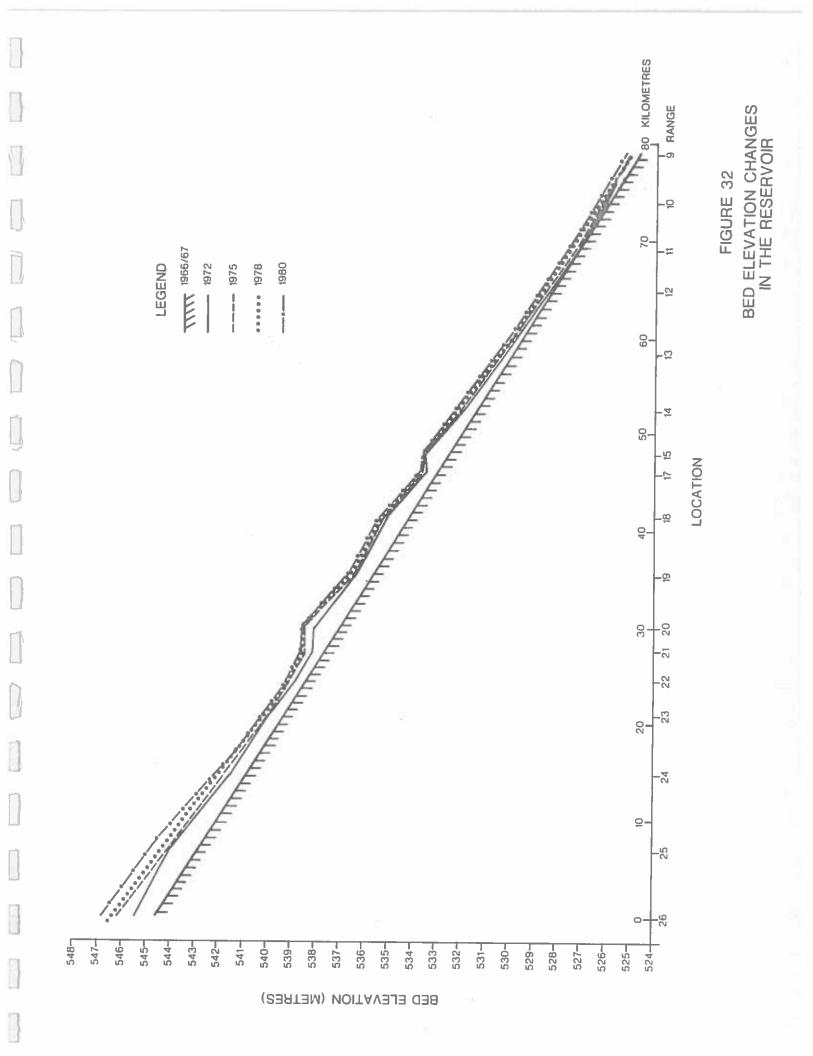


TABLE 18

				Mean Cha	anges (Me	etres) an	nd Elevat	ions (Me	tres) for	Mean Changes (Metres) and Elevations (Metres) for Reservoir Ranges	ir Ranges					
Range	1966	1967	1968	1969	1970	1261	1972	1973	1974	1975	1976	1977	1978	1979	1980	Kuiper's Forecast
26 Mean Change			+0.28				+0.87			+1.41			+1.60			+2.58
Elevation	544.65		544.93				545.52			546.06			546.25			547.23
25 Mean Bed Change						+1.07						+1.76				+2.34
Elevation	542.89					543.96						544.65				545.23
24 Mean Bed Change							+0.70			+0.94						+2.11
Elevation	540.86						541.56			541.80						542.97
23 Mean Bed Change			+0.14				+0.42			+0.50			+0.63			+1.41
Elevation	539,34		539.48				539,76			539,84			539.97			540.75
22 Mean Bed Change						+0°39									+0.61	+0.94
Elevation	538.41					538.80									539.02	539,35
21 Mean Bed Change		+0.20					+0.36			+0.68			+0.63			+0.94
Elevation	537.67	537.67 537.87					538.03			538,35			538.30			538.61
20 Mean Bed Change							+0.46			+0.81			+0.88			+0.71
Elevation	537.07						538.16			538.51			538, 58			537,78
19 Mean Bed Change						+0.69						+0.92			+0.93	+0.71
Elevation	535.69					536.38						536.61			536.62	536.40

TABLE 18 (cont'd)

				Mean Ch	anges (Me	tres) an	d Elevat	ions (Me	tres) for	Mean Changes (Metres) and Elevations (Metres) for Reservoir Ranges	r Ranges					
Range	1966	1967	1968	1969	1970	1971	1972	1973	1974	1975	1976	1977	1978	1979	1980	Kuiper's Forecast
18 Mean Bed Change						+0°98			+1.02						+1.12	+0.71
Elevation	534.11					535.09			535.13						535.23	534.82
17 Mean Bed Change		+0.43					+0.26			+0.46			+0.53			+0.70
Elevation	533.00	533.43					533.26			533,46			533.53			533.70
15 Mean Bed Change						+1.08						+0.94				+0.70
Elevation	532.44					533.52						533,38				533.14
14 Mean Bed Change							+0.75			+0.84						+0.70
Elevation	531.24						531.99			532.08						531.94
13 Mean Bed Change						+0.52									+0.86	+0.47
Elevation	529.76					530.28									530-62	530.23
12 Mean Bed Change															+0.72	+0.47
Elevation	528.18														528.90	528.65
11 Mean Bed Change							+0.07			+0.18						+0.47
Elevation		527.07					527.14			527.25						527,54
10 Mean Bed Change						+0.26						+0.30				+0.47
Elevation		525.83				526.09						526.13				526.30
9 Mean Bed Change							+0.63			+0.54					+0.81	+0.47
Elevation		524.53					525.16			525.07					525,34	525.00

By 1978 it had become apparent that most of the sediment load transported by the South Saskatchewan River settled out of suspension before it reached the Saskatchewan Landing Bridge. Range 25 appears to be the pivotal point for sedimentation with the overall maximum sediment deposition of any of the ranges upstream of the bridge.

The 1980 survey confirms that most of the loads are being deposited in the upper portion with minimal changes to the lower ranges, except where bank slumping provides material.

According to Kuiper (1962) the effects from the South Saskatchewan loadings would be negligible by Range 20. Measured changes confirm this, but show higher than anticipated sedimentation rates below Range 20 due to the bank material contribution.

Table 19 contains the volume losses due to sedimentation in the reservoir section. By 1972 there was 57 300 dam<sup>3</sup> of volume lost, which is approximately 0.6% of the total reservoir capacity. The 1975 results showed that a further 14 400 dam<sup>3</sup> were lost or 0.2% in total volume. The period 1975-1978 revealed that 13 700 dam<sup>3</sup> were infilled or a further 0.2% of total capacity. Limited data for 1980 made it impossible to properly estimate changes, but it is unlikely that the changes would be very significant. Therefore, from 1966 to 1980 about 1% of the total reservoir storage was determined to have been depleted.

TABLE 19

	Volume Change (dam <sup>3</sup> )	-1 707 -1 128 -502 -326 -203 -1 030 -1 138 -1 228 -1 228 -1 228 -1 328 -1 328 -1 328 -1 328 -1 328	
	Percentage Change	8.5000.00 0.4.2.5.000.00 0.4.2.5.00000	
	Years	1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978	
ir Ranges	Volume Change (dam <sup>3</sup> )	-2 276 -2 632 -301 -461 -634 -1 582 -1 201 -732 -1 201 -732 -1 201 -732 -1 201 -732 -1 550 -593 -1 550 -257	
for Reservoir Ranges	Percentage Change	4.00-1-5-6-10-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-	
ercentage and Volumetric Changes	Years	1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975	
age and Volum	Volume Change (dam <sup>3</sup> )	-6 829 -2 068 -1 606 -1 221 -2 245 -5 203 -1 747 -7 007 -6 164 -6 124 -6 124 -7 3 593 -5 300	
Percent	Percentage Change	0 4 8 8 8 8 4 4 4 4 4 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6	
	Years	1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972	
	Original Volume (dam <sup>3</sup> )	56 910 47 004 33 466 27 133 25 358 51 025 81 303 52 955 76 753 140 089 184 383 197 560 256 670 322 183	
	Range	255 23 22 21 20 20 13 13 11 10 10 10	

This 79 km of reservoir bed had an original slope of 0.00025 but because of higher sedimentation rates in the upper ranges the overall slope has increased over time. In 1972 and 1975 the slope had increased to 0.00026 and in 1978 and 1980 it was calculated to have been 0.00027.

#### 4.4.2 Dead Storage Changes

Dead storage losses are not considered important to reservoir operations until that capacity is lost and live storage is directly affected. Figure 33 illustrates the progressive loss of dead storage downstream with time. By 1972, significant losses are noticeable in some areas, especially the first couple of ranges where all the dead storage was lost at Range 26 and 95% of it at Range 25. By 1975, all of the dead storage for Range 25 had been lost and Range 24 had undergone significant change. Below these first few ranges the losses are not as significant as expected and at the wider ranges the changes are minimal. Range 14 marks the point where changes in the dead storage have been negligible since 1972, as limited sediment is transported that far downstream. Between 1975 and 1978 only Range 24 showed significant losses. This lessening of dead storage loss in the latter years is related to the decrease in sediment loadings for the South Saskatchewan River.

From Table 20 the dead storage calculated to have been lost by 1972 was 50 800  $\rm dam^3$ , or 1.0% of the total dead storage capacity of the reservoir. Between 1972 and 1975 only 10 000  $\rm dam^3$  were determined to

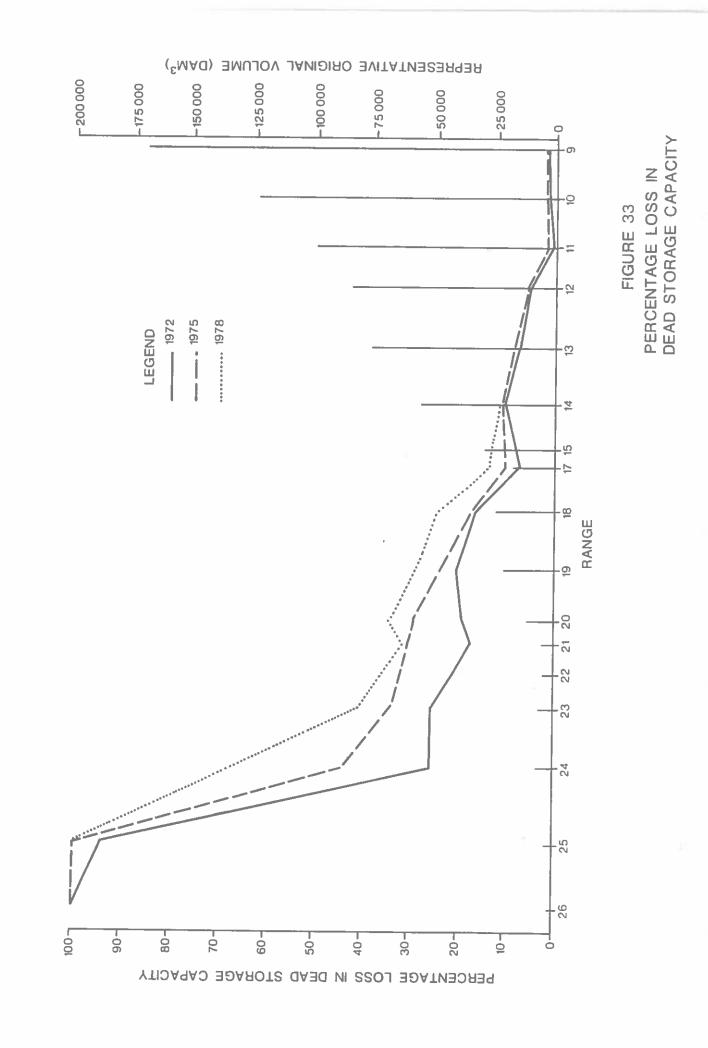


TABLE 20

	ge Volume Change (dam <sup>3</sup> )	ı	ı	-1 433	-487	-103	-51	-651	-861	-1 494	-712	-897	-573	1	4		1	•	-7 300
	Percentage Change			-22	80	-2	-	9-	4-	9-	4-	<sub>ව</sub>	-	1	i		ř	A	
anges	Years	1975-1978	1975-1978	1975-1978	1975-1978	1975-1979	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	1975-1978	
Reservoir R	Volume Change (dam <sup>3</sup> )	ı	-192	-1 303	-487	-567	-721	-976	-861	-249	-534	-894	-573	-773	-859	-1 011	1	1	-10 000
d Storage for	Percentage Change	ŧ	5-	-20	8-	-11	-14	6-1	-4	-	<u>د</u> ا	£.	-	7	_	<del>-</del>	1	ı	
and Volumetric Changes in Dead Storage for Reservoir Ranges	Years	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	1972-1975	
Volumetric	Volume Change (dam <sup>3</sup> )	-835	-3 640	-1 628	-1 584	-1 134	-927	2 170	-4 303	-4 344	-1 425	-2 681	-6 304	-6 186	-5 154	-1 011	-2 513	-5 110	-50 800
Percentage and	Percentage Change	-100	-95	-25	-26	-22	-18	-20	-20	-17	80	6-		80 1	9-	_	-2	r7	
	Years	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1966-1972	1967-1972	1967-1972	1967-1972	
	Original Volume (dam <sup>3</sup> )	835	3 832	6 513	6 093	5 157	5 149	10 848									125 645		
	Range	26	25	24	23	22	21	50	19	18	17	15	14	13	12	11	10	6	TOTAL

have been infilled or 0.2% of the total dead storage. During the period 1975 to 1978 the dead storage loss was estimated at 7 300 dam<sup>3</sup> or 0.1%. The period from 1978 to 1980, due again to the limited hydrographic data and its quality, could not be properly evaluated. Most of the cross sections however suggest minimal change. Therefore, dead storage loss due to sedimentation between 1966 and 1980 was approximately 1.3%. These results show that very little sediment is transported far into the reservoir and that changes will be significant only in the upper portion for many years to come.

# 4.4.3 Live Storage Changes

According to Van Everdingen's (1968) calculations the live storage would increase due to bank erosion, by as much as 7.4% within the first ten years of the reservoir life. The hydrographic data findings do not support this forecast, however, these data do not reflect changes in the lowest section of the reservoir. Bank erosion and slumping are noticeable along the length of the reservoir. However, because of the steep banks a considerable amount of bank material is dumped into the live storage resulting in a loss of live storage at most of the ranges. The terrestrial photogrammetry approach would have provided the best estimates, but unfortunately this portion of the program was not pursued.

By 1972, it was estimated that 7 300 dam<sup>3</sup> of bank derived sediment had reduced the total live storage capacity by 0.2% (Table 21). Between 1972 and 1975 there were considerably more ranges that showed erosion, however,

the net change for the reservoir was a 4 400 dam<sup>3</sup> further loss in storage capacity, amounting to 0.1%. During the period between 1975 and 1978 deposition at some the larger ranges was significant. A total loss of 6 500 dam<sup>3</sup> was determined to have reduced the live capacity by a further 0.2%. The few cross sections extracted from 1980 data revealed minimal change Therefore, it was calculated that about 0.5% of the total live storage was lost due the bank slumping, since the reservoir was established.

TABLE 21

Volumetric Changes in Live Storage for Reservoir Ranges

Range	Years	Volume Change (dam <sup>3</sup> )	Years	Volume Change (dam³)	Years	Volume Change (dam <sup>3</sup> )
25 24 23 22 21 20 19 18 17 15 14 13 12 11	1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1966-1972 1967-1972	-3 189 -440 -22 -87 -214 -75 -845 -970 -322 -82 +140 -821 -970 +125 -1 080 +1 566	1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975 1972-1975	-2 084 -1 329 +186 +106 +87 -606 -340 -483 +481 +510 -408 +35 +266 -539 -257	1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978 1975-1978	-1 707 +305 -15 -223 -152 +192 -169 +356 +447 -331 +433 -1 106 -790 -1 328 -770 -1 611
TOTAL	130131316	-7 300	1316-1313	-4 400	1373-1370	-6 500

## 4.5 <u>Sediment Sampling Analyses</u>

### 4.5.1 Suspended Sediment Sampling

The suspended sediment sampling program provided only limited information because it had been improperly conducted. This was because sampling was undertaken at the same time as the hydrographic surveys, which was after the reservoir had reached FSL. By this time, most of the sediment had settled out of suspension and the concentrations obtained were very low. Only in 1972 was the suspended sediment sampling program conducted at the optimum time - when the reservoir was being filled by the peak discharge. Concentrations at this time are normally sufficiently large enough to perform particle size analyses and provide information on settling characteristics of the incoming sediment loads. The 1972 data revealed that almost 85% of the suspended sediment load had settled out of suspension by Range 21 (Table 22). Furthermore, almost all of the sand component of suspended sediment had been deposited by Range 30. By Range 26 most of the suspended sediment was comprised of clay. The 1972 peak discharge had a return period of 2.43 years, which is just more than the mean annual flood and indicative of the transport capacity of a that magnitude of flood. If this sampling program had been conducted properly over the years, it could have provided this kind of information for the range of peak discharges - valuable supportive information to the hydrographic surveys.

TABLE 22

Concentration and Particle Size Data for 1972 Suspended Sediment Samples

Range	Suspended Sediment Concentration (mg/	Percentage L) Sand	Percentage Silt	Percentage Clay
36	705	49	34	17
33	490	23	45	32
30	326	1	50	49
29	266	1	40	59
26	260	1	26	73
21	111*			
17	24*			
14	51*			
9	22*			

Source: Wiebe and Drennan, 1973

## 4.5.2 Bed material sampling

In most reservoir studies bed material data have seldom been collected, but rather estimated from empirically derived relationships. The most commonly used method of obtaining a density value for the deposited sediment is the Lane and Koelzer (1953) method, where the density is determined by taking into account only the particle size of incoming sediment, the method of operation of the reservoir, and time. The equation used is:

$$W = W_1 + B \log t$$

where: W = density of deposited material after t years

 $W_1$  = initial density

B = constant

t = time (years)

<sup>\*</sup> Insufficient concentration for particle size analysis

The initial density of the sediment is obtained by applying the percentage of each sediment fraction to a mean density value and then summing the three fractions. The required information is available in Table 23.

TABLE 23

Lane and Koelzer's Deposited Sediment Density Values (kg/m<sup>3</sup>)

Reservoir Operation	Sand		Stlt		Clay	
	Wl	В	WŢ	В	Wj	В
Sediment always submerged or nearly submerged	1 490	0	1 041	91.3	481	256.3
Normally a moderate reservoir drawdown	1 490	0	1 185	43.2	737	171.4
Normally considerable reservoir drawdown	1 490	0	1 265	16.0	961	96.1
Reservoir normally empty	1 490	0	1 313	0	1 249	0

Source: Lane and Koelzer, 1953

In most cases only particle size data were available for the bed samples, so therefore it was important to evaluate this computation method against actual measured values. Up to and including 1978 the sampling procedure had not changed, but in 1980 the collection technique and analysis procedures were modified. The result was that a significantly different set of data was obtained compared to previous years. Until the sampling program can be properly evaluated it was determined that these data be omitted. There have been 144 in situ densities collected from Lake Diefenbaker, which provides a good basis for comparison (Appendix J). Based on particle sizes, densities were computed using values for sediment that would always be submerged. As illustrated in Figure 34, the computed values were

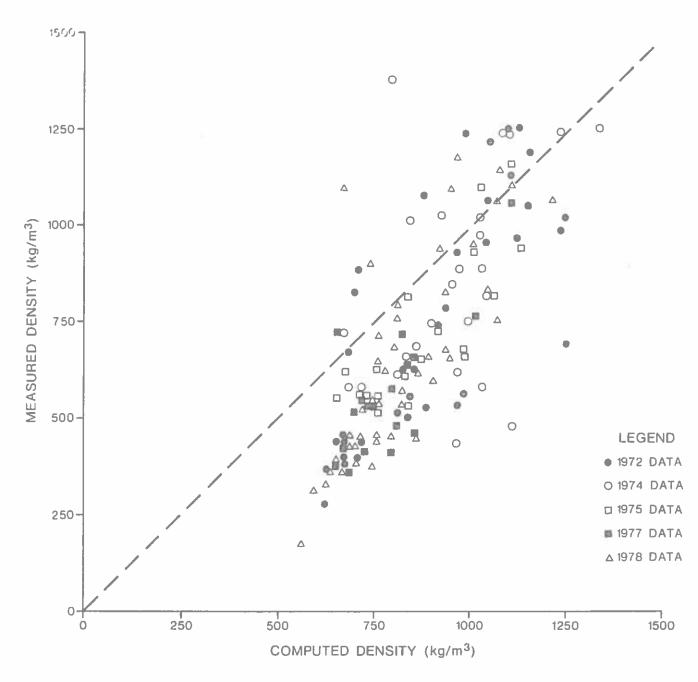


FIGURE 34
MEASURED DENSITIES OBTAINED FROM A
PHLEGER CORER VERSUS COMPUTED
DENSITIES BASED ON PARTICLE SIZES

significantly higher than those that were actually measured, suggesting that the Lane and Koelzer technique tends to overestimate. This weakness has been acknowledged in other studies (Lara and Pemberton, 1963; Vanoni, 1975).

To derive the most suitable density values, multiple regression correlation analysis was applied to the 144 bed material sample data. The equation derived was:

$$Y = 12553 \text{ kg/m}^3 - 123.5 \text{ kg/m}^3 (X_1) - 116.3 \text{ kg/m}^3 (X_2) - 110.8 \text{ kg/m}^3 (X_3)$$

R = .75

where:  $y = density (kg/m^3)$ 

X<sub>1</sub> = Percentage clay

X<sub>2</sub> = Percentage silt

 $X_3$  = Percentage sand

When the average density of each fraction is compared to Lane and Koelzer's (1953) values there is a marked difference for the clay fraction (Table 24).

TABLE 24 Deposited Sediment Densities (kg/m $^3$ ) Comparison

Fraction	Multiple Regression Equation	Lane and Koelzer
Pure Clay	203	481
Pure Silt	923	1 041
Pure Sand	1 474	1 490

The clay density value obtained from this study and others show that the Lane and Koelzer value is too high and may be due to some bias in their data. For the other two fractions there appears to be good agreement, especially so for the sand fraction.

This multiple regression equation was then used to determine densities where only particle size data were available. The densities or particle size data used were from samples that were collected in the thalweg. which best represent densities of the loads transported by the South Saskatchewan River. As Table 25 shows, by 1972 the sand component of the load was being deposited upstream of Range 26 and the wash load transported a considerable distance into the reservoir. The 1975 data revealed that sand had been transported as far downstream as Range 26 but past Range 21 mostly clays were being transported and deposited. The reason that the sand component had been transported as far into the reservoir was because the 1975 discharge was an extremely high discharge. In 1978 the sand portion of the load was found to be deposited in the upper ranges with a major portion of the bed being comprised of clays. This is attributed to the considerably lower discharges recorded between 1975 and 1978 which were only capable of transporting finer material. The 1980 data revealed that the sand portion had been deposited within the first couple of ranges which appears to be the prevalent pattern.

TABLE 25

Bed
the
10
Sizes
Particle
and
$(kg/m^3)$
Density

	Density*	1 474 C		785 C			453 C	387 C	417 C		396 C	
	: Clay		œ	36	200		99	92	17		74	
19	% Silt	E	A2	45	n n		33	22	28		52	
	% Sand	100	S	25.25	đ		-	2	-		-	
	Density*	1 474 C	1 384 C	737 M					705 M	721 M		240 M
1972	% Clay	ı	4	42					67	19		48
	% Silt	ı	7	99					29	37		48
	% Sand	100	89	2					4	2		4
	% Clay	1	40	60			20		8		01	25
	% Silt	2	34	S			49		61		17	10
	% Sand	98	36	87			31		9		69	65
	Range	32 30 30	28	26 26	24	22	20	19	17	<u> </u>	212	2 6

\* M - Measured with Phleger Corer C - Calculated using multiple regression equation

TABLE 25 (cont'd)

				Density	(kg/m³) and	d Particle	Density $(kg/m^3)$ and Particle Sizes of the Bed	Bed	
lange	% Sand	1978 % Silt	% Clay	Density	Sand	1980 % Silt	% Clay	Density	
32	100	1	1	1 474 C	100	ı	1	1 474 C	
30	80	12	60	1 305 C	99	18	16	1 171 C	
29 28	79	Ó	12		ហ	40	52		
27	m	51	46						
26	2	41	57	432 M					
25					4	20	46	614 C	
24	4	59	37						
23	2	38	9	400 M					
22					m	44	53	558 C	
21	6	35	56						
20	٣	33	64	464 M					
19									
18					٣	23	74	407 C	
17	-	26	73	352 M					
15					ı	24	92	376 C	
4									
13	2	28	70	368 ₩	_	31	68	439 C	
12					2	22	9/	387 C	
=									
0	(	(	4	1					
6	2	14	84	329 M					

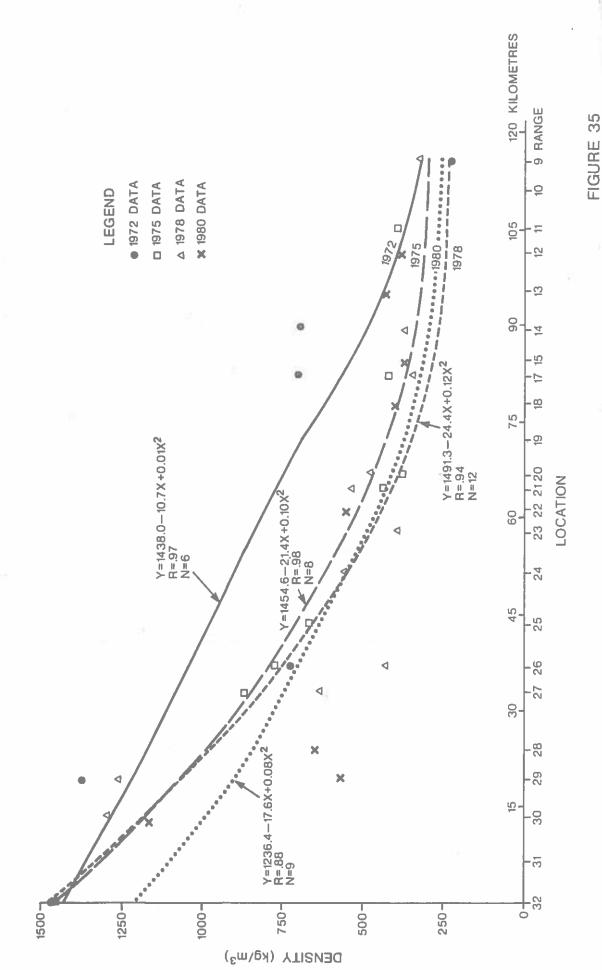
\* M - Measured with Phleger Corer C - Calculated using multiple regression equation

over time. The best data fit was determined to be a quadratic relationship. The 1972 developed relationship was slightly different because filling of the reservoir had distorted the density pattern. The 1975 and 1978 relationships revealed similar sedimentation patterns. By about Range 20 the relationships are asymptotic, as only the finest materials are carried beyond this point. In 1980 the densities were lower in the upper portion than in previous years due to more fines being deposited.

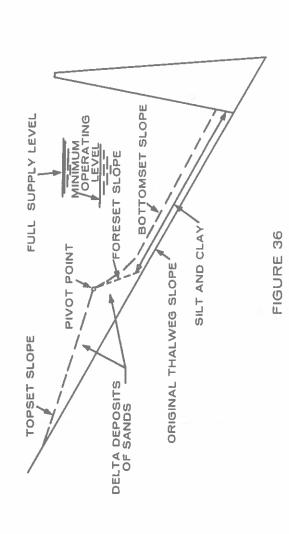
## 4.6 <u>Delta Formation</u>

The sedimentary structure of deltas has been well documented (Walker, 1979; Reineck and Singh, 1980). The delta is comprised of three types of deposits: topset, foreset, and bottomset (Figure 36). However, because Lake Diefenbaker has such a large drawdown range, causing sediment to be deposited over 37 km of reservoir during filling each year, the beds are not readily identifiable by the bed material composition alone. Therefore, the beds have been identified based on the measured deposition rates at each of the ranges and related to a classical reservoir delta structure.

From the hydrographic data, Range 25 was determined to mark the pivot point or in other terms the maximum extent of the topset beds into the reservoir. Since Range 32 marks the beginning of reservoir conditions, the topset beds can be said to extend from Range 32 to Range 25. The original



BED MATERIAL DENSITY RELATIONSHIPS



A CLASSICAL DELTA PROFILE

slope for this reach was 0.00022 and by 1980 it had been reduced to 0.00018. Reservoir studies in the United States have shown that topset beds in reservoirs normally stabilize once the slope has been reduced to half of the original slope (Shen, 1971; U.S. Bureau of Reclamation, 1977). Therefore, in the case of Lake Diefenbaker, the slope should become stabilized at about 0.00011.

Another approach used to estimate the topset slope is that of determining the theoretical slope at which no bed load will be transported (Vanoni, 1975). Using the Scholitsch formula for zero bed load transport this theoretical slope was calculated. The data used were from the Lemsford station. The equation is:

$$S = \begin{pmatrix} \frac{.00021 \text{ DB}}{Q} \end{pmatrix}^{3/4}$$

where: S = slope

D = mean bed material particle size (mm) 0.359

B = bankfull width (ft) 830

 $Q = mean \ annual \ flood \ (ft^3/s) 51 000$ 

Note: data are in both metric and imperial units.

A slope of 0.00004 was determined to be the minimum slope that would ensure no bed load transport. This theoretical value is considerably lower than the empirical findings and probably indicates the lowest limit that the slope might reach.

As noted from both the suspended and bed material data, Range 21 is the beginning of the bottomset deposits and marks the point beyond which only clays are transported. Therefore, the foreset bed slope can be considered to extend from Range 25 to Range 21. Originally the slope for this reach was 0.00025 but has increased to 0.00031 due to deposition. Foreset slopes are normally steep, some 6.5 times greater than topset slopes (U.S. Bureau of Reclamation, 1977). In Lake Diefenbaker, the foreset slope was less than twice that of the topset which shows the effects from factors such as: the large water level drawdown, small sediment loads and an original low slope.

The bottomset slopes are normally very gentle and do not show much change which is the case for Lake Diefenbaker. The original slope for the reach from Range 21 to Range 9 was 0.00025 and by 1980 it had only changed to 0.00026.

## 4.7 <u>Temperature Data Analysis</u>

Temperature data are useful in determining the circulation pattern of inflows into the reservoir, which in turn affects the sedimentation

patterns. Because Lake Diefenbaker is long, narrow, and relatively shallow for a great distance, the inflows from the South Saskatchewan River become well mixed with the existing reservoir water and the suspended sediment becomes relatively evenly dispersed. It is not until the ranges downstream of Swift Current Creek that water stratification is identifiable. The thermal structure of the lake water, as expected in a relatively deep lake, was found to be relatively stable from year to year at the deeper ranges. However, since sedimentation is only significant upstream, the temperature gradient noted in these lower ranges do not affect the sedimentation pattern.

# 4.8 <u>Hydrometric Station Loads Compared to the Measured Deposited Load</u> <u>Estimates</u>

A comparison of the hydrometric station loads to the measured deposited loads obtained from the surveys showed good agreement, considering the many factors/errors involved in obtaining deposited load estimates. For the period 1966 to 1972, a total load of approximately 47 000 000 t was transported into Lake Diefenbaker by the South Saskatchewan River and Swift Current Creek. For that period the deposited load was calculated to have been in the order of 52 000 000 t - a difference of only +11% (Table 26). From 1972 to 1975, some 20 000 000 t were transported into the reservoir, while the measured trapped load was calculated to have been 19 000 000 t, or only -5% difference. The period for which the results did not agree well was from 1975 to 1978. The sediment loading was considered to be about 6 000 000 t, but the trapped load was estimated to be some 13 000 000 t.

Measured Deposited Loads

	es)	82242627	0	
	Load (Tonnes)	-706 678 -1 442 372 -689 122 -1 481 244 -1 792 082 -3 778 429 -2 209 832 -1 622 867 -1 624 023 -1 624 023 -1 624 023 -1 624 023 -1 624 023 -1 624 023 -1 624 023 -1 625 863 -1 624 023 -1 625 863 -1 6	-19 000 000	
	1975 Density (kg/m³)2	1 454 1 322 1 184 1 078 1 078 805 805 713 713 855 474 474 432 369 369 312 312 309 305		
Loads	Volume (dam³)	-1 091 -1 374 -1 374 -1 374 -2 276 -2 276 -2 276 -2 276 -3 201 -1 201 -2 34 -2 38		
Measured Deposited Loads	Load (Tonnes)	+227 215 +216 471 -322 720 -4 112 372 -5 489 187 -4 447 928 -6 774 697 -1 376 409 -1 376 409 -1 376 409 -1 735 469 -1 674 459 -3 717 037 -3 460 163 -1 674 459 -3 587 758 -2 804 928 -3 587 758 -1 674 459 -1 674 459 -1 674 220 -1 358 220 -1 358 220	-52 000 000	
	1972 Density (kg/m³)1	1 438 1 370 1 296 1 236 1 185 1 095 1 061 1 061 1 061 2 773 773 773 773 773 773 773 773 773 773		0.01 x <sup>2</sup> 0.10 x <sup>2</sup>
	Volume (dam3)	+158 +158 -249 -3 327 -4 632 -4 192 -2 068 -1 221 -1 221 -1 747 -2 763 -5 203 -1 747 -7 007 -3 593 -3 593		Y = 1438.0 + 10.7 X + Y = 1454.6 - 21.4 X +
	Year Range	33 33 33 25 25 27 27 28 11 11 11 12 11 10	TOTAL	NOTE: 1 Y

TABLE 26 (cont'd)

	Loads
,	Deposited
	Measured

	Load (Tonnes)	+17 305 -404 972 -1 712 729 -1 207 213 -793 621 -655 076 -373 118	-5 000 000
	1980 Density (kg/m³)4	1 236 1 128 1 014 927 857 741 700	
LUGUS	Volume (dam³)	-1 689 -1 302 -926 -884 -533	
Medsured Deposited Codos	Load (Tonnes)	-68 589 -730 336 -2 426 134 -1 576 925 -1 248 451 -1 079 416 -682 991 -1 118 139 -621 551 -622 991 -1 18 139 -424 478 -145 403 -826 691 -358 487 -77 649 -37 541 -37 242 -195 930 -322 720 -184 639 -380 214	-13 000 000
	1978 Density (kg/m3)3	1 491 1 340 1 184 1 064 970 814 758 655 655 655 251 266 253 253 253 253 253 253	
	Volume (dam³)	-46 -545 -2 049 -1 482 -1 287 -1 326 -901 -1 128 -502 -203 -459 -1 030 -1 138 -1 106 -1 106 -1 328 -1 106 -1 106 -	
	Year Range	32 33 30 24 25 25 25 27 20 11 11 11 10	TOTAL

3 Y = 1491.3 - 24.4 X + 0.12 X<sup>2</sup> 4 Y = 1236.4 - 17.6 X + 0.08 X<sup>2</sup>

TOT AL NOTE:

This shows that a 100% difference is possible when comparing these two methods. Because there are so many factors/errors involved it is extremely difficult to assess where the weaknesses may lie. From 1978 to 1980, a further 6 000 000 t had entered the reservoir and estimates show that by Range 26, 5 000 000 t had been deposited.

Therefore, in total, from 1966-1980 the South Saskatchewan River transported 79 000 000 t and based on the hydrographic surveys 89 000 000 t were calculated to be deposited. This is only a +13% difference. These results show that it is possible to achieve fair agreement between station derived loads and measured deposited loads if a comprehensive reservoir program such as the one for Lake Diefenbaker is implemented.

## 4.9 Swift Current Creek

The hydrographic surveys conducted in 1972 and 1978 were to assess the Swift Current Creek source of sediment. As noted in Chapter III, the sediment loads transported by Swift Current Creek in latter years were significantly smaller. In fact, between 1972 and 1978, only 65 000 t of sediment had been transported into the reservoir by this source. Compared to the long term mean of 41 000 t per annum, this period studied was not characteristic of normal loadings and the results should be taken in this context.

The 1972 contour map (Figure 37) which covers an area of 2.4  $\rm km^2$  reveals the submerged topography of this area. The arrow points to where Swift Current Creek flows into the reservoir. The side where Swift Current

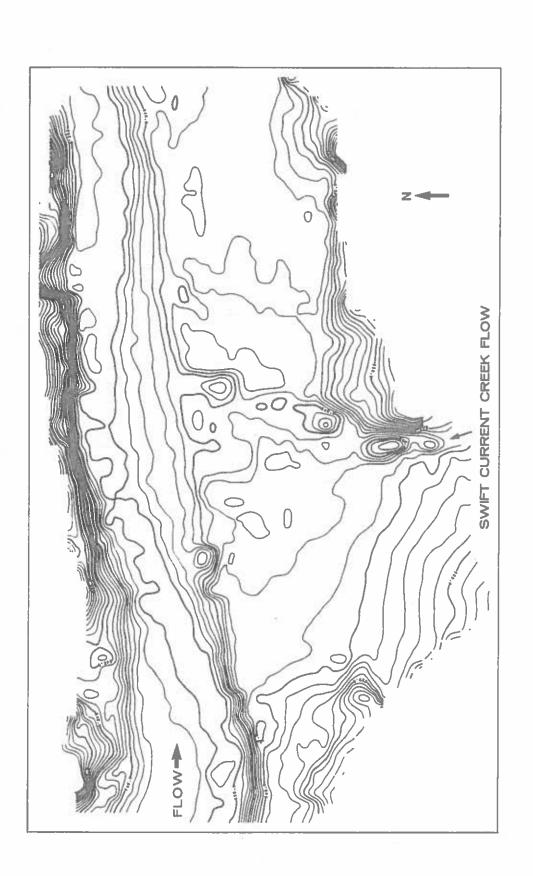


FIGURE 37
SWIFT CURRENT CREEK AREA
1972 CONTOURS
CONTOUR INTERVAL = 1.0 METRE

009

300

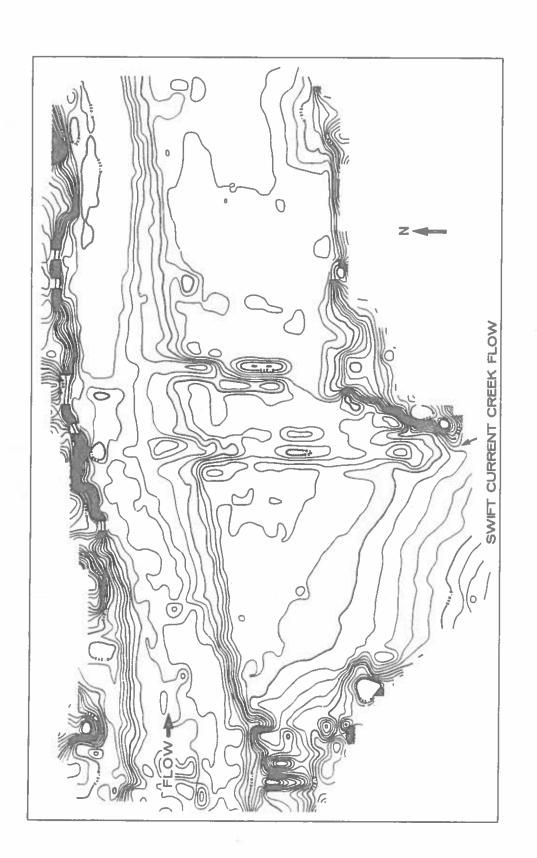
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METRES

Creek drains into the reservoir is marked by a large flood plain, and the channel used by pre-reservoir flows can be identified, as well as the old South Saskatchewan River channel. The north side of the area has steeper slopes. By 1978, (Figure 38) many morphological changes were noticeable. The most significant change was where a bank had slumped across the creek's channel causing the flows to cut a wider channel. Bank slumping was noticeable along the length of the shoreline and is best depicted in Figure 39 of contour differences between 1972 and 1978.

An examination of the bed material samples collected from this reach (sample identifer SCC in Appendix K) reflect the changes that have been occurring. In 1972, the average bed material sample was 71.5% silt and clay, but by 1978 silt and clay made up 90.9% on average. This increase in fines can be attributed to the finer material that was transported by Swift Current Creek, as well as fines derived from bank erosion and slumping. The mean density of the deposited material in 1978 was determined to be only 565 kg/m<sup>3</sup>, a very low density.

The capacity table (Appendix L) revealed that 1 200 dam<sup>3</sup> of sediment had been deposited in this area - 180 dam<sup>3</sup> in live storage and the remaining in the dead storage zone. Based on the average density, this volume was converted to a load of approximately 672 500 t. The 65 000 t estimated from Swift Current Creek during this period is to only 10% of the total value. This study points out that the bank material contribution was the most significant source of sediment in this particular area of the reservoir.

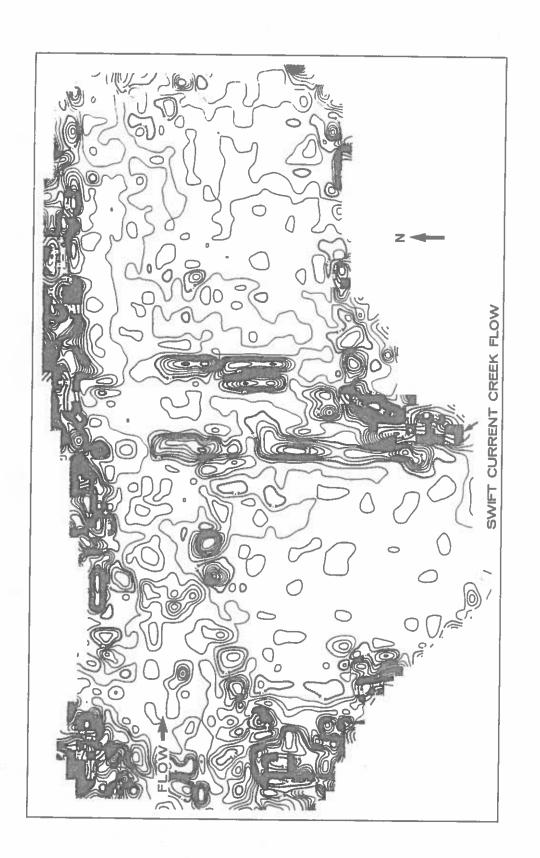


SWIFT CURRENT CREEK AREA 1978 CONTOURS CONTOUR INTERVAL = 1.0 METRE

000

900

METRES



SWIFT CURRENT CREEK AREA 1972-1978 CONTOUR DIFFERENCES CONTOUR INTERVAL = 0.5 METRE

METRES

#### 5. <u>Summary and Recommendations</u>

Based on Lemsford station data the median daily discharge of the South Saskatchewan River is 146 m<sup>3</sup>/s. June accounts for 25% of the total annual flow, most of it originating from mountain runoff. Hydrometric stations located further upstream were used to reconstruct the flows after 1970 for the discontinued Lemsford station, thereby, ensuring a complete period for analysis. Based on 69 years of record the South Saskatchewan River, mean annual flow is 8.3 million dam<sup>3</sup>. Since filling of the reservoir began in 1964, total annual flows were representative of the long term range of flows.

The estimated average annual suspended sediment load based on data from 1962 to 1980 was 5.4 million t, most of which was transported by the mountain flow freshet. Depth integrating samples collected at Lemsford indicated that on average, sand made up 21% of the load, silt 41% and clay 38%. Bed material samples were found to be comprised mainly of sand (87%). The sediment transport relationships were characteristic of large rivers with a high wash load contribution.

Swift Current Creek's mean annual flow was calculated to be 59 000 dam<sup>3</sup>, but for the period under study (1972-1978), flows were significantly less than the long term mean. This in turn severely reduced the sediment loadings. The sediment data showed considerable variability, which is considered common for streams of such a flashy nature.

The upstream ranges (Range 38 - Range 32) had been scoured based on the 1972 hydrographic survey results. This scouring was attributed to at least two major ice jams which had developed in this reach. It was estimated that 4 500 dam<sup>3</sup> of material were eroded from the main channel between 1966 and 1972 due to these ice jams. The maximum depth of scour measured was 1.3 at Range 37. Further bed degradation was observed for 1975 at most of the ranges. The 1978 and 1980 data indicated that infilling was occurring and that slope conditions were beginning to approximate those that had been there prior to the ice jams.

The drawdown reach (Range 32 - Range 26) has been continuously aggrading over the years with the maximum deposition being recorded as 1.9 m at Range 27 in 1980. The rate of deposition was found to be considerably less than had been previously forecasted by Kuiper (1962). By 1980, 19% of the total storage volume of this 37 km-long reach had been lost due to sedimentation. This in turn represents a loss of just less than 1% of the total live storage or 0.4% of the total reservoir volume. The slope of the bed in this reach was significantly reduced due the sediment loads deposited in the lower portion of the reach.

The remaining ranges are located within the reservoir. This analysis however has been limited to ranges upstream of Range 9, because by Range 9 the changes were negligible and there was concern over the

quality of the data at these larger ranges. The data that were used show that approximately 1% of the total reservoir capacity had been lost by 1980. Dead storage changes were only significant in the upper part of the reservoir, with 1.3% of the total dead storage capacity being filled in. Bank slumping was estimated to have reduced the total live storage by about 0.5% over the period studied.

An analysis of the Lane and Koelzer method of deriving densities from particle size and reservoir operation was conducted because for some years only particle size data were available. This method of computing densities from particle sizes however was found to overestimate, especially for the clay fraction, based on multiple regression analysis of 144 in situ bed material samples collected from Lake Diefenbaker. The bed material samples also revealed that the sand portion of the load was being deposited in the upper ranges of the drawdown reach and by Range 21 mostly clays were being deposited. The density to distance relationship was determined to be a quadratic and basically asymptotic beyond Range 21, where only fines were being deposited.

The delta beds were distinguished based on the bed composition and sedimentation rates measured at the ranges. It was determined that the topset beds extended from Range 32 to Range 25 and had a slope of 0.00018 based on the latest hydrographic data. The topset slope should stabilize at about 0.00011 based on empirical findings from other

reservoir studies. The foreset slope extended from Range 25 to Range 21 and had a slope of 0.00031, which is not very steep in comparison to other reservoir studies. The large water level drawdown, relatively small sediment loads and an originally low bed slope were determined to be responsible for affecting the development of the delta. The bottomset beds extended beyond Range 21, with a slope of 0.00026.

The temperature data revealed that the temperature gradients would not directly affect the sedimentation pattern in the reservoir, as water stratification only existed in the lower portion of the reservoir, where there was minimal sedimentation.

Comparison of the hydrometric station measured sediment loads to the loads estimated from the hydrographic surveys revealed good agreement especially from 1966-1972 (11%) and 1972-1975 (-5%), but for 1975-1978 and 1978-1980 the agreement was not as good. Due to the many weaknesses/errors involved in estimating deposited loads, large discrepancies for small loadings using this method are not uncommon. However, for the total period (1966-1980) the difference between these two methods was good (13%).

Swift Current Creek's contribution of sediment was negligible between 1972 and 1978. Almost all the change to the area where Swift Current Creek drains into Lake Diefenbaker was determined to be from bank slumping. In fact, only 10% of the change that was measured was attributed to the creek's sediment contribution.

Finally, it should be pointed out that besides providing scientific information this project has been very important for the development and testing of new techniques and equipment. The experience and knowledge obtained from this project has been transferred to other projects, therefore providing a valuable contribution to this field.

### Recommendations:

- 1. There are some advantages to continuing this project even though the sedimentation rate has not significantly affected reservoir operations. These advantages are:
  - a) The project is providing scientific information on the processes affecting delta development, sedimentation patterns, bank erosion, etc., which is transferable to other sedimentation studies;
  - b) Since development around the reservoir has escalated in latter years, the site specific data have become increasing important for other projects (park development, locating marinas, pipeline proposals, etc.);
  - c) This project also provides an opportunity for the organizations involved to train personnel and expand their knowledge in conducting sedimentation studies.

- Certain changes should be made to the existing program to make it more cost effective and efficient. These changes are:
  - a) Increasing the survey interval from a three to a five-year return. Five years is considered the maximum extension so as to ensure continuity between surveys and for planning purposes.
  - b) Restricting the survey to those range lines upstream of the Saskatchewan Landing Bridge (Range 20), as almost all the load transported by the South Saskatchewan River is deposited by that point.
  - c) Special area studies (Elbow, Swift Current Creek Delta) do not have to be resurveyed as they show negligible sedimentation.
- 3. During the completion of the report it became apparent that certain data collection methodologies and techniques need to be reviewed and evaluated. Such as:
  - a) The Hydac/Hydra Systems need to be properly evaluated and the system errors quantified;
  - b) To compare and evaluate the cross-section vs. area methods of hydrographic surveying more data will need to be acquired;

- c) more comprehensive guidelines are needed for bed material sampling;
- d) The guidelines regarding suspended sediment sampling in reservoirs need to be reexamined.
- 4. To properly quantify bank erosion, which is significant, a study using state-of-the-art techniques (terrestrial photogrammetry, concentrated cross-sectional work) should be considered.
- 5. A numerical model, such as Hec-6 or Mobed should be run using the existing data base, prior to a resurvey, to ensure the cross section network is adequate.

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= Appendix A

Flow Duration of Daily Discharges
for the South Saskatchewan River
near Lemsford

MATER SUNVEY SE CANADA
23/05/02 + PAGE 2
SEDIMENT SURVEY SECTION

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Appendix B

Total Monthly Discharges at Lemsford,
Bindloss and Hwy 41, and Reconstructed
Monthly Discharges for Lemsford

SOUTH SASKATCHEAN RIVER NEAR LEMSFORD (STN. NO. 05HGOOl)

TOTAL MONTHLY DISCHARGE (M<sup>3</sup>/S)

DEC.		3 316	2 886	2 165	2 492	2 319	
NOV.		3 151	3 353	5 117	3 415	3 384	
100		4 283	3 056	8 045	3 803	3 441	
SEPT.		5 059	3 511	6 746	4 196	2 660	
AUG.		7 459	5 986	6 399	8 109	4 4770	
JULY		17 234	18 015	9 283	32 081	12 914	
JUNE		23 523	45 700	17 841	19 216	24 291	
MAY		12 727	24 181	5 623	16 91	10 925	
APR.		•	13 791	4 030	18 403	8 032	
MAR.		ŧ	5 117	4 718	4 874	3 226	
FEB.		ŀ	2 992	2 884	2 676	2 400	
JAN.		ľ	2 736	2 865	3 109	2 032	
	YEAR	1966	1967	1968	1969	1970	

Source: Environment Canada, Surface Water Publications

	DEC.		3 154	2 119	2 613	2 278	2 489	
	NOV.		3 747	3 390	5 210	3 062	3 405	
	OCT.		4 559	3 159	7 832	3 901	3 515	
	SEPT.	!	5 239	3 523	6 720	3 961	2 638	
	AUG.		7 586	5 734	6 470	7 264	4 808	
SCHARGE	JULY		17 495	17 248	9 665	30 707	12 676	
TOTAL MONTHLY DISCHARGE (M <sup>3</sup> /S)	JUNE		24 623	44 082	18 064	20 228	25 107	
TOTAL N	MAY		13 823	25 109	5 961	16 524	11 858	
	APR.		ŧ	12 735	3 773	17 452	8 041	
	MAR.		ı	4 861	5 578	4 607	3 243	
	FEB.		ı	2 902	2 626	2 663	2 224	
	JAN.			2 872	2 934	3 145	2 151	
	ı	YEAR	1966	1967	1968	1969	1970	

SOUTH SASKATCHEWAN RIVER AT HIGHWAY NO. 41 (STN. NO. 05AKOO1)
AND
RED DEER RIVER NEAR BINDLOSS (STN. NO. 05CKOO4)

Source: Environment Canada, Surface Water Publications

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TOTAL	

	JAN. FEB.	YEAR	971 (A) 2 353 3 231 (B) 2 291 3 176	972 (A) 1 892 1 832 (B) 1 827 1 767	973 (A) 2 803 2 828 (B) 2 745 2 770	974 (A) 2 416 2 767 (B) 2 355 2 708	975 (A) 2 374 1 869 (B) 2 313 1 804	976 (A) 3 852 3 733 (B) 3 801 3 681	977 (A) 2 369 2 822 (B) 2 308 2 764	978 (A) 1 666 2 014 (B) 1 600 1 950	979 (A) 2 017 2 041 (B) 1 959 1 983	980 (A) 1 430 1 804 (B) 1 366 1 744	
	MAR.		3 368	8 743 8 726	5 256 5 215	3 560	2 946 2 889	5 069 5 026	3 293	5 269	6 126 6 109	2 650 2 599	
	APR.		17 543 17 588	9 434 9 422	8 384 8 365	15 391 15 421	6 937 6 908	6 742 6 711	3 455 3 401	8 486 8 467	6 325 6 310	8 267 8 272	
	MAY		15 669 15 701	17 857 17 904	10 823 10 821	22 484 22 563	17 692 17 718	12 035 12 041	4 090 4 041	13 176 13 190	13 422 13 478	10 920 10 951	
()	JUNE		25 367 25 467	31 182 31 322	17 190 17 232	29 940 30 072	29 355 29 482	9 654 9 644	5 664 5 626	20 873 20 941	12 029 12 071	21 868 22 009	:
	JULY		9 486 9 474	18 333 13 383	10 097 10 090	15 129 15 157	16 540 16 578	8 965 8 950	2 877 2 819	12 149 12 156	4 733 4 702	6 280 6 265	
	AUG.		5 686 5 648	10 611 10 607	4 561 4 515	7 416 7 390	5 486 5 446	13 695 13 713	3 892 3 841	7 131 7 103	3 022 2 974	4 260 4 225	
	SEPT.		2 840 2 782	6 471 6 438	3 576 3 523	4 748 4 703	4 619 4 573	6 240 6 206	3 409 3 355	8 792 8 776	2 192 2 136	3 295 3 249	
	OCT.		3 048 2 991	5 647 5 609	2 803 2 745	3 514 3 461	4 582 4 536	3 908 3 857	3 144 3 088	6 737 6 706	2 038 1 980	4 712 4 681	
	NOV.		3 331 3 276	4 305 4 257	2 355 2 293	2 845 2 787	3 027 2 970	2 550 2 490	1 914 1 849	3 321 3 266	1 983 1 925	4 247	
	DEC.		1 825	2 193 2 130	2 995 2 938	2 360 2 299	3 238 3 183	2 122 2 059	1 807	3 426 3 382	1 381	2 704 2 653	

Total monthly combined discharge from Bindloss and Highway 41 Total monthly discharge near Lemsford (Y = 1.01x - 78.0) (B)

Appendix C

Frequency Analysis of the
South Saskatchewan River Flows

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											M = .1163E+03	5.8351 M = .3421E+03								
SOUTH SASKATCHEWAN RIVER	SAMPLE STATISTICS	= 8587851. S.B. = 3185038.8 C.S. = .9868 C.K. = 3.8894	SAMPLE STATISTICS (LOGS)	- 15.9020 S.D3590 C.S0644 C.K. = 3.0498	MIN = 3289421. SAMPLE MAX = 16290880. N = 69	PARAMETERS FOR CURBEL I A = .000000 U = 7161866.	TERS FOR LOGNORMAL M = 15.9020 S = .3590	TERS FOR THREE PARAMETER LOGNORMAL A = 510487. N = 15.8318 S = .3849	STATISTICS OF LUGIK-A)	= 15.8318 S.D. = .3849 C.S. =0104 C.K. = 3.0988	TERS FOR LOG PEARSON III BY HOMENTS A = .0116 B = .9641E+03 LOG(M) = 4.7565	FOR "OG PEARSON III BY MAXIMUM LIKELIHOGO A "0126 B 7980E+03 LOGIM)	DISTRIBUTION STATISTICS MEAN = 15.902U S.D. = .3564 C.S. = .0708	GUMBEL I LDGNORMAL THREE PARAMETER LDG PEARSON III	R DATA T ESTIM	3120000. 3200000. 3270000. 3270000. 3270000. 4490000. 6010000. 5960000. 5960000.	6060000. 8020000. 8030000. 5.03 4090000. 5.14 10900000. 5.13 12800000.	5-75 14500000 6-63 14700000 7-49 14600000 7-51	6.44 18600000. 7.64 18400000. 10.90 18800000. 11.80 18900000. 1	. 6.67 20300000. 8.98 2080000. 12.40 20700000. 13.90 20800000. . 6.92 22700000. 9.80 2330000. 14.80 23200000. 16.80 23300000.
		MEAN		MEAN	SAMPLE	PARAN	PARAMETER	PARAMETER		NEAN	PARAMETER	PARAMETERS	OISTR		RETURN PEKIUD ES	1.050 312 1.050 446 1.250 601		20.000 1440		500.000 22200000 500.000 22200000

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## Appendix D

Depth Integrating Particle Size Analysis
of Suspended Sediment Collected at the
Lemsford Station

SUSPENDED SEDIMENT SOUTH SASKATCHEMAN RIVER NEAR LENSFORD 0.6 SIZE ANALYSIS DEPTH INTEGRATING PARTICLE 23/08/82 , PAGE 1 EDIMENT SURVEY SECTION

STATION NO. 05HB001

PERCENT SILT CLAY 909 DSO 460.00 181 203 1150 1150 .10 0 7 50 7 4 50 e 90 4.6 4: -5 TOTAL D50 GD 0.015 0.002 0.003 0.002 MICROMETRES 1000 2000 512E, IN 250 500 1 00000000 66 400000 INDICATED 31 62 125 100 80 81 92 92 99 100 97 98 99 1001 THAN 16 3 FINER 4 らりようごうほうこうきゅう 4 とこれでしゅうりょうきょう 4 もっしょう 2 とっしょう 2 とっしゅう 3 とっしゅう 3 とっしゅう 3 とっしゅう 3 とっしゅう 3 とっしゅう 3 とうしゅう 3 とっしゅう 3 とうしゅう 3 とうしゅう 3 とうしょう 3 とうしゅう 4 とうしゃく 4 とりしゅう 4 とりしゃく 4 とりょう 4 とりょ PERCENT - 2 4 222 222 222 222 234 434 434 CONC. DISCHARGE (M3/S) II. I I IIII I TI. TT XX I I II 167 306 447 467 436 487 487 487 99.7 94.9 63.7 1100 IDENTIFIER TIME 1964 5961 9961 964 964 SAMPLE I 113 22 22 22 22 23 24 27 110 110 11 APPULL ARREST PECULIAL LILLARY PROFILE CON NATIONAL SERVICE CON NATIONAL MAY JUN JUN SEP

CONCENTRATION FOOTNOTE SYMBOLS

R - SAMPLES COLLECTED IN SEVERAL VERTICALS

K - SAMPLE(S) COLLECTED IN A SINGLE VERTICAL

FOOTNOTE SYMBOLS
- INSTANTANEOUS
- DAILY MEAN

DISCHARGE

GDG (GRADING) = SQUARE ROOT (D84/D16)

• - UNABLE TO COMPUTE DUE TO INSUFFICIENT DATA

• - NO SANDS (>0.062 MM)

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- SAMPLE(S) COLLECTED IN A SINGLE VERTICAL ×

GDG (GRADING) = SQUARE ROOT (DR4/D16)

\* - UNABLE TO COMPUTE DUE TO INSUFFICIENT DATA

\*\* - NO SANDS (>0.062 MM)

DAILY MEAN

6 I

	NDED SEDIMEN
SOUTH SASKATCHEWAN RIVER NEAR LEMSFORD	DEPTH INTEGRATING PARTICLE SIZE ANALYSIS OF SUSPENDED
ATER SURVEY	SEDIMENT SURVEY SECTION

STATION NO. 05HB001

SEDIMENT SU	RVEY SE	CTION	DEPTH IN	NTEGRAT	ING	PARTI	CLE	3715	ANAL	YSIS	OF S	USPENDED S	EDIMENT						
SAMPLE IDEN	TIFIER	DISCHARGE	ONC	PERCE	NTF	INER		OX.	CAT	15 0	4	N MICROME	RES	AL	SAN	٥	9	RCE	
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16 196	200	N	10	27			0-	8	•	6 2	ů	6	.01	•	.14	- 0	36	40	
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04 23 196	140	0	40	13		92	m	m	4	5	6		. 05	7.3	• 13		17	37	
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61	213	T 486	1 620 K	16	25	37	E	70 8	94 9	2 9	4 99	10C	0.015	2.6	0.125	1.9	25	50	16
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GDG (GRADING) - SQUARE ROOT (D84/D16) + UNABLE TO COMPUTE DUE TO INSUFFICIENT DATA

STATION NO. 05HB001	
SOUTH SASKATCHEWAN RIVER NEAR LEMSFORD	DEPTH INTEGRATING PARTICLE SIZE ANALYSIS OF SUSPENDED SEDIMENT
JF CANADA	72 73

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GDG (GRADING) \* SQUARE ROOT (D84/D16) • - UNABLE TO COMPUTE DUE TO INSUFFICIENT DATA

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SOUTH SASKATCHEMAN RIVER NEAR LEMSFORD		DEPTH INTEGRATING PARTICLE SIZE ANALYSIS OF SUSPENDED	
WATER SURVEY OF CANADA	23/08/82 , PAGE 5	SEDIMENT SURVEY SECTION	

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DISCHARGE FOOTNOTE SYMBOLS
BLANK - INSTANTANEOUS
H - DAILY MEAN

CONCENTRATION FOOTNOTE SYMBOLS

R - SAMPLES COLLECTED IN SEVERAL VERTICALS

K - SAMPLE(S) COLLECTED IN A SINGLE VERTICAL

GDG (GRADING) = SQUARE ROOT (D84/D16) \* - UNABLE TO COMPUTE DUE TO INSUFFICIENT DATA \*\* - NO SANDS (>0.062 MM)

Appendix E

Bed Material Particle Size Analysis
of Data Collected at the Lemsford Station

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\* - UNABLE TO CAMPUTE DUE TO INSUFFICIENT DATA

A PROBLEM STREET STREET											
	PARISE	PANTICLE SIZE ANALYSIS UF BEU MATERIAL	ANAL:	Y 52.5	UF dE	U MAI	EKIA-				
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0767 67		101	c	76	1,13	100			0.313	1.0	LANE
0/61 67			7.7	15	7.7.	ŕ'n	U U		0.346	5	LANE
		10	13	3.5	74	45	1.1		0.332	1.0	8M-54
0/67 67	*5	0 1	13	Loū					0.135	4.1	BB-54
0/67 6		To	97	d C		100			0.352	1.0	84-54
AJG 23 1970 1300	40	11	23	/ p	p }	~	D.C.		0.217	2.0	LANE

Appendix F

Monthly Suspended Sediment Loads
at Lemsford, Bindloss and Hwy 41,
and Reconstructed Monthly Loads
for Lemsford

SOUTH SASKATCHEWAN RIVER NEAR LEMSFORD (STN. NO. 05HB001)

TOTAL MONTHLY SUSPENDED SEDIMENT LOADS

DEC.	6 199	14 989	4 164	9 941	2 665
NOV.	10 101	15 025	38 451	32 323	9 119
OCT.	38 491	9 363	184 880	91 765	62 800
SEPT.	75 343	39 652	313 546	222 068	17 306
AUG.	230 290	45 456	101 600	92 118	68 681
JULY	250 120	517 630	398 870	825 390	854 620
JUNE	153 000 1 250	009	1 040 870	296 970 3	160
MAY	531 380 1 453	300 000 6 137	198 500 1 (	620 990 1 2	362 454 3 219
APR.		077 104 076 1 244 400 3	33 864	282 740	501 154
MAR.	1	104 076 1	65 110	99 058 3	7 800
FEB.	2	5 077	10 386	1 620	3 346
JAN.	i	4 957	20 589	2 563	2 822

Source: Environment Canada, Sediment Data Publications

		NOV.	6 802	12 376	18 662	7 776
		OCT.	26 051	8 323	123 381	45 612
		SEPT.	45 462	7 894	169 216 123	265 553
Y NO. 41	LOADS	AUG.	136 152	35 508	107 251	82 620
SOUTH SASKATHCEWAN RIVER AT HIGHWAY NO. 41 (STN. NO. 05AKOO1) AND AND RED DEER RIVER NEAR BINDLOSS (STN. NO. 05CKOO4)	TOTAL MONTHLY SUSPENDED SEDIMENT LOADS (TONNES)	JULY	716 430	393 399	384 696	641 090
HCEWAN RIVER AT HI (STN. NO. OSAKOOI) AND EER RIVER NEAR BIN (STN. NO. OSCKOO4)	SUSPENDED (TONNES)	JUNE	485 1 870 910 1 716 430	37 250	960 550	120 770 3
ASKATHCEW (STN RED DEER	MONTHLY	MAY	465 485 1 8	156 3 321 660 4 937 250	350	608 340 1 420 770 3 641 090
SOUTH S	TOTAL	APR.	- 46	4	9 005 204	370
		MAR.	1	43 756 1 11	19 874	3 167 145 874 3 038
		FEB.	1	3 791 4	3 858 1	3 167 14
		JAN.	ı	3 435	3 019	3 367

YEAR

9961

1968

1961

Source: Environment Canada, Sediment Data Publications

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3 587 081

427 867

549 419

2 663

3 444

1970

1969

4 609

DEC.

4 259

3 089

TOTAL MONTHLY SUSPENDED SEDIMENT LOADS (TONNES)

	JAN.	FEB.	MAR.	APR.	MAY	JUNE	JULY	AUG.	SEPT.	OCT.	NOV.	DEC.
YEAR												
1971 (A) (B)	3 122	14 742 2 937	7 903	4 018 543 4 234 955	680 781 706 941	1 991 590 2 092 466	152 781 148 852	57 013 47 618	13 595	10 629	7 803	2 107
1972 (A) (B)	2 065	2 233	631 533 654 885	384 704 393 987	1 332 177	3 136 169 3 302 286	604 780 626 607	143 947 139 507	56 986 47 589	12 137	6 167	3 304
1973 (A) (B)	3 096	3 716	29 841 18 897	498 602 514 377	282 203 285 644	1 143 210 1 195 728	204 802 203 831	75 765 67 439	37 125 26 596	7 365	2 448	2 601
1974 (A) (B)	3 136	6 152	9 593	3 816 723 4 021 631	2 409 250 2 533 932	2 645 230 2 783 363	353 873 60 342	73 136 64 660	37 835 27 347	14 230 2 396	3 678	2 108
1975 (A) (B)	2 052	2 597	4 315	834 824 869 764	1 368 190 1 433 532	5 605 907 5 912 799	681 993 708 222	51 092 41 359	20 742 9 279	10 105	902 9	5 469
1976 (A) (B)	11 001	8 471	57 064 47 672	182 979 180 764	560 566 579 873	211 903 211 336	196 200 194 738	920 235 960 043	26 618 14 433	7 898	2 289	1 316
1977 (A) (B)	2 491	3 630	6 755	65 189 56 620	199 750 198 491	161 090 157 627	17 202 5 538	38 047 27 571	164 390 161 115	37 949 27 361	6 527	2 210
1978 (A) (B)	2 323	5 122	234 431 235 149	643 167 677 183	789 710 822 078	1 537 810 1 612 820	205 315 204 373	178 628 176 165	452 384 465 525	31 966 21 143	8 1	ŧ 0
1979 (A) (B)	192	152	14 347 2 563	116 351 110 687	739 565 771 294	441 611 445 463	71 482 63 126	59 750 50 690	11 388	6 213	8 8	1 1
1980 (A) (B)	1 1 =	0 1	3 260	653 539 680 106	1 161 839 1 218 904	1 613 890 1 698 078	332 859 340 186	188 977 187 670	28 727 17 806	23 545 12 313	1 1	1 1

Total monthly combined suspended sediment load from Bindloss and Highway 41 Total monthly suspended sediment load near Lemsford (Y=1.06x-12.645.0) (B)

Appendix G

Flow Duration of Daily Discharges
for Swift Current Creek near
the Mouth and the Leinan Stations

AATEK SUMVEY DE CANADA 23/08/02 : PAGE 2 SEULMENT SUMVEY SECTIUN

SAIFT CURRENT CREEK NEAR THE MUUTH

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AATEK SURVET JF CANAUA 23/UN/42 , PAGE J SEUIMENI SURVET SECTION

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HATER SUNVEY OF CAHADA 23/UN/02 , PAGE 2 SEUTINY SEUTHENT SUNVEY SECTION

SMIFT CORRENT CREEK NEAR LEINAN

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MATCH SURVEY OF CANADA 23/Ud/82 , PAGE 3 SEUIMENT SURVEY SECTION

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## Appendix H

Depth Integrating Particle Size Analysis
of Suspended Sediment Collected from
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GDG (GRADING) = SQUARE ROOT (D84/D16)

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GDG (GRADING) . SQUARE ROOT (D84/D16) . - UNABLE TO COMPUTE DUE TO INSUFFICIENT DATA Appendix I

Measured Bed Material Density and
Particle Size for Samples Collected
from Lake Diefenbaker

No.	Year	Sample Identifier	% Sand	% Silt	% Clay	Measured Density (Kg/m <sup>3</sup> )	Calculated Density (Kg/m <sup>3</sup> )
1234567890112314516718901223456789011234567890112345678901233456789041234456789	1972 1972 1972 1972 1972 1972 1972 1972	R26 R17 R14 SCC 3 SCC 4 SCC 5 SCC 7 SCC 10 SCC 11 SCC 12 SCC 13 SCC 12 SCC 18 SCC 17 SCC 18 SCC 20 SCC 21 SCC 22 SCC 30 SCC 31 SCC 32 SCC 33 SCC 36 SCC 37 SCC 37 SCC 36 SCC 37 SCC 36 SCC 37 SCC 37 SCC 37 SCC 36 SCC 37 SCC 37 SCC 36 SCC 37 S	11 47 158 46 39 30 49 50 67 82 11 85 82 11 85 82 11 82 82 11 82 82 82 82 82 82 82 82 82 82 82 82 82	56 29 48 34 36 37 28 42 31 50 50 50 50 50 50 50 50 50 50 50 50 50	33 67 39 31 20 31 20 40 40 31 40 31 40 31 41 41 41 41 41 41 41 41 41 41 41 41 41	905 684 854 1 110 988 1 148 980 950 1 209 1 275 412 1 073 416 405 527 290 444 649 1 017 697 559 548 372 450 524 453 580 545 549 638 447 756 1 280 1 042 801 1 646 1 252 1 408 905 1 046 1 267 633 905 529 487 452 1 056 745 1 267	737 705 721 902 1 155 1 136 1 076 992 1 182 1 150 727 1 184 695 689 867 641 686 883 1 264 1 278 870 756 636 676 836 689 1 014 905 985 849 737 892 1 131 1 068 959 1 072 812 1 059 1 062 1 004 998 996 1 057 1 104 993 857 686 1 261

No.	Year	Sample Identifier	% Sand	% Silt	% Clay	Measured Density (Kg/m <sup>3</sup> )	Calculated Density (Kg/m <sup>3</sup> )
50 51 52 53 55 56 57 56 61 62 63 64 65 66 67 77 77 77 77 77 77 77 77 81 82 83 84 85 86 87 89 99 99 99 99 99 99 99 99 99 99 99 99	1974 1974 1974 1974 1974 1975 1975 1975 1975 1975 1975 1975 1975	62-5 62-6 62-8 62-10 63-2 63-3 D1 D2 D3 D4 D5 D6 D7 D8 D9 D10 D11 D12 D13 D14 D15 D16 D17 D18 D20 D21 D22 D23 D24 D25 D27 10C 12R 12C 12L 13R 19C 19L 22R 22C 22L	27 35 41 40 13 28 17 22 1 2 30 10 4 23 37 29 21 39 28 46 3 4 4 4 4 4 4 4 4 12 14 14 14 16 17 18 18 19 19 19 19 19 19 19 19 19 19 19 19 19	58 50 39 40 53 53 54 53 54 61 52 53 53 53 53 54 54 54 54 54 54 54 54 54 54 54 54 54	15 15 20 31 19 23 60 55 43 23 38 22 20 44 32 40 40 40 40 40 40 40 40 40 40 40 40 40	844 1 267 1 267 1 267 7 793 1 456 860 1 057 593 593 625 673 766 626 636 665 939 1 115 848 687 562 745 835 572 562 529 581 633 844 671 950 628 1 190 375 785 678 388 497 1 067 408 421 444 540 461 730 524 658	1 078 1 175 1 381 1 108 926 1 060 977 939 710 743 825 844 1 020 873 846 1 004 1 044 1 060 860 1 016 678 942 1 068 730 778 746 788 867 897 1 173 692 1 136 700 1 036 881 670 828 1 144 814 729 695 764 879 678 726 863

No.	Year	Sample Identifier	% Sand	% Silt	% Clay	Measured Density (Kg/m <sup>3</sup> )	Calculated Density (Kg/m³)
97 98 99 100 101 102 103 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 120 121 123 124 125 127 128 129 130 131 132 133 134 135 136 137 138 139 140 141 142 143 144	1977 1978 1978 1978 1978 1978 1978 1978	25R 25C 25L SCC 3 46 7 8 9 SCC 5CC 5CC 10 SCC 11 SCC 11 SCC 11 SCC 12 26C 26R 24C 23C 23R 21C 21C 20C 21R 20C 21R 21C 21C 21C 21C 21C 21C 21C 21C 21C 21C	5 11 19 10 13 10 13 10 13 10 13 10 13 13 14 16 10 16 10 16 10 16 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	35 1 45 1 45 1 45 1 45 1 45 1 45 1 45 1 4	60 44 44 529 643 540 542 643 543 543 543 543 543 543 543 543 543 5	554 589 729 381 687 360 798 453 820 621 452 1 081 399 559 312 641 921 761 432 543 599 557 961 471 407 543 1 124 1 161 463 734 1 134 605 346 971 364 429 849 463 1 188 681 655 770 671 831 1 076	729 819 844 767 964 687 809 732 835 875 786 1 097 670 780 613 798 754 1 110 698 737 855 852 950 875 714 769 975 1 110 697 782 1 139 934 636 1 035 658 785 995 580 713 964 820 689 979 783 828 913 1 065 1 256

Appendix J

1972 and 1978 Swift Current Creek
Area Elevation - Capacity Tables

SECTION SURVEY SECTION OTTAMA, ONT.

LOSC72 SC57M1

\* ELEVATION - CAPACITY TABLE \*

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LOSC78 SC57M1 SEDINENT SURVEY SECTION

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530.50	1231.6	551.00	21167.3		• • 1		
539.00	* 4.0551	551.50	22313.6		• •		
539.50	1095.1	552.00	23470-0		•		
540+00	2260.6 *	552.50	24634.7				
540.50	2644.3	553 + 00	25606.5		<b>•</b> • •		
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