



ENVIRONMENT CANADA CONSERVATION AND PROTECTION

OPTIMAL ALLOCATION OF WATER IN THE OKANAGAN RIVER BASIN

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ABSTRACT

objective of this study is to demonstrate the use of linear programming as a planning tool in the allocation of water within a river basin. A linear programming model is developed which maximizes economic benefits from water use in the Okanagan of British Columbia. Many activities including agriculture, recreation, domestic use and sport fishing compete for the sometimes scarce water supplies in this Basin. The model allocates water among these activities subject to water supply, storage and resource constraints. An average water supply year, broken down into seven time periods is represented by the model. The solution represents a sustainable allocation of water over time in that no long term depletion of stored water is allowed. the study was intended as a demonstration project, data-gathering efforts were kept to a minimum. A previous Canada Water Act study in the region provided most of the necessary data to construct the although some estimates of the economic value of water-use activities had to be made. The model solution is consistent with previous studies which showed shortages of water relative to potential demands on most tributaries of the system. However. better data and extensive sensitivity analysis would be required before the model could be used for water management and planning in the region. It was concluded that the approach was feasible in terms of data, manpower and computer requirements and integrated into future studies related to long term water planning and management. ...

RESUME

Le but de cette étude est de démontrer l'utilisation de linéaire comme un outil de planification pour l'allocation d'eau dans un bassin hydrographique. Un modèle de programmation linéaire, qui maximise les avantages économiques de la rivière l'utilisation d'eau du bassin de Okanagan en Colombie-Britannique, est exposé. Plusieurs activités telles que l'agriculture, les loisirs, l'utilisation domestique d'eau et la pêche sportive se font concurrence pour le peu d'eau qu'il y a, parfois, dans ce bassin. Le modèle alloue de l'eau pour ces activités selon l'approvisionnement en eau, l'emmagasinage d'eau et les contraintes d'autres ressources. Une moyenne d'alimentation annuel en eau est répartie entre sept périodes de temps. Le résultat du modèle est une allocation d'eau supportable pour une période de temps mais ne permet pas, à long terme, l'épuisement d'eau retenue. Les données recueillies pour cette étude furent minimes parce que l'intention fut d'élaborer un projet de démonstration. La plupart des données nécessaires pour constuire le modèle furent extrait d'une étude antérieure entreprise sous la Loi sur les ressources en eau du Canada pour la région Okanagan. Il fut aussi nécessaire de faire des estimations de valeur économique concernant la façon que l'eau est utilisé pour les dîtes activités. La solution du modèle est compatible avec des études antérieures qui démontrent la pénurie d'eau relative aux demandes possibles placé sur la majorité des tributaires du bassin. Cependant plus de

données et d'analyses de sensibilité sont nécessaires pour que le modèle puisse être utilisé pour la gestion et la planification des eaux de la région. En somme l'approche démontré par cette étude est réalisable étant donné les exigences de données, de main-d'oeuvre et d'informatique et qu'elle peut être incorporé dans des études futures reliées à la planification et gestion des eaux à longue terme.

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I. INTRODUCTION

The allocation of water among competing demands is a central issue in river basin planning. In a given river basin, the type, location and timing of potential water demands must be considered in the allocation of the available water supply. The type of water demands may encompass a diverse range of activities such as industry, irrigation and maintenance of minimum flows for fish habitat. Locations of these demands in a river system may vary from the upper tributary reaches to the lower valley lakes. The time of year in which these demands occur is critical in the management of water storage in the system. Thus the allocation decision can be thought of as having three dimensions: how to use the water; where to use it; and, when to use it.

Many constraints exist which limit the choices of how, when and where to use the water in a river basin. These constraints may be physical factors such as available runoff and storage or institutional factors such as committed water licences and apportionment agreements. The constraints may also take the form of regulations or policies protecting the basin from environmental degradation. The effect of these constraints is to restrict the choices available when planning present and future allocation of water in a river basin.

When selecting the "best" allocation of water from the many some specific criteria must be used. possible choices. developed in this report uses the criterion of maximizing social benefits when allocating water. Where possible, these benefits are quantified in economic terms using standard methods of benefit-cost analysis. Non-quantifiable factors are also entered in the model through the imposition of constraints which limit the type activities or distribution of water in the system. linear programming model is developed where economic benefits expressed as a mathematical function to be maximized physical and institutional constraints which are also expressed as mathematical functions.

A. Objectives

There are two basic objectives to this report:

- Construct a water use optimization model for the Okanagan River
 Basin to demonstrate the technique of linear programming.
- 2) Discuss possible extensions of the model to improve its applicability in other river basins.

The model developed in this study can be classified as a planning model. The optimal solution sets out long range levels for activities which require water in a river basin. Seasonal patterns of storage, release and withdrawal are also given in the solution. However, the model is not intended as a tool to aid in the short term operations of a river basin system. It will not,

for example, give daily or weekly guidelines for reservoir operations or lake levels. It would be up to the system managers to gear short term operations to meet the needs of the longer term water use activities selected by the planning model.

Since the primary purpose of the study is to demonstrate the applicability and flexibility of the linear programming technique, data gathering efforts were kept to a minimum. Therefore many of the relationships built into the model are based on preliminary data and rough estimates of key variables. While the model may provide some insights into management of the water resources of the Okanagan system, the results should be interpreted with caution.

B. The Problem Setting

The Okanagan River Basin in the southern interior of Columbia was chosen to illustrate the use of the water use optimization model (figure 1). The major reason for its selection was the availability of of data from a previous Canada Water Act Study (Canada, British Columbia, 1974). This data included extensive information on water supplies, water potential water use activities for a number of sub-basins of the watershed. With a relatively small amount αf supplementary data and some assumptions, it was feasible construct a useful demonstration model using linear programming.

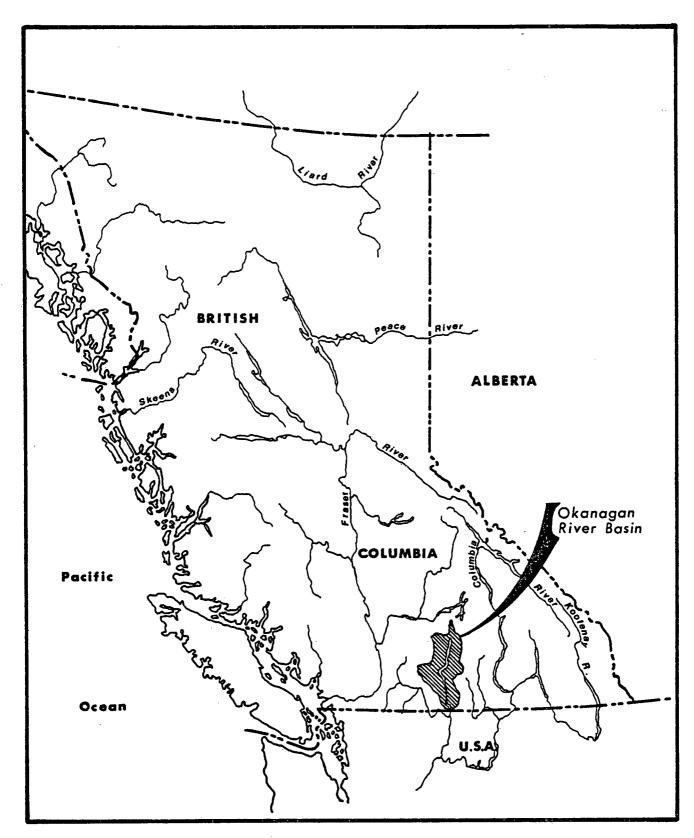


FIGURE 1 OKANAGAN RIVER BASIN - KEY MAP

Several features of the Basin make it an interesting case study for water use optimization. A wide variety of current and potential socio-economic activities require water. These include agriculture, industry, households, recreation and fisheries representing both consumptive and non-consumptive uses. These activities occur in a number of different reaches throughout the geographically diverse watershed of the Basin. Because the watershed is diverse, ranging from alpine snowfields to near-desert valley floors and benchlands, there are significant variations in both supplies and demands for water within the Basin. The possible choice of water use activities is broadened by the availability of large amounts of storage on the main valley lakes and small but important storage reservoirs on the tributaries of the system.

The Okanagan River Basin clearly illustrates the three dimensions (how, when and where) of choice when allocating water within a river basin. Several demands compete for scarce water in areas of the Basin, particularly in the lower reaches of the tributaries. Efficient management of water storage in the system is necessary to ensure that water is supplied at the time and place it is required. The application of the optimization model will demonstrate an approach to allocation of the water resources in the Basin in order to maximize economic benefits.

The next chapter outlines the basic structure of linear programming models and presents the theoretical concepts used in the measurement of economic benefits. The detailed structure of the model as applied to the Okanagan River Basin is presented in chapter 3, and the optimal solution is discussed in chapter 4. The

final chapter presents the conclusions that are drawn from the project and discusses methods for improving the model and extending its application to other river basins.

II. THEORETICAL BACKGROUND

A. Introduction

The purpose of this chapter is twofold. First, the basic theory of linear programming is outlined, with emphasis on how it can be used to model water use allocation in a river basin. A brief review of other studies which have utilized similar methodology in the planning of water resources is undertaken. Second, the basic theory underlying the measurement of individual and aggregate economic benefits is presented.

B. Linear Programming Models

This section examines the basic structure and theory of linear programming models. The suitability of linear programming as a tool for allocation of water in river basins is discussed. Emphasis is placed on its ability to handle large dimensional problems expressed as linear equations.

1. Basic Structure

Linear programming is a subset of a larger class of maximization or minimization problems referred to as mathematical programming models. Mathematical programming attempts to maximize or minimize an objective function subject to a number of constraints, some of which are expressed as inequalities. It

differs from classical optimization problems solved by calculus in which all constraints are in the form of equalities. In linear programming both the objective function and the constraints are expressed as linear equations. Equation (1) is an example of a linear programming model.

(1) Maximize b1x1 + b2x2 + b3x3 subject to:

$$d1x1 + d2x2 + d3x3 < r2$$

e1x1 + e2x2 + e3x3 < r3

Where:

x1.x2,x3 = levels of various activities

c1,c2,c3

d1.d2.d3 = coefficients representing relationships
 between activity levels,

e1.e2.e3

r1,r2,r3 = right hand side values of linear constraints

The objective function represents a goal or target to be maximized such as economic benefits from water in a given river basin. The optimal solution to the above example is the combination of activities that results in the maximum benefits as expressed in the objective function. It would also be possible to formulate a linear programming model where the goal is to minimize

the objective function. An example of such a model might be to minimize cost associated with supplying water to the various users in a river basin.

In the above example, the three constraints state that certain linear combinations of activities be less than the right hand side values. r1-r3. It is also possible to construct a linear programming model where some or all of the constraints state that combinations of activity levels be greater than (or equal to) the right hand side variables. The only restriction is that at least one of the constraints be expressed as an inequality.

The linear constraints represent the availability of resources and the linkages between the activity variables (x1,x2,x3) in the model. For example, a simple constraint might state that the surface run-off in a particular reach of the Basin is equal to 1200 acre-feet. Another constraint might state that the amount of water withdrawn from a stream is less than the total amount of water flowing into the reservoir. Linear constraints can generally be used to represent all of the important hydrologic and economic relationships in the area under study.

An additional constraint on the above model and on all linear programming models is that the activity variables must be non-negative. This does not represent a problem when applying linear programming to water use optimization since most physical activities related to the use and distribution of water are also non-negative. It is possible to conceive of variables which can take negative values such as change in reservoir levels. However, the use of such activities in the model can be avoided by defining

other constraints which use only positive activities to express the same relationships.

An important consideration when solving systems of equations is the relationship between the number of equations and the number of activities. However, in linear programming there are no restrictions on the relationship between the number of constraints and the number of activities. There may be more constraints than there are activities or there may be more activities than constraints. This adds considerable flexibility when applying linear programming to water use optimization since constraints can be added or deleted as needed to reflect the characteristics of the basin under study.

Solution Method

The method of solving linear programming problems and the computational effort required are important factors when applying this technique to water use optimization. Depending on the basin under study and the degree of disaggregation required, the number of activities and or constraints can be in the hundreds or even thousands. Therefore, it is important to have an efficient algorithm for finding the optimal feasible solution.

Fortunately, there is a reliable and relatively fast method of solving linear programming maximization or minimization problems which is referred to as the "simplex" method. This method rests on the fact that an optimum solution to a linear programming problem will also be a "corner" solution. Figure 2 illustrates the concept of a corner solution for a problem with two activities and three

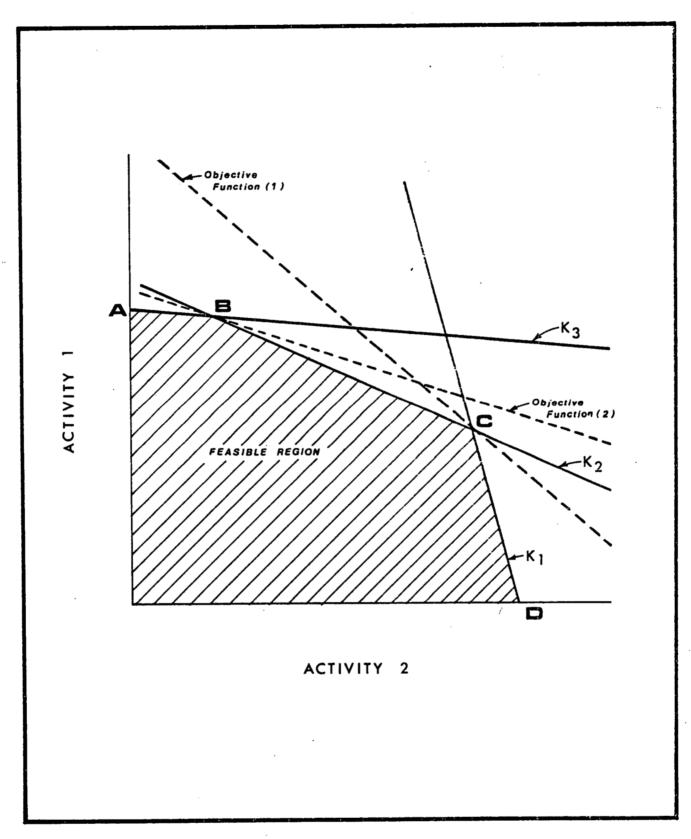


FIGURE 2 CORNER SOLUTIONS IN LINEAR PROGRAMMING

constraints. The three constraints are represented by the three solid lines labelled k1,k2 and k3. For a solution to be feasible it must lie inside all of these constraints in the area referred to as the feasible region. The four points A,B,C and D, are called corner solutions since they lie on the corners of the feasible region.

The objective function, represented by the dotted line. becomes greater as the line is moved out from the origin. maximum feasible solution is the point within the feasible region at which the objective function is at its maximum. In figure 2 the maximum solution is at point B. If the slope of the objective function were to change, the maximum solution would also change but would still lie on a corner point, as shown by the second dotted line which intersects with point C. It is possible for the objective function to be parallel to one of the constraints in which case there could be a range of maximum solutions bordering the feasible region. However, at least one of the corner points will be included in this range. Note that for larger dimensional problems the feasible region and corner points will be defined by the intersection of planes or hyperplanes rather than lines.

The computational problem of finding the maximum feasible solution is simplified in that only corner points have to be examined. However, for a larger problem there may be a many corner points. The advantage of the simplex method is that it can find the maximum feasible solution without having to examine all of the corner points. Without going into any detail, the simplex method starts with an initial corner point, then moves along the

border of the feasible region to another corner solution. The best direction to move along the border can be determined by examining the relative change in the objective function. Using this method, the maximum solution can be found by examining only a small fraction of the feasible points. This is an important consideration when solving larger models which are used in water use optimization.

There are two possible problems which can prevent an optimum solution being found. First, there may be no feasible solution that satisfies all of the constraints. If this were to occur in the case of water use optimization in a river basin, it would be safe to assume that one or more of the constraints had not been specified correctly. The problem should not occur if the situation is modeled correctly. Second, the solution may be "unbounded" the objective function can be means that indefinitly without violating any of the constraints. Again, if this were to occur in the case of water use modeling, it would be because of an error in the manner in which the constraints or the objective function were specified. Thus neither of the problems should be encountered in water use optimization if the situation is correctly specified and modeled.

3. Dual Activities

For any given linear programming problem there exists another problem known as the "dual". For instance, if we consider the problem stated earlier in equation (1) which is to maximize benefits subject to constraints on resource levels, the dual

problem can be stated as follows: minimize the resource levels subject to the constraint that benefits are at a stated maximum. Equation (2) shows the dual problem to equation (1).

(2) Minimize r1y1 + r2y2 + r3y3 subject to:

$$c1y1 + d1y2 + e1y3 > b1$$

$$c2y1 + d2y2 + e2y3 > b2$$

where:

y1,y2,y3 = dual activities

The dual problem is formulated by transposing the original (primal) problem and introducing the variables, y1 - y3 which are referred to as dual activities. Note that the coefficients on the primal activities, x1 - x3, occur in the dual problem but in a different order. The benefit coefficients, b1 - b3, of the primal problem become the right-hand side values, and the primal right-hand side values, r1 - r3, become the coefficients on the objective function of the dual problem.

The dual problem is automatically solved at the same time as the primal solution. This is a useful feature of linear programming because of the relationship between the dual activities and the original (primal) problem. The most important relationship for the purposes of this study is that the levels of the dual activities in the optimal solution represent "shadow prices" of

the resource levels r1,r2 and r3 in the primal problem.

The shadow price of a resource represents the amount by which the objective function would increase if the level of the resource were increased by one unit. In economic terms it can be thought of as the marginal value product of the resource. It is a useful concept when analyzing the allocation of water in a river basin. For example, if we consider storage capacity as a resource, then the shadow price will tell us how much an additional unit of storage is worth in terms of increased benefits. Shadow prices on other constraining resources such as monthly run-off in different reaches of the Basin can be interpreted in the same manner.

C. Applications of Mathematical Programming in Water Management

Mathematical programming techniques have been used in a broad range of applications in water planning and management. It is convenient to classify these studies into two categories: operational models and planning models. A brief discussion of these two classes of models is given below.

Operational models are used in the short term management of water supply systems and are concerned with such problems as determining the daily or weekly release from storage reservoirs in order to meet demand requirements. These models are sometimes "real-time" implying that they are continuously solved and updated to determine what actions are required by system managers at any given time. Other operational models are short term models which are solved to give daily or weekly guidelines for management of the

water supply system. In general, operational models are oriented towards supply management with fixed water demands. The objective function in these models often attempts to minimize the deviations from target water supply levels to the various users. Numerous studies which have developed operational models have been reported in the literature. Because these studies have a different emphasis than the planning model discussed in this report, no attempt was made to review this body of literature.

Planning models are concerned with a broader range of objectives over a longer term. These objectives include such factors as the optimum allocation of water among various users, the amount of storage required in the water supply system and the effects of management policies such as pricing and other conservation measures. Planning models present a framework for addressing the questions of where, when and how the water resources in a basin should be used. In contrast to operational models, the concepts of value and demand for water are explicitly considered in framework. A planning model may form this part of hierarchical structure in which it is used to determine a longer term allocation of water which can be incorporated into the structure of a shorter-term operational model.

Previous studies which have developed planning models for water resources are numerous although less common than studies on operational models. These studies have been undertaken for a wide variety of problems over a broad range of geographical areas. A brief review of the published literature follows.

Several planning models have directly addressed the problem of water supply capacity expansion. Most of these studies attempted to determine the optimum time path for expansion of storage and delivery systems based on the criterion of maximizing economic benefits. Recent examples of such studies are Moore and Yeh (1980) for the Eel River system in California and Lin (1981) for the Umatilla River basin in Oregon. Armstrong and Willis (1979) carried out a similar study over a number of regions in California but also considered optimal allocation of water among competing users at the same time.

A number of studies have attempted to determine the optimal allocation of water among competing users. Many of these studies concentrated on a single predominant user of water in a basin and attempted to optimize the amount and timing of water supplied to this use. For example Timmons (1982) carried out a detailed analysis of irrigation profitability for various crops in Utah and Abate (1975) developed a general model for crop selection and water allocation for dryland farming. Guise and Flinn (1970) used an optimization model to analyze agricultural demand for water and to develop optimal pricing policies for areas of Australia.

Other studies have developed models to allocate water over multiple uses. For example, Nieh (1979) studied allocation of water between agricultural and urban areas as did Armstrong and Willis (1979) for selected areas of California. The variety and scope of multi-use models is noteworthy. While many studies such as Hopkins et.al. (1982) and Kindler (1982) have concentrated on the river basin as the management unit, others such as Kieth

et.al. (1982) and Arizona (1971) have carried out state-wide analysis of water allocation. There is also a contrast between the structure and focus of the models. For example the Arizona (1971) study was based on an aggregate input-output model of the state economy in contrast to a study by Bartlett (1974) which developed a detailed model of the ecology of rangeland in order to optimize allocation of water.

The specification of the objective function in previous studies is of particular interest. The basic question is what are models attempting to optimize? As mentioned, these operational models simply attempt to minimize the deviations of water supply from pre-set target levels. Other studies such as and Rea (1972) and Arizona (1971) have utilized concepts such as gross state product or value added as the criteria for maximization. The disadvantage of these concepts is that they do not account for non-market factors such as recreational activity, leisure time and environmental quality. Most of the other studies optimized economic benefits from water use. In these quoted have studies, economic benefits are defined in the traditional cost-benefit framework. A few studies such as Kindler (1982) and Flatt and Howard (1976) have included optimization of some variables for which economic benefits cannot be quantified dollar values. These models allow the managers to assign their own weights in the objective function to non-monetary variables.

Maximization of economic benefits is the criterion used in the present study to determine optimum water allocation in the Okanagan River Basin. Economic benefits are defined as in traditional cost-benefit analysis using consumer's surplus as the basis for calculating benefits. The next section describes some of the basic theory of consumer's surplus and related concepts.

D. The Measurement of Economic Benefits

When choosing between different allocations of water in a basin, it is necessary to have some method of ranking the economic benefits or utility which individuals derive from use of the water. This measure of benefits can then be incorporated into the objective function for maximization. Some theoretically sound methods exist for measuring utility changes for individuals. However, when attempts are made to sum up individual changes in utility to arrive at the aggregate benefits for a group of people, several problems are encountered. These problems implications both for the model developed in this study and for conventional cost-benefit analysis.

Measuring Changes in an Individual's Utility

Given a change in prices of goods and/or income, there are three general methods which can be used to estimate the resulting change in the utility of an individual. These are the equivalent variation, the compensating variation and the consumer's surplus. 1 Although weaker on theoretical grounds, consumer's surplus is utilized in applied cost-benefit analysis because it is the easiest to observe empirically.

The consumer's surplus for a particular commodity is the area under the individual's demand curve for the commodity but above the price paid for it. For a change in the price of the good, the difference in consumer's surplus can be used as an indicator of the change in the individual's utility. However, if the price of more than one good has changed, then there is not a unique value for the change in consumer's surplus associated with the change in utility. There will be several possible values for the difference in consumer's surplus depending on the order in which the prices are changed. However, it has been shown by Willig (1976) that for many situations, the consumer's surplus will not differ appreciably from the equivalent variation or compensating variation which are exact measures of individual's utility.

In the region under study there is little metered pricing of water. Water is allocated mainly through quantity rationing by water licences or on a first come first serve basis. Therefore we are interested in the effect on an individual's utility when the amount of water allocated to him is changed. This situation is illustrated in figure 3. The amount of water rationed to the

^{1.} This report uses the Marshallian definition of consumer's surplus and the Hicksian definition of compensating and equivalent variation. These terms are sometimes defined in a different fashion.

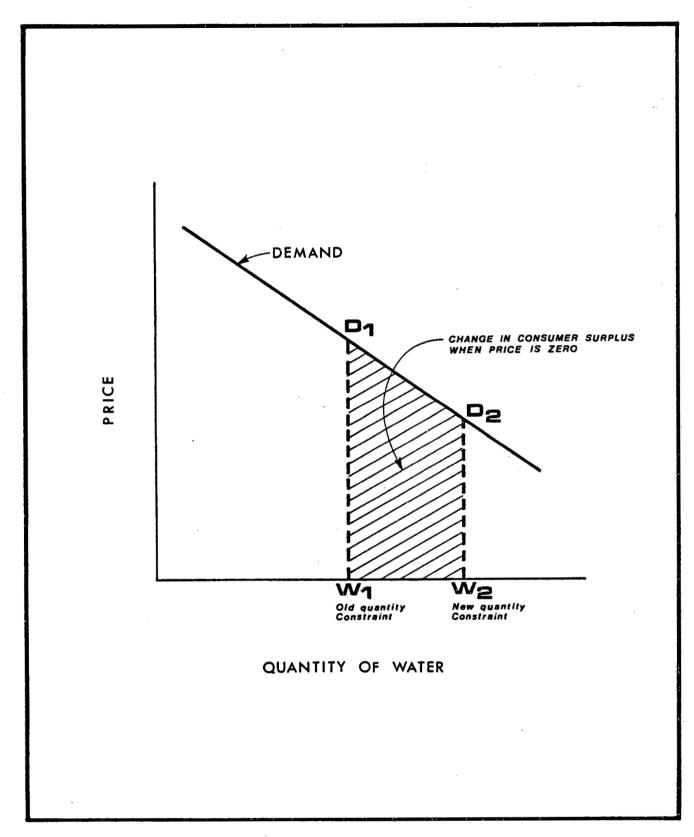


FIGURE 3 CONSUMER'S SURPLUS WITH QUANTITY CONSTRAINTS

individual is increased from W1 to W2. Assuming a zero price for water, the change in area under the demand curve represents the change in consumer surplus and can be used as an indication of the change in the individual's utility. This area is represented by D1,D2,W1,W2 in figure 3.

Equivalent variation and compensating variation are measures of the change in utility for an individual whose original allocation of goods (state A) has changed to a new allocation (state B). The equivalent variation is the amount of income that would have to be given to or taken away from the consumer in state A (keeping the original prices) which would result in him just achieving his new utility in state B. The compensating variation is the amount of income that would have to be given to or taken from the individual in state B (keeping the new prices) in away order for him to reach his original utility in state A. Both the compensating and equivalent variation have a one to correspondence with changes in utility, and are therefore theoretically sound measures of the individual's utility change.

In order to calculate the change in consumer's surplus illustrated in figure 3, it is necessary to have an estimate of the demand curve. This can be a problem when there are no observable prices for water in the region. In applied cost-benefit analysis, the consumer's surplus is often estimated by indirect methods, without direct estimation of the demand curve.

2. Measuring Changes in Aggregate Utility

Since most changes in allocation or prices of goods will affect many individuals, it would be desirable to have a measure of change in aggregate utility (economic benefits). The approach in assessing the economic benefits of a project is simply to add up the changes in each individual's utility resulting from the project. The aggregate utility can then be used to compare group welfare for different allocations of water. The major problem encountered with this approach is that it does not consider the relative utilities between individuals. For instance. should a change in utility for a wealthy individual be given the same weight as a change in utility for a poor individual? of the difficulty in comparing the utility of individuals, applied welfare analysis has usually relied on the Pareto principle.

The Pareto principle states that state B is preferred to state A, if at least one person is better off in state B, and no one is worse off than in state A. A "Pareto improvement" is said to result when the economy moves from state A to state B. If it is not possible to make any further Pareto improvements by moving from state A to a different state, then State A is said to be "Pareto optimal". The Pareto concept is useful because we do not need to know anything about the relative utilities between individuals. All that is needed is a ordinal ranking for each individual relating his utility to the allocation of goods he receives. This ranking can be based on any of the three utility indicators discussed previously.

A number of problems with the Pareto principle are recognized. Most public projects will affect a number of people, some advantageously and some adversely. Such projects can not be directly assessed using the Pareto principle. A second problem is that the Pareto principle gives no way of comparing between Pareto optimal states. A third problem with the Pareto principle is that some Pareto optimal states cannot be shown to be superior to all non-pareto optimal states.

These problems can be overcome to some extent by assessing projects as to their potential for Pareto improvements using the compensation. If states A and B are principle of Pareto non-comparable it might still be possible to rank them if all the individuals who gain from going to state B were to compensate the individuals who were made worse off from the transition to state If this compensation results in everyone being at least as well off in state B as in state A, with at least one person better off, Pareto improvement will have taken place. It would then a therefore seem possible to assess a public project by its potential Pareto improvement if the gainers were to compensate the for This is in fact the object of almost all cost-benefit losers. analysis; to determine whether a project can result in a potential Pareto improvement. Such analysis is not usually predicated on the assumption that compensation will actually take place. Ιt iş usually assumed that compensation will somewhat magically occur somewhere else in the system.

Unfortunately, even if we assume that compensation will occur somewhere else in the system, the compensation principle has a number of problems. A major theoretical problem is that two different states can be shown to be superior to each other using the compensation principle. This is often referred to as the problem of reversals. A second problem is that of intransitivity. Using the compensation principle, state C could be ranked as superior to state B and state B as superior to state A, while at the same time state A could be ranked superior to state C. The only way to avoid the problem of reversals and intransitivities is to make some highly restrictive and unrealistic assumptions about individual preferences.²

Even if we ignore the problems of reversals and intransitivities, the best that can be said is that an increase in aggregate compensating or equivalent variation is necessary for a potential Pareto improvement to occur, but not sufficient. In terms of benefit-cost analysis this implies that a benefit-cost ratio greater than unity is necessary but not sufficient to conclude that a project is worth undertaking. Boadway and Bruce (1984, p.271) make the following cautionary statement about measuring benefits using the compensation principle:

The use of the unweighted sum of household compensating or equivalent variations as a necessary and sufficient indicator of potential Pareto improvement is rife with difficulties. At best such measures can be used as a preliminary attempt to rank social states.

^{2.} The restrictions are that all households have linear and parallel income expansion lines or Engle's curves.

3. The Approach Used in this Study

The model developed in this study is based on maximization of total consumer's surplus from water use in a river basin. The assumption is made that consumer's surplus is a good approximation to the aggregate compensating variation, and therefore the model maximizes the aggregate compensating variation from water use in the Basin.

It is recognized that this approach only yields an "optimal" allocation of water in a very limited sense. The previous discussion points out the difficulties in assuming that maximizing consumer's surplus will lead to maximization of social welfare, even if potential gainers were to compensate potential losers. Therefore it is important to recognize that the model is simply a tool which can be used as an input into the allocation decision. A variety of other factors such as distribution of the benefits, public safety and ecosystem health are also of major importance in the allocation and use of the water resource.

E. Summary

The linear programming approach has several advantages for modeling water use in a river basin. It is capable of handling large multidimensional problems encountered when modeling allocation of water. The computational methods are efficient and the simultaneous solution of dual activities gives shadow prices of the constraining resources. The economic theory behind the optimization of water use is somewhat weaker. It cannot be said

that maximization of consumer's surplus will result in the maximum social welfare from water use. Therefore the model should only be used as an aid to planning within a broader decision framework.

In the next chapter we utilize the linear programming approach and the economic concepts discussed here to develop a model of water use allocation in the Okanagan River Basin. The detailed specifications of the constraints and the objective function are presented.

III. MODEL STRUCTURE

The model is constructed in modular form, with a distinct sub-model for each reach in the Basin. These sub-models are linked together to form the basin-wide model using constraints to represent the inflow-outflow relationships between the reaches. An objective function is formulated which represents the total benefits from water-use activities over the whole basin. Because the structure of the sub-models are basically similar, the detailed specification is given for only two sub-models; the first representing a tributary reach and the second representing a mainstem reach.

A. Basin Configuration

The Basin was divided into a number of reaches in the model representing both tributaries and sub-basins of the mainstem river and lakes. The breakdown is consistent with a number of previous supply and demand studies carried out in the Okanagan Basin Study allowing for a detailed analysis for most areas of the Basin where competition for scarce water has occurred in the past. The following eleven reaches are modeled:

- 1. Penticton Creek
- 2. Mission Creek
- 3. Kelowna Creek
- 4. Equesis Creek
- 5. Powers Creek
- 6. Peachland Creek
- 7. Trout Creek
- 8. Okanagan Lake
- 9. Skaha Lake
- 10. Okanagan River
- 11. Osoyoos Lake

The seven creeks shown are the major tributaries to Okanagan Lake based on volume of flow. These seven tributaries account for almost all of the water-use in the Basin which is not supplied from the mainstem lakes and river. However, the model does not include the highly developed Kalamalka-Vernon Creek system because of the lack of data on natural run-off and monthly water use. Several other smaller tributaries to the system were also omitted because of data limitations. The net inflow from these tributaries into the mainstem system was included as an exogenous variable in the model.

The remaining four reaches comprise the mainstem system of the Basin. Of these, Okanagan Lake is the dominant reach in terms of storage and urban development. However, both Skaha and Osoyoos

Lakes support high density recreational uses and Okanagan River supplies water to significant areas of agricultural land in the south. Figure 4 shows the location of the eleven reaches incorporated in the model.

It can be seen from figure 4 that most of the run-off occurs in the higher elevations of the Basin. Some of the seasonal run-off from the higher elevations is captured and stored in reservoirs on the major tributaries for use later on in the year. This storage, although minor in comparison to the natural storage of Okanagan Lake, is important because it is a source of water for major areas of agriculture on the lower reaches of the tributaries. In fact most agricultural land around Okanagan Lake relies on water from the tributaries where it can be obtained from gravity fed systems. This is generally a much more economical system than pumping water from the lake itself.

The tributaries also support a resident trout population and serve as spawning ground for trout and kokanee that reside in the mainstem lakes. The resident trout population is generally located in the upper reaches and reservoirs of the creeks while the spawning areas are usually found in the lower levels. Historically, the fish population has declined as a result of the water withdrawn for other uses on the tributaries. Figure 5 illustrates the location of water use and storage activities on the tributary and mainstem reaches of the model.

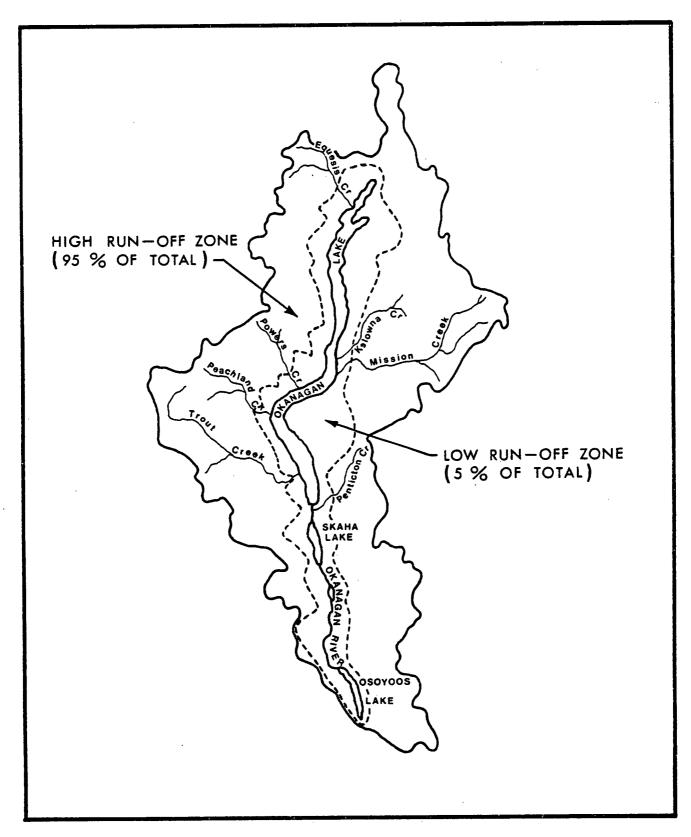


FIGURE 4 OKANAGAN RIVER BASIN - LOCATION OF REACHES IN MODEL

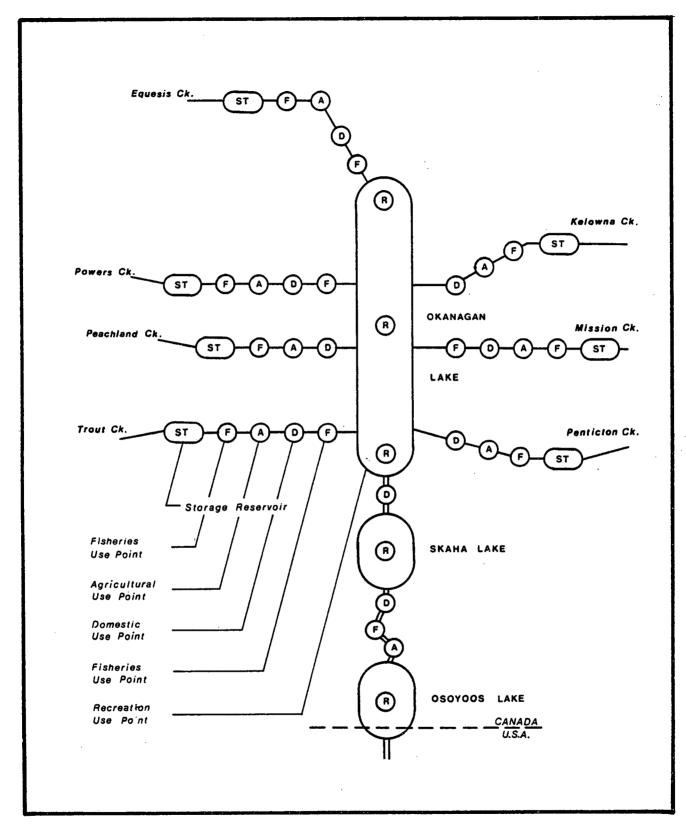


FIGURE 5 OKANAGAN RIVER BASIN - SCHEMATIC DIAGRAM OF WATER USE AND STORAGE

B. Seasonal Breakdown

The model allocates water over a single year which is divided into seven periods. The individual months from May to October form the first six periods while the remaining months are grouped together to form the seventh period. The disaggregation into sub-year periods allows analysis of the seasonal patterns of water-use, storage and run-off. Agricultural and domestic demands are at their maximum during the summer months. At the same time lake levels should be maintained to meet the requirements for water based recreation. Instream flows for fisheries are required the month of October. earlier in the year and later on in Meanwhile most of the natural run-off occurs in the freshet during May and early June. Thus the seasonal aspect of storing water and allocating it to its various uses is an important consideration in the model.

A weakness of the model is that it considers the allocation of water for only a single year. This is a problem for the mainstem system where inter-year trade-offs in water use are possible because of the large storage capacity of Okanagan Lake. In any given year, there is a choice of using the stored water or retaining it for use in the following year. This is an important decision because of the possibility of a drought occurring in the subsequent year. However, under normal snow-pack conditions, run-off in subsequent years would be adequate to replenish the water stored on the mainstem lakes. The possibility of inter-year storage is also a consideration on some tributaries where the

storage capacity and water use are large compared to the normal run-off.

Without considering the demand for water in subsequent years, there will be a tendency for the model to "mine" the storage of water in the lakes and reservoirs. In other words, the optimal likely use as much water solution will from both storage and run-off as needed to maximize benefits in the current year without consideration for benefits in future years. To protect against this likelihood, a constraint is placed on the model where the end of year storage must be at least equal to the beginning year storage. The optimal solution will then represent an allocation of water that is sustainable over the long term, given average run-off conditions. Although this will prevent a solution which mines the storage, it is not necessarily the optimal policy in all run-off conditions. In years with very low run-off it may in fact be desirable to deplete some of the storage with the hope that it will be replenished in future years. In years of high run-off it might be a good policy to store as much as the water as possible use in future years when less run-off would likely occur. possibilities can be included in the model using dynamic programming methods. This was beyond the scope of the present study but could be carried out as a logical extension of the current work. Chapter five discusses how dynamic programming could be used to analyze inter-year trade-offs in water use.

C. Specifying the Objective Function

The model defines three "final use" activities which have positive values in the objective function. These are (1) irrigated acreage under production, (2) harvestable population of sport fish, and (3) lake levels falling within desirable ranges for recreation. There are other final use activities which obviously have value associated with them including domestic, commercial and industrial uses of water. The model constrains the solution to supply as much water as is required by these activities before any water is allocated to the other three activities specified in the objective function. Thus an implicit assumption is made that domestic, commercial and industrial use of water takes precedence or has a higher value than other users in the model. This assumption, while probably valid, was made for the reason that consumer's surplus values or rents associated with domestic, commercial and industrial uses of water were not readily estimated. Estimating these values would again be a logical extension of the current study.

For each acre of irrigated land, a net benefit of \$800, is assumed.³ This is entered in the objective function simply as:

(3) 800A1 + 800A2 + 800A3 + 800A4 + 800A5 + 800A6 +800A7 + 800A8 + 800A9 + 800A10 + 800A11

where A1 to A11 are variables representing the number of irrigated acres in the eleven reaches of the Basin.

^{3.} This figure is based on a case study of an Okanagan orchard carried out by the British Columbia Ministry of Agriculture (1983).

The figure of \$800 per acre may not be representative of the average net return in all areas of the Basin, but was used as a first approximation. Because of lower tree fruit prices in recent years, the net return may in fact be much lower. This points out the need for sensitivity analysis using a range of values in the objective function before the model could to be used for planning purposes.

Populations of spawning kokanee, spawning trout and resident trout enter as activities in the objective function. A positive benefit is assigned to each fish produced in the tributaries. This value was derived from net willingness to pay figures obtained in a survey of sport fishermen in the Okanagan Basin Study⁴. Equation (4) represents the fish population activities in the objective function for a specific tributary.

(4) 10Rt1 + 10Rt2 + 10Rt3 +10Sk +10St

where:

Rt1,Rt2,Rt3 are resident trout populations

St is the spawning trout population

Sk is the spawning kokanee population

^{4.} A value of \$10 per fish was based on catch rates and willingness to pay for an angler day. The basic information was taken from the fisheries and wildlife technical supplement to the Okanagan Basin Study (Canada-British Columbia 1974d, p.123).

The resident trout are divided into three separate activities in the objective function because of a non-linear relationship between tributary flows and population. A series of linear functions are used to approximate this non-linear relationship, with each linear function having a corresponding value in the objective function. This process is explained in more detail in section 4 which specifies the relationship between water supply and fish populations. Note that equation (4) represents objective function values of fish for one tributary only. In the complete model the objective function values in equation (4) are entered for all seven tributaries. No attempt was made to model fish populations and values on the mainstem system because of a shortage of data.

Water based recreation enters into the objective function in the form of positive values associated with desirable lake levels for recreation. Certain minimum lake levels are required for convenient use of launches, docks and beaches on the mainstem system. If lake elevations fall below these levels, some of these facilities will be unusable resulting in extra travel time and aesthetic losses to recreationists. Critical lake levels are defined for Okanagan, Skaha and Osoyoos lakes below which recreational losses will occur. Positive values in the objective function are associated with an increase in lake levels up to the critical elevations. Equation (5) shows the lake level activities in the objective function.

(5) 0.089o2REC + 0.356o3REC + 0.356o4REC + 0.089o5REC +

0.509s2REC + 2.036s3REC + 2.036s4REC + 0.509s5REC +

1.000y2REC + 4.000y3REC + 4.000y4REC + 1.000y5REC

where:

o2REC - o5REC = Level of Okanagan Lake expressed in acre-feet of storage. (Below a critical level of 336.800 acre-feet)

y2REC - y5REC = Level of Osoyoos Lake expressed in acre-feet of storage. (Below a critical level of 16,980 acre-feet)

The label numbers 2,3,4 and 5 refer to periods in the model corresponding to the months of June, July, August and September. No value is assigned to lake levels in other months of the year when water-based recreation is relatively insignificant. The objective function values are highest in periods 3 and 4 when recreational use of the lake is at a maximum. The objective function values associated with lake levels are derived from estimates made in the Okanagan Basin Study⁵ and from recent field surveys relating recreational losses to Osoyoos Lake levels.

It should be noted that the objective function only assigns values to lake levels up to critical elevations. The model does not prevent the lake levels from going higher than the critical lake elevations: it simply stops assigning any increases in recreational value once these levels are reached.

^{5.} Rough estimates of the annual recreational losses on Okanagan Lake due to extreme fluctuations in lake-levels were taken from the Okanagan Basin Study (Canada-British Columbia 1974a, p.30).

D. Specifying the Constraints

There are over 800 constraints in the model so it is impractical to list them all in this report. The large number of constraints is due to the disaggregated structure of the model which includes 11 reaches and seven periods and numerous activities. However, the structure of the model can be illustrated by defining the constraints for only specific reaches and then demonstrating the linkages between reaches.

The model as a whole can be considered as a linked series of sub-models with each sub-model representing one of the eleven reaches. The seven tributary sub-models all have more or less the same structure and therefore only the constraints for a single illustrative tributary are presented. Trout Creek is used as the example tributary because it includes all of the fishery and agricultural activities specified in the objective function. The four mainstem sub-models all have the same basic structure so only the sub-model for the Okanagan Lake reach is presented. This sub-model demonstrates how both recreational and return-flow activities are incorporated as constraints.

1. Trout Creek Sub-model

The constraints for this sub model fall into four categories:

- 1) water supply constraints,
- 2) downstream demand constraints
- 3) constraints linking supply and demand
- 4) linking constraints with the downstream reach.

a. Water Supply Constraints

Water can be supplied from three sources: run-off above the storage reservoir; run-off below the reservoir; and, water released from storage. In the model it is assumed that there is only one storage reservoir on each tributary reach. For tributaries which have more than one reservoir, storage capacity is summed and modeled as if it were all contained in a single large reservoir. In general, release from storage provides most of the water requirements for the downstream reaches. Run-off below the reservoir is a less significant source of water supply for the downstream activities. Almost all of the water using activities in the model are located below the storage reservoirs so the high levels of run-off above the reservoir serve more to replenish storage than as direct supplies for water use activities.

The model first defines the run-off in each period above the reservoir as shown in equations (6) to (12). These figures represent the average run-off for the Trout Creek basin based on historical records.

- (6) a1ROA = 25,104 acre-feet⁶
- (7) a2ROA = 15,283
- (8) a3ROA = 1,831
- (9) a4ROA = 888
- (10) a5ROA = 755
- (11) a6ROA = 755
- (12) a7ROA = 5,065

where:

ROA = run-off above the reservoir.

In all equations the variables are prefixed by a two character code signifying the sub-basin and the season.

The run-off below the reservoir is presented in a similar fashion in equations (13) to (19).

^{6.} Acre-feet are used in this and all subsequent equations in order to be consistent with the units used in the Okanagan Basin Study from which most of the relationships in the report are derived.

- (13) a1ROB = 799
- (14) a2ROB = 388
- (15) a3ROB = 127
- (16) a4ROB = 36
- (17) a5ROB = 36
- (18) a6ROB = 36
- (19) a7ROB = 346

ROB = run-off occurring below the storage reservoir

Because not all the run-off can be stored or utilized, the model incorporates a wastage factor. Wastage occurs on certain days of high run-off when not all the water can be utilized or when storage is not available. When the daily run-off is aggregated to give monthly figures, an attempt should be made to account for the wastage that has occurred on such days. Other losses may occur because of leakage and evaporation of the diversion and conveyance systems. In this study it is assumed that 10% of the monthly run-off is wastage. Figuations (20) to (33) define the relationship between the run-off and the actual water available for use over the seven periods of the model.

^{7.} This is a general estimate of wastage based on figures used in Canada-British Columbia (1974c). Actual wastage will vary between tributaries, but sufficient data were not available to model this.

- (20) a1ROAW .9a1ROA = 0
- (21) a2ROAW .9a2ROA = 0
- (22) a3ROAW .9a3ROA = 0
- (23) a4ROAW .9a4ROA = 0
- (24) a5ROAW .9a5ROA = 0
- (25) a6ROAW .9a6ROA = 0
- (26) a7ROAW .9a7ROA = 0
- (27) alrobw .9alrob = 0
- $(28) \ a2ROBW .9a2ROB = 0$
- $(29) \ a3ROBW .9a3ROB = 0$
- (30) a4ROBW .9a4ROB = 0
- (31) a5ROBW .9a5ROB = 0
- (32) a6ROBW .9a6ROB = 0
- (33) a7ROBW .9a7ROB = 0

- ROAW = the amount of run-off occurring above the reservoir available for use.
- ROBW = the amount of run-off occurring below the reservoir available for use

The next set of constraints are the storage balance equations which define the relationship between storage levels, inflows to the reservoir and outflows from the reservoir. The inflow is represented by the run-off above the reservoir after wastage has

been deducted and the outflow is represented by the release from storage. Evaporation losses were not included because of a lack of data. Equations (34) to (40) define the storage balance relationship for each period.

- (34) aOSTO + a1ROAW a1REL a1STO = 0
- (35) a1STO + a2ROAW a2REL a2STO = 0
- (36) a2STO + a3ROAW a3REL a3STO = 0
- (37) a3STO + a4ROAW a4REL a4STO = 0
- (38) a4STO + a5ROAW a5REL a5STO = 0
- (39) a5STO + a6ROAW a6REL a6STO = 0
- (40) a6STO + a7ROAW a7REL a7STO = 0

where:

STO = the amount of water stored in the reservoir

REL = the amount of water released from storage

The variable aOSTO represents the storage level at the beginning of the year before any water has been added or withdrawn. This level is defined as a fixed value in the model as shown in equation (41). An additional constraint shown in equation (42) is that the end-of-year storage, a7STO, be equal to the beginning-of-year storage.

- (41) aOSTO = 5000
- (42) a7STO = 5000

b. Water Demand Constraints

For each acre of irrigated land, a fixed amount of water is required. Based on estimates made in the Okanagan Basin Implementation Agreement⁸, a value of three acre-feet per acre is used for the Trout Creek area. Irrigation occurs only in the first five periods with the heaviest applications in the second, third and fourth periods as shown in equations (43) to (47).

- (43) aOACRE 2.22a1ACR = 0
- (44) aOACRE 1.33a2ACR = 0
- (45) aOACRE 1.33a3ACR = 0
- (46) aOACRE 1.33a4ACR = 0
- (47) aOACRE 3.33a5ACR = 0

where:

aOACRE = the number of irrigated acres

a1ACR - a5ACR = the amount of water required in each period.

As previously mentioned, the model is forced to supply all domestic and industrial water needs before any remaining water can be allocated to other uses. This is done by setting up simple constraints defining domestic/industrial use in each time period as shown in equations (48) to (54).

^{8.} The water requirements are given in the main report (Canada-British Columbia 1974a, p.84). Higher requirements occur in the southern areas of the Basin and are reflected in the coefficients for the Okanagan River and Osoyoos Lake sub-models.

- (48) allows = 29
- (49) a2DOMR = 32
- (50) a3DOMR = 29
- (51) a4DOMR = 29
- (52) a5DOMR = 32
- (53) a6DOMR = 29
- (54) a7DOMR = 212

DOMR = a fixed amount of water supplied to domestic and industrial uses in each period.

Modeling the water use for production of sport fish is somewhat more complex than modeling consumptive uses by agriculture or domestic. The populations of spawning trout and kokanee are the amount of flow in certain months in the lower related sections of the reach. A rough flow-population relationship for the production of spawning fish was derived in the Okanagan Basin Study9 and has been used as the basis for all of the seven in the model. Rainbow trout require significant tributaries streamflows during the freshet when they ascend the tributaries to spawn. The resulting fry begin their downstream migration in June although about half may stay in the tributaries for at least a year before entering Okanagan Lake, thus requiring year-round flows. Equations (55) to (61) relate the population of spawning trout to levels of flow in the lower reaches.

^{9.} This relationship related improvements in spawning escapement with modifications in discharge (Canada-British Columbia 1974d, p.45-52).

- (55) aOSTR .31a1FISR < 0
- (56) aOSTR .47a2FISR < 0
- (57) aOSTR .47a3FISR < 0
- (58) aOSTR .47a4FISR < 0
- (59) aOSTR .47a5FISR < 0
- (60) aOSTR .47a6FISR < 0
- (61) aOSTR 1.2a7FISR < 0

aOSTR = the population of spawning trout

a1FISR - a7FISR = flows available for sport fish in the lower reaches

The constraints relating kokanee population to flows are set out in the same manner as for spawning trout. In general there are much higher populations of kokanee than trout using the lower reaches of the tributaries for spawning and less units of flow are required for each kokanee produced. Because kokanee spawn in the autumn and the fry descend to the lake in the freshet, they do not require flows in the critical summer months and thus are less competitive with consumptive uses. Equations (62) to (68) represent the relationship between flows and spawning kokanee population.

- (62) aOKOK 7.5a1FISR < 0
- (63) aOKOK 9.0a5FISR < 0
- (64) aOKOK 5.0a6FISR < 0
- (65) aOKOK 1.3a7FISR < 0

aOKOK = the population of spawning kokanee

Modeling the relationship between the resident trout in the upstream reaches of the tributary is more complex than for the spawning kokanee and trout on the lower reaches. This is because a non-linear relationship exists between fish populations and stream flows. 10 This non-linear relationship can be approximated with three linear segments as shown in figure 6, each modeled with a separate set of constraints. Equations (66) to (72) represent the population-flow relationship for the segment A-B in figure 6.

- (66) aORTR1 15.9a1FL1 = 0
- (67) aORTR1 15.9a2FL1 = 0
- (68) aORTR1 15.9a3FL1 = 0
- (69) aORTR1 15.9a4FL1 = 0
- (70) aORTR1 15.9a5FL1 = 0
- (71) aORTR1 31.9a6FL1 = 0
- (72) aORTR1 191 a7FL1 = 0
- (73) aORTR1 < 3986

^{10.} This relationship was derived in the Okanagan Basin Study (Canada-British Columbia 1974d, p.32).

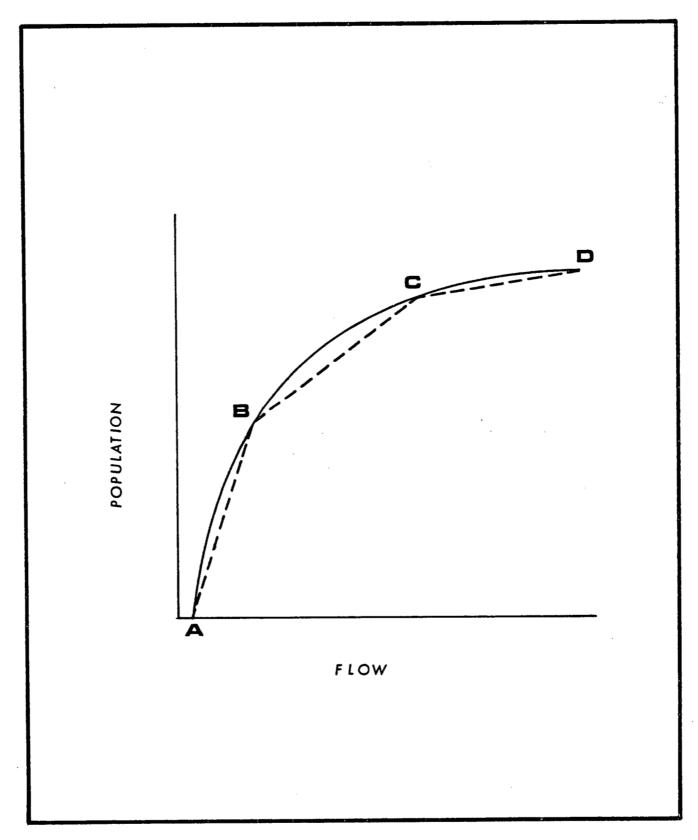


FIGURE 6 RELATIONSHIP BETWEEN TROUT POPULATION AND STREAMFLOW

aORTR1 = Resident trout population on segment A-B a1FL1 - a7FL1 = flows in the upper reaches in periods periods one to seven.

The second linear segment, B-C, is modeled by equations (74) to (81).

- (74) aORTR2 3.2aOFL2 = 0
- (75) aORTR2 3.2a2FL2 = 0
- (76) aORTR2 3.2a3FL3 = 0
- (77) aORTR2 3.2a4FL2 = 0
- (78) aORTR2 3.2a5FL2 = 0
- (79) aORTR2 4.4a6FL2 = 0
- (80) aORTR2 38.4a7FL2 = 0
- (81) aORTR2 < 1594

where:

aORTR2 = resident trout population on segment B-C a7FL2 - a7FL2 = flows in the upper reaches in periods one to seven

The population-flow relationship for the final segment C-D is shown in equations (82) to (89).

```
(82) aORTR3 - 1.1a1FL3 = 0
```

(83) aORTR3 - 1.1a2FL3 = 0

(84) aORTR3 - 1.1a3FL3 = 0

(85) aORTR3 - 1.1a4FL3 = 0

(86) aORTR3 - 1.1a5FL3 = 0

(87) aORTR3 - 2.2a6FL3 = 0

(88) aORTR3 - 13.3a7FL3 = 0

(89) aORTR3 < 552

where:

RTR3 = resident trout population on segment C-D a1FL3 - a7FL3 = flows in the upper reaches in periods one to seven

c. Constraints Linking Supply and Demand

These constraints specify that water demands in the system must be less than or equal to water supplies. These constraints apply on the upper reaches just below the storage reservoir before any water is withdrawn for consumptive uses and on the lower reaches below the withdrawal points for consumptive uses. For the upper reaches the constraints state that upstream uses (flows for resident trout) must be less than or equal to the upstream water supply (release from storage).

```
(90) a1FL1 + a1FL2 + a1FL3 - a1REL < 0
```

- (91) a2FL1 + a2FL2 + a2FL3 a2REL < 0
- (92) a3FL1 + a3FL2 + a3FL3 a3REL < 0
- (93) a4FL1 + a4FL2 + a4FL3 a4REL < 0
- (94) a5FL1 + a5FL2 + a5FL3 a5RE1 < 0
- (95) a6FL1 + a6FL2 + a6FL3 a6REL < 0
- (96) a7FL1 + a7FL2 + a7FL3 a7REL < 0

The linkage constraints for the lower reaches state that the total demand for agriculture, domestic and spawning kokanee and trout be less than or equal to the total water supply in each time period. The total supply is equal to release from storage plus natural run-off below the reservoir. These constraints are shown in equations (97) to (101).

- (97) alack + albomk + alfisk alkel alkobw < o
- (98) a2ACR + a2DOMR + a2FISR a2REL a2ROBW < 0
- (99) a3ACR + a3DOMR + a3FISR a3REL a3ROBW < 0
- (100) a4ACR + a4DOMR + a4FISR a3REL a4ROBW < 0
- (101) a5ACR + a5DOMR + a5FISR a5REL a5ROBW < 0
- (102) a6ACR + a6DOMR + a6FISR a6RCBW < 0
- (101) a7ACR + a7DOMR + a7FISR a7REL a7ROBW < 0

d.Linkages to Downstream Reaches

There are two sets of constraints which link the tributary sub-model to the next downstream reach. The first set of constraints defines the remaining flow in each time period which will pass into the downstream reach (Okanagan Lake in this case). The remaining flow is defined as any excess in water supply over consumptive uses. Note that flows for fisheries can be reused in the downstream reach.

(102) -alrem + alreL + alrob - alack - albomr = 0

(103) -a2REM + a2REL + a2ROB - a2ACR - a2DOMR = 0

(104) -a3REM + a3REL + a3ROB - a3ACR - a3DOMR = 0

(105) -a4REM + a4REL + a4ROB - a4ACR - a4DOMR = 0

(106) -a5REM + a5REL + a5ROB - a5ACR - a5DOMR = 0

(107) -a6REM + a6REL + a6ROB - a6ACR - a6DOMR = 0

(108) -a7REM + a7REL + a7ROB - a7ACR - a7DOMR = 0

where:

alREM to a7REM = the remaining flow to the downstream reach

The second set of linking constraints defines the return flows from consumptive uses in the sub-model. It is assumed that these return flows pass directly to the downstream reach (Okanagan Lake) and are

not re-usable in the tributary itself. During the Okanagan Basin $Study^{11}$ it was estimated that 90 percent of domestic water and 50 percent of irrigation water was returned to the system. Equations (109) to (115) define the return flows for each period based on these coefficients.

$$(109) - a1REM + .05a0ACRE + .9a1DOMR = 0$$

(110)
$$-a2REM + .07aOACRE + .9a2DOMR = 0$$

$$(111)$$
 -a3REM + .07a0ACRE + .9a3DOMR = 0

$$(112) - a4REM + .07a0ACRE + .9a4DOMR = 0$$

$$(113) - a5REM + .06a0ACRE + .9a5DOMR = 0$$

$$(114) - a6REM + .05aOACRE + .9a6DOMR = 0$$

$$(115) - a7REM + .13a0ACRE + .9a7DOMR = 0$$

where:

a1RET to a7RET = return flows in periods one to seven

This completes the specification for the Trout Creek sub-model. The other sub-models for the tributaries have the same specification of constraints although the coefficients on the variables will be specific to the tributaries. Right hand side values defining limits to variables such as run-off, storage, domestic use and agricultural land will also be specific to each sub-basin.

^{11.} These estimates were taken from the technical supplement on water quantity (Canada-British Columbia 1974b, p.491) of the Okanagan Basin Study.

2. Okanagan Lake Sub-model

This sub-model has many of the same features of the tributary sub-models and only the differences will be highlighted in this section. The general structure of the Okanagan Lake sub-model is the same as the Trout Creek sub-model except that all run-off is considered to take place above the lake and all consumptive uses are supplied directly from lake storage rather than from downstream release. Other differences involve the inclusion of return flows and remaining flows from the tributaries as input into the available supply from Okanagan Lake. The mainstem sub-models also do not model sport fish production explicitly because data were not available to construct these relationships. However, minimum flow requirements for spawning sockeye salmon are imposed on the Okanagan River sub-model.

a. Supply Constraints

For purposes of clarity, all remaining flows and return flows from the tributaries are summed to give a single variable for each period. The constraint defining this variable for period 1 is shown in equation (116).

a to g = prefixes signifying the seven tributaries

Similar equations are constructed for the remaining periods in the model.

The storage-balance equations are different from those on the tributaries since water for consumptive uses is drawn directly from the lake. Equation (117) defines the storage-balance relationship for the first period.

where:

olRET = the return flow from agriculture and domestic within the sub-basin

o1EVA = evaporation from Okanagan Lake

Unlike the tributaries, the return flows are available on the same reach since Okanagan Lake is large enough to catch and store the

return flows. An evaporation variable is also introduced because of the large surface area of the lake. It should also be noted that the run-off variable (ROA) represents the net inflow from unmodeled tributaries.

Equation (117) also implicitly contains the link between supply and demand in the Okanagan Lake reach, so further constraints defining the relationship between available water and water use are not necessary for this reach.

b. Demand Constraints

The constraints setting out agricultural and domestic demand are similar to those for the tributary sub-models and so are not presented here. Some additional constraints are necessary in order to define the limits to recreational benefits. As specified in the objective function, recreational benefits occur when lake levels are increased up to certain elevation. Equations (118) to (125) define the relationship between lake levels for recreation and lake storage levels.

- (118) o2REC o2STO < 0
- (119) o3REC o3STO < 0
- (120) o4REC o4STO < 0
- (121) o5REC o5STO < 0
- (122) o2REC < 336800
- (123) o3REC < 336800
- (124) o4REC < 336800
- (125) o5REC < 336800

o2REC - o5REC = desirable lake levels for recreation

While the Okanagan Lake sub-model does not include sport fish production, some consideration of minimum flows for the spawning sockeye salmon are included. These minimum flows are specified by setting minimums for the release from Okanagan Lake storage as shown in equations (126) to (132).

(126) o1REL > 18000

(127) o2REL > 18000

(128) o3REL > 18000

(129) o4REL > 50000

(130) o5REL > 58000

(131) o6REL > 21000

(132) o7REL > 63000

where:

o1REL - o7REL = release from Okanagan Lake

These minimum flows were recommended in the Report on the Okanagan Basin Implementation (Canada-British Columbia, 1984). In general, the flows should be adequate for the returning sockeye run. although increased flows would be desirable. However, there were no data to construct a population-flow relationship as was done for the tributary sub-models. Okanagan Lake also supports considerable

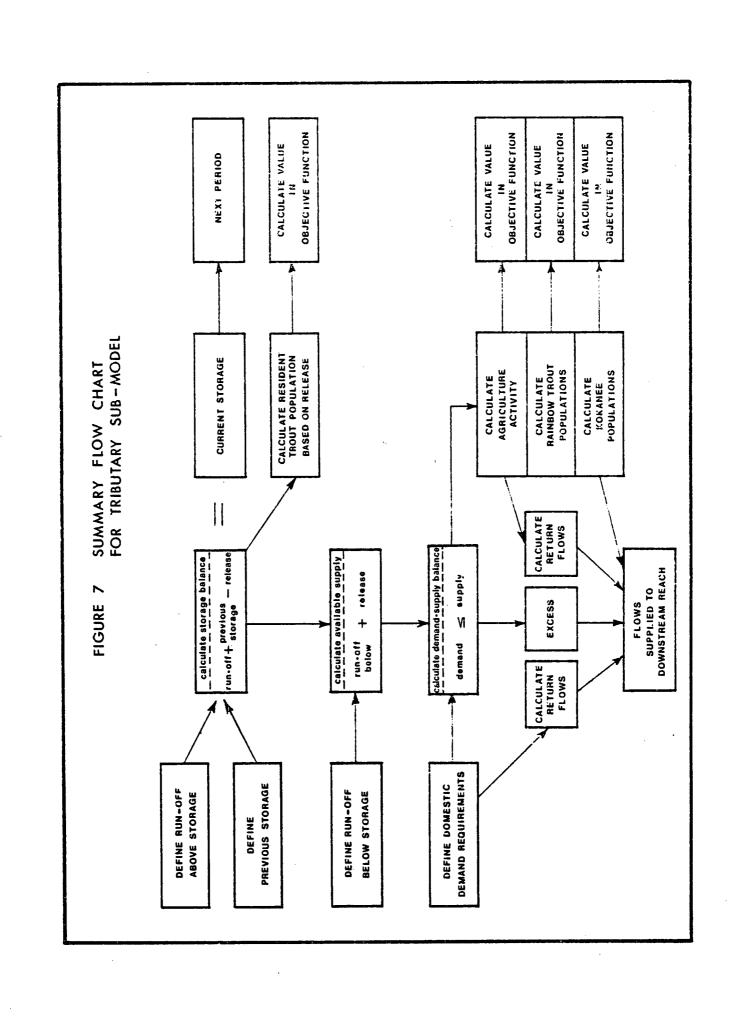
shore-spawning kokanee for which certain lake levels are desirable in different seasons of the year. Again there was not sufficient data with which to relate populations to lake levels and this aspect of fish production was not modeled.

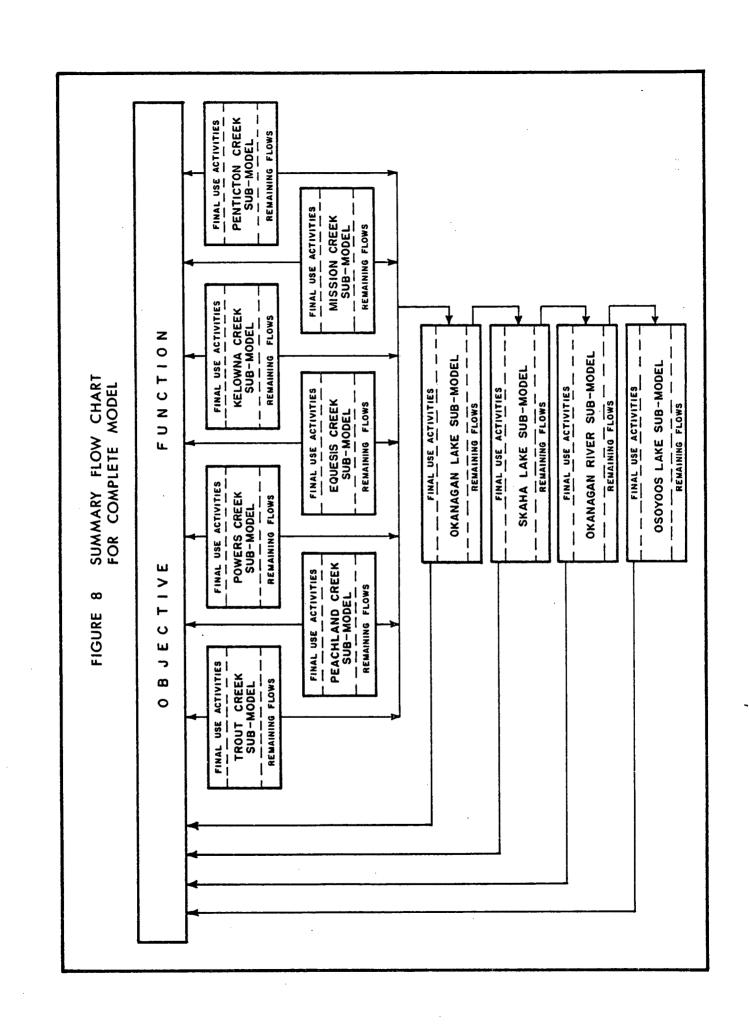
E. Summary of Model Structure

The model is composed of 11 sub-models each representing one reach of the Basin. Seven of these sub-models represent tributaries flowing into Okanagan Lake. The structure of each of the seven sub-models in terms of the number and kinds of constraints and activities is the same with the only differences being in the coefficients of the equations. Figure 7 illustrates the general structure for a tributary sub-basin model.

The seven tributary sub-models are linked to the mainstem by their outflow which includes return flows and remaining flows after diversions for consumptive uses. This outflow becomes inflow into the Okanagan Lake sub-model of the mainstem. Outflow from Okanagan Lake then becomes an inflow into the Skaha Lake sub-model. Okanagan River and Osoyoos Lake form the final downstream sub-models of the Basin. The general structure of the complete model is summarized in figure 8.

Given the specification of the objective function and constraints, the next step is to solve the model in order to find the maximum feasible value of the objective function subject to the constraints specified. The next chapter presents the model solution giving optimal levels of the water use activities and water management variables.





IV. MODEL SOLUTION

A. Existence of an Optimal Solution

A feasible solution to the model was obtained which maximized the value of the objective function and determined the optimal allocation of water among various uses. However, it was concluded that the solution obtained did not represent a unique maximum of the objective function. This conclusion was reached when examination of the solution showed that a surplus of water existed on the mainstem reaches of the model which could either be stored (up to a certain maximum) or released (up to a limit on maximum flows). Neither the storage limit nor the maximum flow limit were binding constraints and it followed that there were several different periods when the surplus water could be released without affecting the maximum level of the objective function. The disposal of surplus water would be critical if flood control considerations were incorporated in the model. In this were the case the disposal of the surplus would be managed so as to minimize the danger of subsequent flooding. Given this additional factor, it would be more probable that a unique optimal solution would exist.

The non-uniqueness of the optimal solution is not considered a problem with the model since the solution represents the maximum possible benefits that can be derived from use of the water. However, the disposal of surplus water should be included in future applications of the model to areas where flooding is of concern.

B. Computer Costs

The model was solved using the MPSX software package on a mainframe computer. An optimal solution was obtained in about nine seconds of central processing unit time using 46 page-minutes of virtual memory. Computer costs for a single solution were about \$10.00 based on low-priority commercial rates. Based on these costs it would be feasible to carry out extensive sensitivity analysis and or modifications of the model in future applications.

The total computer costs of developing the model were, however, fairly significant. The basic data entry, MPSX coding and file editing required significant connect time. Each of the 11 sub-models was tested separately and modified when necessary. Many runs of the complete model were required before it could be certain that the model was specified correctly. A complete accounting of the developmental computer costs was not kept but an estimate of total cost is approximately \$2,000.

C. Optimal Activity Levels

The results for the optimal solution are presented for each of the 11 reaches in the model. Patterns of storage and release were determined for each tributary and mainstem reach as was the allocation of the available water among various final use activities. Where applicable, the shadow prices of water supply are also presented. A complete computer printout of the optimal solution for all activities is also given in Appendix One.

1. Penticton Creek Sub-basin

The model indicates that there is a relative scarcity of water in this sub-basin and that there is competition for its use among a number of activities. In the average run-off year represented by the model, there is not enough water to supply all the potential irrigable land in the sub-basin and to ensure that domestic needs are met. In the optimal solution, no water at all is supplied for the spawning trout or kokanee. Table 1 shows the levels of the final use activities chosen by the model.

TABLE 1

Penticton Creek Sub-Basin
Optimal Level of Final Use Activities

Activity	Optimum	Upper Bound	Shadow Price (\$)
Irrigated Acres	1,665	2,000	0.0
Resident Trout	1,249	1,776	0.0
Spawning Trout	0	120	0.0
Kokanee Trout	0	4,000	0.0

Note: The shadow price is the amount by which the objective function would increase if the upper bound were increased by one unit.

It can be seen from Table 1 that all of the final use activities are below their maximum possible values (upper bound). This indicates that an increase in the supply of water would be of value in the sub-basin and should thus have a positive shadow price. Table 2 indicates the shadow prices for an increase in natural run-off and storage capacity in the Penticton Creek system.

TABLE 2

Penticton Creek Sub-basin

Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)
ROA3	453.0
ROA4	453.0
ROA5	453.0
ROA6	453.0
ROA7	453.0
ROB3	453.0
ROB4	453.0
ROB5	453.0
Storage Capacity	453.0
· · · · · · · · · · · · · · · · · · ·	

Run-off above and below the reservoir as well as storage capacity all have a positive shadow price of \$453. The shadow price represents the amount by which the objective function would increase if any of these variables were to increase by one unit

(one acre-foot). The shadow prices are quite high because of the potential for increasing irrigated acreage in this sub-basin and for increasing the resident trout population. Because there is a surplus of water which cannot be stored in the first two periods, the shadow price for an additional unit of run off in these periods is zero. Storage capacity has a positive shadow price because increasing it would enable some of the surplus water in the first two periods to be stored for use in the following water-short periods.

There are three competing water-use activities in the lower reaches of this sub-basin which are domestic/industrial, agricultural and spawning kokanee and trout. The model is constrained to supply a fixed amount to domestic/industrial users while allocating the remaining amount between agriculture and flows for fisheries. Figure 9 shows the allocation of water among these competing downstream uses. Agriculture and domestic activities are both significant users of water, although agricultural use is confined to the first five periods. It should again be noted that the final period includes the six months from November to April, so the domestic/industrial use for this period appears higher than for the single months representing the first six periods.

The model also determines the pattern of storage and release during the year necessary to maximize the objective function. This pattern is shown in Figure 10. Starting from an initial storage level of 5,000 acre-feet, the model increases the storage level over the next two periods until maximum capacity of about 10,000

REST FIGURE 9: PENTICTON CREEK SUB-BASIN OCT DOMESTIC-INDUSTRIAL SEPT ALLOCATION OF WATER AUG JULY JUNE AGRICULTURE MAY 9.8 9.0 4.0 2.4 2.2 <u>.</u> ဖ 7. 0.2 ~ 0 ACRE-FEET (Thousands)

REST FIGURE 10: PENTICTON CREEK SUB-BASIN OCT SEPT Release AUG STORAGE AND RELEASE + JULY Storage JUNE МАҰ BEGIN 10 0 œ 9 8 თ S M (Thousands)

acre-feet is reached. The stored water is steadily depleted over the remaining periods in order to meet the various demands. The pattern of release reflects the higher requirements for domestic and agricultural use in the summer months. A minor amount of water is released in all periods to supply the resident trout population.

2. Trout Creek Sub-basin

There is enough water in this sub-basin to supply irrigation to all of the potential agricultural land while at the same time supplying some water for downstream fisheries. However, there is still an overall shortage in the sense that water is a limiting factor in the production of sport fish. Table 3 indicates the optimal level of final use activities for this sub-basin.

TABLE 3

Trout Creek Sub-basin
Optimal Level of Final Use Activities

Activity	Optimum	Upper Bound	Shadow Price (\$)
Irrigated Acres	6,150	6,150	771.0
Resident Trout	4,263	6,132	0.0
Spawning Trout	٥	1,000	0.0
Kokanee Trout	1,451	2,000	0.0

Note: The shadow price is the amount by which the objective function would increase if the upper bound were increased by one unit.

Irrigated acres are at their upper bound and there is thus a positive shadow price for an additional unit of agricultural land. However, all of the sport fish populations are below their maximum values, indicating that an increase in the supply of water would have a positive benefit. The shadow prices for additional water supply and storage are shown in Table 4.

TABLE 4

Trout Creek Sub-basin

Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)
ROA3	14.0
ROA4	14.0
ROA5	14.0
ROA6	14.0
ROA7	14.0
ROB3	14.0
ROB4	14.0
ROB5	14.0
ROB6	14.0
ROB7	5.8
Storage Capacity	15.6

The shadow prices in the Trout Creek sub-basin are much lower than for the Penticton Creek sub-basin because all available agricultural land has been used in the model. The shadow prices shown in Table 4 are based only on the extra production of sport fish that would be possible if extra water were available. Storage

capacity has a positive shadow price since an increase in storage would enable extra water from the freshet to be stored for use both by resident and spawning fish.

Most of the downstream water is allocated towards irrigation as shown in Figure 11. Domestic/industrial use is relatively insignificant while flows for fisheries are also relatively small. Domestic/industrial use is fairily constant throughout the year. while flows for the fishery are highest in the months of May and October.

The optimal storage and release patterns are shown in Figure 12. The storage pattern over the seven periods is similar to that of Penticton Creek. Storage is brought to a maximum during the high run-off periods and gradually depleted to meet requirements during the summer months. In the final periods storage levels are brought back to their beginning levels. However, there are significant differences between the patterns of release for the two sub-basins because there is a higher ratio of run-off to storage in the Trout Creek sub-basin. Releases are high on the Trout Creek system in May and June because the high volume of run-off cannot be fully contained in the limited storage capacity. Releases are low in the periods when irrigation is not required.

FISH REST FIGURE 11: TROUT CREEK SUB-BASIN OCT SEPT DOMESTIC - INDUSTRIAL ALLOCATION OF WATER AUG JULY JUNE MAY **AGRICULTURE** 4.5 3.5 2.5 1.5 0.5 S 0 M 2 YCKE FEET (Thousands)

REST OCT FIGURE 12: TROUT CREEK SUB-BASIN SEPT Release AUG STORAGE AND RELEASE + JULY Storage JUNE MAY BEGIN ACRE FEET (Thousands)

3. Peachland Creek Sub-basin

In this sub-basin the supply of water is high relative to the current uses. The model supplies all final use activities up to their maximum possible levels as shown in Table 5. Because of the limited agricultural and domestic/industrial use of water, the model takes advantage of the abundant water to supply flows for maximum fishery production.

TABLE 5

Peachland Creek Sub-Basin
Optimal Level of Final Use Activities

Activity	Optimum	Upper Bound	Shadow Price
Irrigated Acres	459	459	800.0
Resident Trout	939	939	10.0
Spawning Trout	2,760	2,760	10.0
Kokanee Trout	21,260	21,260	10.0

Note: The shadow price is the amount by which the objective function would increase if the upper bound were increased by one unit.

Because all potential agricultural, domestic/industrial and fishery activities can be fully supplied with water, there is no value to an additional unit of water in this sub-basin. In the optimal solution, shadow prices in all periods for both run-off and

storage are zero (table 6). Two conclusions can be drawn from the zero shadow prices. First, as previously stated, there is no value to an additional unit of water in the Peachland Creek sub-basin. Second, there is no value from an additional unit of water in the reaches downstream of this sub-basin since excess water from Peachland Creek also has the potential for use in downstream reaches.

TABLE 6

Peachland Creek Sub-basin

Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)
ROA3	0.00
ROA4	0.00
ROA5	0.00
ROA6	0.00
ROA7	0.00
	1
ROB3	0.00
ROB4	0.00
ROB5	0.00
ROB6	0.00
Storage Capacity	0.00

The allocation of water among downstream users in this subbasin is somewhat different from the other tributaries. Because there is a relatively abundant supply of water and agricultural lands are limited, the model allocates water to the sport fishery until maximum population levels are obtained. This is in contrast to most other tributaries where sport fish production is limited by the available water supply. Figure 13 illustrates the downstream allocation of water for Peachland Creek. It can be seen that the allocation of water to fishery flows is unusually high in the month of June. This is due to the high flow requirements for both kokanee and rainbow trout spawning in this period. Aside from this month, the allocation of water among the various uses is fairly even.

The optimal patterns for storage and release are shown in Figure 14. The pattern of release over the year is somewhat atypical, as the highest release occurs in the month of May due to the earlier than usual freshet in the sub-basin and because of fishery requirements. During the remaining periods, the amount of water released is fairly constant. The storage pattern is more consistent with the other tributaries in that storage is brought to a maximum during the freshet month of June and gradually depleted during the summer and fall with refill occurring in the winter months.

FISH REST FIGURE 13: PEACHLAND CREEK SUB-BASIN OCT SEPT DOMESTIC-INDUSTRIAL ALLOCATION OF WATER AUG JULY JONE **AGRICULTURE** MA≺ 2.4 9.0 0.4 2.6 2.2 ₩. 1.6 4. 0.8 0.2 0 1.2 (Linousands)

ACRE FEET

REST FIGURE 14: PEACHLAND CREEK SUB-BASIN OCT SEPT Release AUG STORAGE AND RELEASE + JULY Storage JUNE MAY BEGIN ဖ S M 8 ACRE FEET (Thousands)

4. Powers Creek Sub-basin

Powers Creek also has a fairly abundant supply of water relative to potential demands. Agriculture is the predominant user of water, although irrigated lands are not extensive. This subbasin was identified in the Okanagan Basin Study as having a fairly high potential for sport fish production and the model does allocate a significant amount of water to flows for the fishery. Table 7 shows the downstream final use activities.

TABLE 7

Powers Creek Sub-basin
Optimal Level of Final Use Activities

Activity	Optimum	Maximum	Shadow Price (\$)
Irrigated Acres	1,167	1,167	796.0
Resident Trout	877	897	0.0
Spawning Trout	2,560	2,560	9.6
Kokanee Trout	34.040	34,040	0.0

Note: The shadow price is the amount by which the objective function would increase if the upper bound were increased by one unit.

Both irrigated acres and spawning trout are at their upper bound and their shadow prices are thus positive. Kokanee and resident trout are near but not quite at their maximum possible limits, indicating that additional water would have a positive value. Table 8 indicates the shadow prices on additional water supply.

TABLE 8

Powers Creek Sub-basin

Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)
ROA3	1.71
ROA4	1.71
ROA5	1.71
ROA6	1.71
ROA7	1.71
ROB3	1.71
ROB4	1.71
ROB5	1.71
ROB6	1.71
Storage Capacity	1.71

The shadow prices for additional water are positive but low in most periods. The low values occur because all agricultural land has been supplied and the only remaining remaining productive use of the water is in supplying flows for fisheries. Storage also has a positive shadow price since excess water preserved in the high run-off months could be used for sport fish production in later months.

Agriculture is again the main user of downstream water in the model. although a fairly significant and constant amount of water is allocated to flows for fish production in each period (Figure 15). DomestTc and other use is low throughout the year.

The storage pattern for Powers Creek is roughly similar to that of Peachland Creek with reservoir levels being built up in May and June and depletion of stored water taking place in the remaining months. The release pattern for Powers Creek is unusual in that there is no peak in the month of May occurs in most other sub-basins. There is no peak in release because the storage capacity on this system is high compared to the spring run-off, so the peak inflows to the reservoir can be captured and stored for later use. As in most other sub-basins, the amount of water released from storage increases in the summer months in order to meet consumptive requirements (Figure 16).

FISH REST FIGURE 15: POWERS CREEK SUB-BASIN OCT SEPT DOMESTIC-INDUSTRIAL ALLOCATION OF WATER AUG JULY JUNE **AGRICULTURE** MAY ان 4. J. 1.2 6.0 0.8 9.0 0.5 0.7 4.0 0.3 0.2 0.1 0 --(spubsnoul) ACRE FEET

REST FIGURE 16: POWERS CREEK SUB-BASIN OCT SEPT Release AUG STORAGE AND RELEASE + JULY Storage JUNE MAY BEGIN 7 S T Υ Ο M 9 ACRE FEET (Thousands)

5. Equesis Creek Sub-basin

Equesis Creek has a fairly abundant water supply relative to consumptive uses. There is only a small amount of potentially irrigable land in this sub-basin and there is virtually no domestic or industrial use of water. As a result, the model allocates water to these uses until they are at or near their maximum as shown in table 9. Considerable flows are also allocated towards the production of kokanee and trout.

TABLE 9

Equesis Creek Sub-basin
Optimal Level of Final Use Activities

Activity	Optimum	Maximum	Shadow Price (\$)
Irrigated Acres	356	356	800.0
Resident Trout	728	736	0.0
Spawning Trout	2,460	2,460	10.0
Kokanee Trout	27,600	27,600	10.0

Note: The shadow price is the amount by which the objective function would increase if the upper bound were increased by one unit.

The downstream uses are all close to their maximum possible levels. As a result the shadow prices of additional water are quite low as shown in table 10.

TABLE 10

Equesis Creek Sub-basin
Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)
ROA3 ROA4 ROA5	0.67 0.67 0.67 0.67
ROA6 ROA7 Storage Capacity	0.67
2001491 04940107	

Downstream water is allocated about equally between irrigation and flows for fisheries as shown in Figure 17. This is an unusual occurrence for the tributaries, as agriculture is usually a much more predominant user of water. Equesis Creek is an exception because most land is undeveloped or unsuitable for irrigation while the potential for sport fish production is quite high.

The storage pattern for the Equesis Creek system is similar to that of Peachland and Powers Creek, with accumulation occurring in the peak run-off months of May and June and depletion occurring throughout the remaining periods. The peak release occurs in June when storage capacity is not sufficient to contain the high run-off. For the remaining periods, release from storage is fairly constant (Figure 18).

REST FIGURE 17: EQUESIS CREEK SUB-BASIN OCT FISH SEPT ALLOCATION OF WATER AUG AGRICULTURE JULY JUNE MAY2.8 3.2 2.6 2.2 . 8. 1.6 0.8 9.0 Ŋ 1.2 0.4 0.2 (Thousands) ACRE FEET

REST FIGURE 18: EQUESIS CREEK SUB-BASIN OCT SEPT Release AUG STORAGE AND RELEASE + JULY Storage JUNE MAY BEGIN 0 0.5 2.2 <u>π</u> 1.2 0.8 9.0 4.0 2 (Thousands)

6. Kelowna Creek Sub-basin

From the model solution it is evident that there is a shortage of water in Kelowna Creek relative to potential demands. Kelowna Creek is the only tributary modeled where there is a major shortfall of water relative to potential irrigation demands. However, this shortage may not accurately reflect the actual situation as some water demands in this sub-basin are supplied by Okanagan Lake water or by diversion from other sub-basins. There is only a limited potential for sport fish production in this system and no water is supplied to this activity in the optimal solution (table 11).

TABLE 11

Kelowna Creek Sub-basin
Optimal Level of Final Use Activities

Activity	Optimum	Maximum	Shadow Price (\$)
Irrigated Acres	3,009	4,202	0
Resident Trout	0	208	0
Spawning Trout	0	125	0
Kokanee Trout	0	4,000	0

Note: The shadow price is the amount by which the objective function would increase if the upper bound were increased by one unit.

It is felt that the model solution does not accurately represent the actual water supply and demand balance in this sub-

basin since some transfer of water into this system occurs from other sources. The amount and location of these transfers were not modeled because of a lack of data. Based on information from the Okanagan Basin Study, it appears that the amount of water transferred into the sub-basin is sufficient to supply all agricultural use in the area.

Shadow prices on water supply are quite high because of the apparent shortage of water in the system. Increased storage capacity also has a high shadow price since the excess run-off occurring in the freshet could also be used to supply agricultural needs if storage were available. Table 12 shows the shadow prices for the water supply variables.

TABLE 12

Kelowna Creek Sub-basin

Shadow Prices on Water Supply

Supply Variable	Shadow Price
	(\$)
ROA3	457
ROA4	457
ROA5	457
ROA6	457
ROA7	457
ROB3	457
ROB4	457
ROB5	457
Storage Capacity	457

Agriculture is the predominant user of water in the optimal solution as shown in Figure 19. Domestic requirements are relatively minor although they occur throughout the year. No water was allocated to flows for sport fish production.

The seasonal pattern of storage and release on the Kelowna Creek system is shown in Figure 20. The storage pattern for Kelowna Creek is similar to other sub-basins in that water is accumulated during the freshet and depleted in the subsequent months. In most sub-basins the depletion phase of storage takes place throughout the year until the minimum required year-end level is reached in the last period. However, in the case of Kelowna Creek, the minimum storage level reached in the autumn months is below the year-end minimum required. Reliance is placed on winter precipitation to bring the storage levels back up to the required level at year-end. The Trout Creek system is the only other tributary which has a similar pattern. The release pattern for Kelowna Creek peaks in the month of June and remains high during the summer months in order to supply agricultural needs. Very little water is released during the fall and winter months.

REST FIGURE 19: KELOWNA CREEK SUB-BASIN OCT DOMESTIC-INDUSTRIAL SEPT ALLOCATION OF WATER AUG JULY JUNE ZZ AGRICULTURE МАҮ 9.0 0.5 o. œί 9. 3 5.3 7. 0.0 0.8 0.7 . ACRE FEET (Thousands)

REST FIGURE 20: KELOWNA CREEK SUB-BASIN OCT SEPT Release AUG STORAGE AND RELEASE JULY Storage JUNE MAYBEGIN 5 7 0 M ဖ ACRE FEET (Thousdada)

7. Mission Creek Sub-basin

Mission Creek is exceptional in that it has both a high level of agricultural activity and a very large potential for sport fish production, particularly kokanee. While there is enough water to meet all agricultural requirements, the high potential production of sport fish is not realized due to a shortage of water to maintain desirable flows. Table 13 shows the level of final use activities on the system.

TABLE 13.

Mission Creek Sub-basin
Optimal Level of Final Use Activities

Activity	Optimum	Maximum	Shadow Price (\$)
Irrigated Acres	8,717	8,717	398.0
Resident Trout	10,805	13,782	0.0
Spawning Trout	2,675	9,565	0.0
Kokanee Trout	1,475,000	1,475,000	7.7

Note: The shadow price is the amount by which the objective function would increase if the upper bound were increased by one unit.

The shadow prices for the water supply variables are shown in table 14. Despite the fact that the upper bound of agricultural activity has been reached, the shadow prices for an additional unit of water are quite high. This is because a large number of sport fish can be produced for a small increment in flows. In other words, an additional unit of water on Mission Creek is much more valuable for fish production than the same increase would be on another creek.

TABLE 14

Mission Creek Sub-basin
Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)
ROA3	199
ROA4	199
ROA5	199
ROA6	199
ROA7	199
ковз	199
ROB4	199
ROB5	199
ROB6	199
ROB7	190
Storage Capacity	221

The allocation of water for downstream uses in Mission Creek is different from most of the other tributaries. A significant amount of water is allocated towards flows for sport fish production. particularly in the month of June when Kokanee requirements are highest. Agricultural water use is high compared to other tributaries, but is small relative to the amount of water allocated towards flows for the sport fishery. Domestic and other water uses are relatively low (Figure 21).

The storage and release patterns for Mission Creek are fairly typical of the tributary systems. Storage is again brought to a maximum during the month of June and then gradually depleted throughout the rest of the year. The amount released is at a maximum in May during the freshet when all the run-off cannot be captured in storage. Only small amounts of water are released in June in order to build up storage levels. The amount of water released increases in the summer months and declines in the fall and winter (Figure 22).

FISH REST FIGURE 21: MISSION CREEK SUB-BASIN OCT SEPT DOMESTIC-INDUSTRIAL ALLOCATION OF WATER AUG JULY JUNE AGRICULTURE MAY 30 28 26 20 0 22 8 9 0 ∞ 2 (Thousands) ACRE FEET

REST OCT FIGURE 22: MISSION CREEK SUB-BASIN SEPT Release AUG STORAGE AND RELEASE + JULY Storage JUNE MAY BEGIN 20 19 18 16 5 10 12 0 ACRE FEET (Thousands)

8. Okanagan Lake Sub-basin

The water supply and demand conditions in this sub-basin are considerably different from the tributary sub-basins discussed in previous sections. For the mainstem reaches in general, there is a significant surplus of water in an average run-off year. The Okanagan Lake sub-basin receives all of the remaining flows and return flows from the developed tributaries as well as the natural run-off from several lesser developed streams. As a result, the total volume of supply is large compared to present and potential uses and all final use activities are at maximum possible levels in the optimal solution as shown in table 15.

TABLE 15

Okanagan Lake Sub-basin
Optimal Level of Final Use Activities

Activity	Optimum	Maximum	Shadow Price
Irrigated Acres	1,911	1,911	800
Lake Levels For Recreation			
period 2	336.8	336.8	N.A.
period 3	336.8	336.8	N.A.
period 4	336.8	336.8	N.A.
period 5	336.8	336.8	N.A.

Note: Lake levels are shown as amount of storage in thousands of acre-feet. Shadow prices are not applicable to lake levels.

Because all final use activities can be fully supplied by water, there is no value to an additional unit of water in the Okanagan Lake sub-basin and shadow prices on all water supply variables are zero as shown in table 16. This is true for all sources of water supply including run-off, remaining flow from upstream tributaries and return flows.

TABLE 16

Okanagan Lake Sub-basin

Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)
Runoff (all periods)	0.00
Return flows (all periods)	0.00
Remaining flows (all periods	0.00
Storage Capacity (all period	s) 0.00

The amount of water allocated for domestic/industrial uses is high relative to other sub-basins primarily because part of the water supply for greater Kelowna is drawn from the lake. There are also several developments near the shoreline which use Okanagan Lake water. In contrast there is little agricultural development that draws water directly from Okanagan lake as it is generally more economical for irrigation districts to use tributary water.

Figure 23 shows the allocation of water to consumptive uses.

The pattern of storage and release for Okanagan Lake is in Figure 24. The storage is operated in a similar fashion to most of the reservoirs on the tributaries, with a build-up of storage in the freshet and gradual depletion during the summer months. The build-up takes place in order to reach lake levels that maximize recreational values. The amount of usable storage in the lake is immense compared to the reservoir storage the on For example a peak storage of level of over 400,000 tributaries. acre-feet is reached in June compared to a maximum storage level of 18,000 acre-feet on Mission Creek and 10,000 acre-feet on Penticton Creek. Most of the other tributaries have less than 5,000 acrefeet of usable storage.

The amount of water released is very small (less than 20,000 acre-feet per month in the summer) compared to the amount in storage. This suggests that there would be considerable flexibility in operating the system to meet drought year conditions, as extra water could be supplied from lake storage. Release from Okanagan Lake is relatively constant, increasing only slightly in the winter months. This is in contrast to the patterns ofrelease from the tributary reservoirs which were often irregular. The even pattern of release from Okanagan Lake desirable because of flow requirements for sockeye salmon and for intake operation. The large storage capacity makes this even release possible.

REST FIGURE 23: OKANAGAN LAKE SUB-BASIN ALLOCATION OF WATER OCT DOMESTIC-INDUSTRIAL SEPT AUG JULYJUNE AGRICULTURE MAYŊ 0 (spubsnoul)

ACRE FEET

REST FIGURE 24: OKANAGAN LAKE SUB-BASIN OCT SEPT Release AUG STORAGE AND RELEASE + JULY Storage JONE 山 MAY BEGIN 400 350 -300 250 -100 50 -0 200 150 450 ACRE FEET (Thousands)

9. Skaha Lake Sub-basin

This sub-basin also has a high ratio of water supply to water demand. For most months, consumptive demands can be fully met with about 1,000 acre-feet of water. This requirement is only a fraction of the available water released from Okanagan Lake of about 20,000 acre-feet per month. Thus final use activities are at maximum possible levels as shown in table 17.

TABLE 17

Skaha Lake Sub-basin
Optimal Level of Final Use Activities

Activity	Optimum	Maximum	Shadow Price (\$)
Irrigated Acres	1,746	1,746	800
Lake Levels For Recreation			
period 2	7.07	7.07	N.A.
period 3	7.07	7.07	N.A.
period 4	7.07	7.07	N.A.
period 5	7.07	7.07	N.A.

Note: Lake levels are shown as amount of storage in thousands of acre-feet. Shadow prices are not applicable to lake levels.

Because of the excess water supplies in the Skaha Lake subbasin, shadow prices of all water supply activities are zero as shown in table 18.

TABLE 18

Skaha Lake Sub-basin
Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)	
Runoff (all periods)	0.00	
Return flows (all periods)	0.00	
Remaining flows (all periods)	0.00	
Storage Capacity (all periods)	0.00	

The allocation of water to consumptive uses is shown in Figure 25. Agriculture is the predominant user in the summer months while only a small amount of water is supplied to domestic/industrial activities. Total water use supplied directly from Okanagan Lake is not high, because the major population center of Penticton and a significant portion of the agricultural lands are supplied by the tributaries.

The pattern of storage and release is somewhat different from Okanagan Lake (Figure 26). Skaha Lake has only a fraction of the storage of Okanagan Lake, so excess water must be released immediately. This results in a slight peak in release in May with lower releases occurring in the summer months. Storage is increased in the summer in order to obtain optimal lake levels for recreational activities.

REST SKAHA LAKE SUB-BASIN OCT DOMESTIC-INDUSTRIAL SEPT ALLOCATION OF WATER AUG JULY 25: JUNE FIGURE AGRICULTURE MAY 0.9 0.8 9.0 0.5 0.4 0.3 0.2 0.7 0.1 ACRE FEET (Thousands)

REST OCT FIGURE 26: SKAHA LAKE SUB-BASIN SEPT RELEASE AUG STORAGE AND RELEASE + JULY STORAGE JONE MAYBEGIN 240 -220 -180 -120 -100 80 — 20 — 200 160 -**- 09** 40 260 140 (spubsnoul) ACRE FEET

10. Okanagan River Sub-basin

Despite having the heaviest agricultural demands for water of all the sub-basins, there is still a large surplus of water in the Okanagan River sub-basin. The river is mainly fed by the continual releases from Okanagan and Skaha Lakes with some additional water being supplied by tributaries along the way. Only one activity, agriculture, is included in the objective function of the the model for this sub-basin. Because of the surplus of water, agriculture is at a maximum level in the optimal solution.

Because all final use activities can be fully met with current supplies, there is no value for additional water supplies in this sub-basin. All water supply activities have a zero shadow price as shown in table 19.

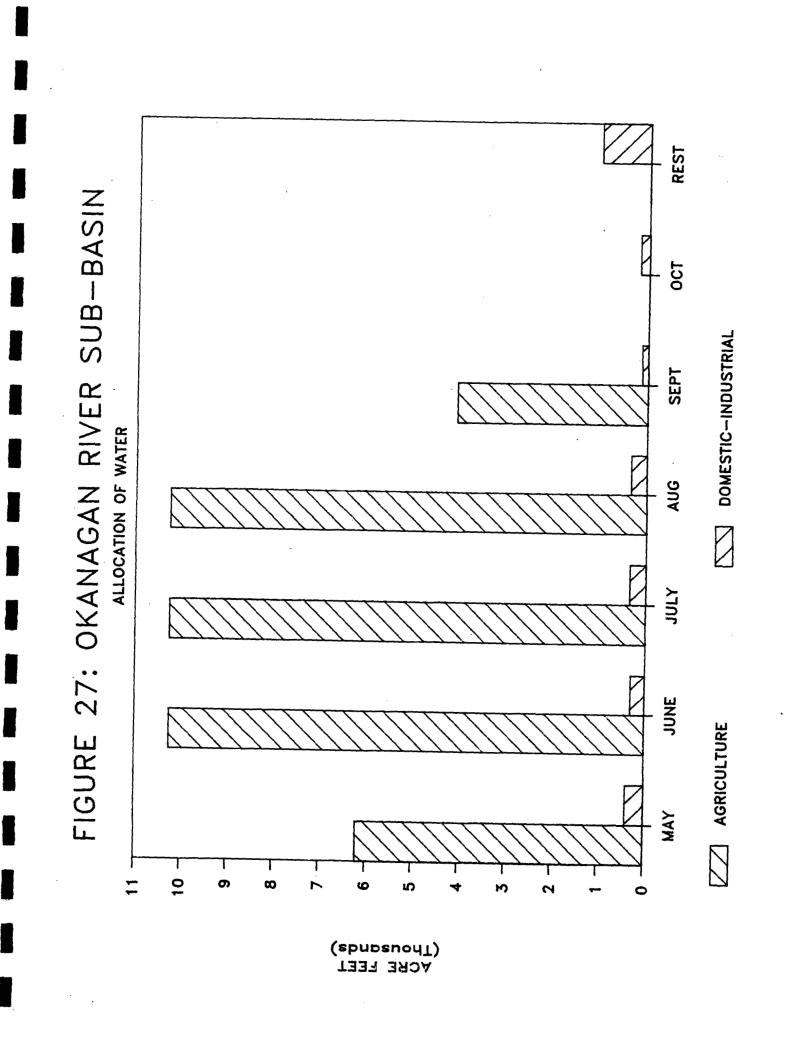
TABLE 19

Okanagan River Sub-basin
Shadow Prices on Water Supply

Supply	Variable	Shadow Price (\$)	
Runoff	(all periods)	0.00	·
Return	flows (all periods)	0.00	
Remain	ing flows (all periods	0.00	

The Okanagan River reach has the most water allocated towards agriculture of all the sub-basins. As a result the summer demands are very high as shown in Figure 27. Only a small amount of water is required to meet the needs of domestic/industrial users in this sub-basin.

The amount of storage in the Okanagan River sub-basin relatively small and was not included in the model. For purposes of comparison, the release (outflow) from this reach is shown along with the inflow from upstream reaches in Figure 28. Because there is no storage, the outflow and inflow patterns are almost identical. Outflow is slightly less than inflow in the spring and summer months because of diversions for agricultural and domestic In the winter months, when there is no agricultural diversion, outflow is slightly greater that inflow because of additional run-off which flows directly into the river. inflow and outflow peak in the freshet, decline in the summer months and then increase slightly in the winter. The pattern of inflow and outflow is quite regular throughout the year comparison to tributary flows. This is the result of the upstream regulation made possible by Okanagan Lake storage.



REST FIGURE 28: OKANAGAN RIVER SUB-BASIN OCT SEPT INFLOW AUG INFLOW AND RELEASE + JULY RELEASE JUNE MAY BEGIN 260 -240 180 -120 -100 - 09 20 — 220 -40 200 280 160 80 140 (Thousands)

11. Osoyoos Lake Sub-basin

Although there are fairly extensive demands for water from Osoyoos Lake, the supply of water from Okanagan River is great enough to create a substantial surplus in an average year. Agricultural and domestic/industrial demands account for about 4.000 acre-feet per month in peak periods while an additional 6.000 acre-feet of water per month is accounted for by American demands. Inflows from Okanagan River rarely fall below 20,000 acre-feet per month so there is a surplus of several thousand acre-feet even in peak periods. All final use activities are at their upper bounds in the optimal solution as shown in table 20.

TABLE 20
Osoyoos Lake Sub-basin
Optimal Level of Final Use Activities

Activity	Optimum	Maximum	Shadow Price
Irrigated Acre	s 2,811	2,811	800
Lake Levels For Recreation			
period 2	16.98	16.98	N.A.
period 3	16.98	16.98	N.A.
period 4	16.98	16.98	N.A.
period 5	16.98	16.98	N.A.

Note: Lake levels are shown as amount of storage in thousands of acre-feet. Shadow prices are not applicable to lake levels.

As in the other mainstem reaches, shadow prices on all water supply variables are zero as shown in table 21.

TABLE 21 Osoyoos Lake Sub-basin Shadow Prices on Water Supply

Supply Variable	Shadow Price (\$)
Runoff (all periods)	0.00
Return flows (all periods)	0.00
Remaining flows (all periods)	0.00
Storage Capacity (all periods)	0.00

Figure 29 shows that agriculture is the predominant user of water in this sub-basin, accounting for almost all of the water demands in the summer months. The high agricultural demands occur because of a combination of hot dry climate, sandy soils and extensive orchard and vineyard developments. Domestic/industrial use of water in this sub-basin is relatively small.

The storage and release patterns during the year are shown in Figure 30. Because there is not a great deal of storage on the lake, high inflows require that significant amounts of water be released. At the same time, maximization of recreational values causes the model to keep the lake above a certain minimum elevation.

REST OSOYOOS LAKE SUB-BASIN OCT DOMESTIC-INDUSTRIAL SEPT ALLOCATION OF WATER AUG JULY FIGURE 29: JUNE AGRICULTURE MAY 7.2 0.5 0 د. 3.5 2 M ACRE FEET (Thousands)

REST FIGURE 30: OSOYOOS LAKE SUB-BASIN OCT SEPT RELEASE AUG STORAGE AND RELEASE + JULY STORAGE JUNE MAY BEGIN 20 🗗 280 -260 -140 -240 -220 -180 120 -100 0 300 80 60 200 160 40 (spubsnoq1) ACRE FEET

It can also be seen from Figure 30 that it would be possible to change the storage and release patterns without affecting the value of the objective function. This relates to the problem of a non-unique maximum that was mentioned at the beginning of this chapter. For example, in the months of May and June, storage is kept at maximum levels. These levels are well above the minimum which maximizes recreational values. The storage is not required to meet needs in later months because of the continuous supply of water from Okanagan River. Therefore, lower storage levels and higher releases could take place in May and June without affecting the objective function value.

D. Summary of Model Solution

The solution to the model illustrates a general difference between the supply/demand balance for the tributary reaches and for the mainstem reaches. Most of the tributaries do not have enough water to supply domestic/industrial needs and to utilize fully all irrigable lands and potential for sport fish production. In contrast, the four mainstem reaches all have a surplus of water in the average run-off year represented by the model and all domestic/industrial, agricultural and recreational users can be supplied to maximum possible levels. The difference between the tributary and mainstem systems demonstrates the need for disaggregation of the model into a number of reaches. Without this disaggregation, the surplus existing on the mainstem reaches would have masked the shortages on the tributaries.

The allocation of water to final use activities is similar for the seven tributary reaches. In general, as much water as possible is supplied to agricultural needs before any flows are allocated towards sport fish production. However, on reaches where there is potential for resident trout production, a certain amount of water is supplied to meet their requirements. This can usually be accomplished without interfering with agricultural downstream. In contrast, the spawning trout and kokanee compete directly with agriculture for downstream water. and the solution will not supply any water for their needs unless agricultural demands have already been fully met.

Because of the relative shortage of water on the tributaries, there is usually a positive shadow price on water supplies. Shadow prices range from a few dollars to several hundred dollars per acre foot. depending on the amount of water currently available and the potential for agricultural and sport/fish production. tributaries there is also a positive shadow price on reservoir storage capacity. Increasing storage capacity would have value because it would enable more of the peak run-off to be captured and stored for later use. The usual pattern of storage operation the optimal solution is to bring storage to its maximum possible level during the freshet and to gradually deplete it during the dry summer and fall months. Release patterns are somewhat more variable, depending on the volume of run-off and the sport fish populations on the system.

On the four mainstem reaches there is a surplus of water in all time periods of the model. All final use activities are supplied to their maximum possible levels. Lake levels are maintained which maximize recreational values and minimum flow requirements for spawning salmon are easily satisfied. The shadow price of an additional unit of water is zero in the average year represented by the model. The shadow price of storage on the mainstem system is also zero since there would be no point in retaining more water in the lakes if a surplus already exists. The pattern of storage and release in the model is governed primarily by recreational lake level requirements and to some degree by the disposal of excess inflows.

It was also concluded that the model solution, while maximizing the value of the objective function, was not a unique maximum: there are other solutions which will result in the same maximum being reached. This conclusion was based on the fact that there were several alternative methods of disposing of the surplus water on the mainstem which would not affect the level of any of the final use activities. This was not considered a serious problem with the model, given that its purpose was to demonstrate the use of the linear programming techniques in maximizing the benefits of water use. If the model were also to include floodcontrol objectives, this would reduce the disposal options available, and it is more likely that a unique optimum would be found.

V. SUMMARY AND CONCLUSIONS

A. Summary

The purpose of this study was to demonstrate the use of linear programming to optimize allocation of water among competing uses in a river basin. A linear programming model was developed which determined optimal allocation of water on a monthly basis over 11 reaches of the the Okanagan River Basin. Several competing uses including agriculture, domestic/industrial, sport fish production and water-based recreation were included. The solution to the model determined both the optimal allocation of water among these activities and the optimal storage management of the system.

There are three notable features of the model relative to previous studies which have utilized linear programming or other optimization techniques. First, the model is developed for planning rather than operational purposes and provides a framework for addressing longer term questions of water allocation and storage requirements. Second, the model includes as many water use activities in the objective function as possible. In so doing. necessary to have either the net economic benefit associated with these activities or a measure of the value of water to each Most previous studies have not considered such a broad range use. of competing water use activities in their determination of the optimal allocation of water, and have often focussed on a detailed analysis of just one or two activities. The third notable feature

of the model is that it uses a single river basin. disaggregated into a number of reaches. as the geographical unit for water allocation. While several previous studies have also used the river basin as a natural planning unit. many optimization models have considered only a single system of reservoirs controlled by a particular management agency. Other models have been constructed on a larger geographic scale than a single river basin in order to address such issues as regional water deficits and the need for inter-basin transfers.

The model was constructed to represent a single river basin in order to be consistent with the majority of planning studies in which Inland Waters Directorate has been involved. The ability of the model to choose among multiple water use activities is also consistent with the comprehensive nature of previous Canada Water Act studies. The dissagregation of the river basin into sub-basins or reaches allowed the model to address localized problems of water shortages which can often be disguised in aggregate supply-demand comparisons. In terms of the scope and scale of the model, it could readily be integrated into future planning studies under the Canada Water Act.

The Okanagan River Basin was selected because much of the basic water supply and water use data were available from a previous Canada Water Act Study. This data had to be supplemented to some extent from other sources, particularly when estimating the economic value of water to direct and indirect users. Since the project was intended for demonstration purposes only, efforts to obtain additional data and information were kept to a minimum. In

some cases, rough estimates or assumptions were used rather than undertaking additional studies to obtain more reliable information. These limitations should be considered before any conclusions regarding allocation of water in the Okanagan River Basin are drawn from the model in its present form.

The model, as applied to the Okanagan River Basin, has only a single-year time horizon and represents an average run-off year. Constraints are imposed which state that all end-of-year storage levels be greater than or equal to beginning-of-year levels. constraint rules out any solutions to the model that would result longer term depletion or "mining" of the stored water in the The model, in its present form, could be used as a longer term allocation model if a single assured run-off scenario were incorporated, and if any inter-year trade-offs in the use of water were ruled out. These restrictions would limit its usefulness in many river basins. However, the model could be used as the basis more complex models which would consider both stochastic run-off and inter-year trade-offs in storage. Construction of sophisticated models was not undertaken at this stage because of the considerable extra work and computer time that would be required. The basic concepts and data requirements for these types of models are described in the concluding section.

The model includes seven time periods within the year, 11 reaches, and numerous activities relating to the use of water and management of the system. As a result it is quite large, with a total of 830 constraints and 1039 activities. The model was constructed as a linked set of 11 sub-models, one for each reach. This feature simplifies the understanding of its structure and allows for easy incorporation of changes and testing of the model. The computer costs and time necessary to obtain an optimal solution were also reasonable.

The solution to the model gives optimal levels of a number of variables which are of interest to planners and managers of the water resource. The optimal level of final-use activities is determined for each reach as is the amount of water allocated to these activities in each reach and time period. At the same time, the model also solves for the optimal pattern of storage and release for the lakes and reservoirs in the basin. Shadow prices are calculated for supply of water and for storage capacity in all reaches and time periods.

B. Conclusions

1. Practical Application of Linear Programming

The application of linear programming to water use allocation in the Okanagan River Basin demonstrates that it is a practical and feasible tool for comprehensive river basin planning. The application to the Okanagan River Basin showed that data, manpower and computer requirements are within the scope of resources that

have been available in previous Canada Water Act agreements. The modeling technique was found to be flexible enough to approximate the basic physical and socio-economic relationships encountered in river-basin planning. In general, these relationships could be easily represented by linear equations. In some cases, non-linear relationships had to be approximated by a series of linear segments, but it was not felt that this caused any significant errors in the results.

of the data required for the model were taken from Most under a Canada Water Act agreement for the previous studies done These data were originally intended for Okanagan River Basin. planning purposes such as forecasting, comparing water supply and use and assessing the need for improved infrastructure in It was found that the data could also be supply systems. water programming optimization model, with little in the linear Additional data and information were required for the alteration. economic relationships in the model relating to net benefits from final-use activities and valuation of water. This suggests that optimization models could be integrated Canada Water Act into studies if sufficient resources were directed towards Planning estimation of the economic value of water to competing users.

Given the availability of the basic data, the time required to construct the model was not great. Application of the model to another basin would be expected to take even less time since the same formulations could be used for many of the basic equations. The large size of the model did not prove to be prohibitive in terms of the computer costs required to solve it. Costs were low

enough to conclude that extensive sensitivity analysis and expansion of the model could be undertaken at reasonable expense.

An additional question to consider when assessing the feasibility of the model is: how much confidence do we have in the solution? Since the model is a normative tool, it does not give results which can be tested empirically. The optimal solution depends upon all of the data, relationships and assumptions built into the model. Insofar as any of the internal structural relationships are incorrect, the optimal solution could also be incorrect. Thus the only way to verify the model solution is to check that each structural relationship in the model is correct. As noted, some of the structural relationships are only rough estimates and considerably more work would have to be done before we could have reasonable confidence in the solution.

An additional method of verifying the model is to compare activity levels in the optimal solution to levels that might reasonably be expected. For certain activities there are obvious bounds within which the activity levels should lie. For example, if flows in Okanagan River exceed channel capacity in the optimal solution, then it would be reasonable to conclude that the model was specified incorrectly. Other relationships that are apparant in the optimal solution can be compared to findings from previous studies and models. For example, the optimization model indicated that a surplus of water existed on the mainstem reaches of the system in an average run-off year. This finding is consistent with the conclusions from the simulation model of water supply and water use which was constructed during the Okanagan Basin Study. The

optimization and simulation models were also consistent in that both indicated that there were conflicts in water use between fishery and agricultural requirements on many of the tributaries to the mainstem system. These similarities support the reliability of the model structure. However, it should be restated that many of the structural relationships of the model were approximated with rough data and assumptions. These would have to be refined before full confidence could be placed in the results of the model.

2. Methods for Improving the Model

There are a number of ways in which the model could be both in its specific application to the Okanagan River improved. Basin and in its application to river basins in general. applying the model to the Okanagan River Basin, some obvious areas for improvement would be in the estimation of economic values associated with water use activities. More precise information on the net benefits of irrigation and sport fishing would greatly The effect of lake level improve the usefulness of the model. fluctuations on recreational values would also have to be studied more closely in order to better specify this relationship in the Other areas for improvement include better specification of model. the relationship between flows and fish populations and inclusion of the effects of lake levels on kokanne production.

There are a number of ways in which the model could be improved that would apply to all river basins. These improvements relate to the conceptual structure of the model and concern such factors as stochastic run-off, inter-year trade-offs in water use.

incorporation of industrial and domestic demand functions. and sub-models for optimizing the amount of irrigation water applied. These improvements are discussed below.

a. Stochastic Water Supply and Inter-Year Trade-offs

In many river-basins, a model which considers the possibililty of inter-year storage of water and stochastic run-off would be more useful than the single year model developed in this study. A dynamic programming approach would be able to incorporate both of these considerations. Dynamic programming is a method representing a sequential decision making process which considers both the current state of certain variables plus future actions. The solution of a dynamic programming model would give the optimal use of the water resource for a given level of storage or expected In arriving at the optimum, explicit consideration would be given to the trade-off between using the water in the current period or saving it for use in future periods. The linear programming model used in the present study could be used as the basic building block for such a dynamic programming model.

Numerous engineering studies have used dynamic programming for short term and real time operational models of reservoir systems. However, most of these studies have considered fixed demands for water from most sectors and have optimized over only one or two variables. An example of such a study in a Canadian setting is Silver, Okun and Russel (1972) which used dynamic programming to maximize the firm power production from the ALCAN hydroelectric system. Examples of the use of dynamic programming in planning

models of water allocation can be found in Worthington, Burt and Brustkern (1985). Brown and Deacon (1972) and Burt (1964).

b. Valuation of Industrial and Domestic Water

In the application of the model to the Okanagan River Basin, no explicit values were given to domestic or industrial water use. Instead, constraints were imposed that restricted the model to supplying a fixed amount of water to these uses before allocating the remainder to other uses. Knowledge of the willingness to pay (consumer's surplus) for water use by industrial and domestic users would allow incorporation of these values directly into the objective function and the model would be free to optimize over industrial and domestic uses as well as agricultural, fishery and recreational uses. This would be an important improvement in the model if it were to be applied to river basins where there are significant industrial and domestic demands.

Incorporation of willingness to pay values for industrial and domestic water into the model would be fairly straightforward if these values were known. As outlined in chapter three, the willingness to pay is the area under the demand curve for water. This area can be translated into a marginal value function which would be directly included in the objective function. The marginal value function is non-linear but could be approximated by linear segmentation in a linear programming model or alternatively a non-linear programming model could be used. The important point is that once the demand functions for water are known, the problem of incorporating them in the model is relatively easy.

Recent work by Renzetti (1986a.1986b) focussed on econometric estimation of industrial water demand in British Columbia for a number of different industries. These studies relied for a large part on data from a national survey of industrial water use. and the scope of such estimations could be expanded to include other areas of Canada. The estimated demand functions could be used to generate willingness to pay figures which could be incorporated into the optimization model for river basins where the type and size of industrial establishments are known.

There has not been a great deal of work done on the estimation of domestic water demands in Canada. The most recent study was carried out by Sigurdson (1982) for Saskatchewan and Manitoba. Other Canadian studies include Sewell and Roueche (1974) which examined municipal water demands in Victoria, B.C. and Grima (1972) which looked at residential demand for regions in Ontario. These studies do not provide a sufficient basis for any generalized estimates of domestic water demands which are sensitive to climate and socio-economic variables that vary across regions of Canada. In order to estimate domestic water demand functions which could be used in optimization models, it would be necessary to use a much broader data base or to estimate demand functions specific to river basins under study.

c. Improved Modeling of Agricultural Water Use

In its present form the model uses a fixed application rate of water per acre based on observed irrigation practices in the Basin. model does not have the capacity to optimize the application rate which is a significant limitation given the importance of irrigation in the region. It would be possible to develop a sub-model to determine the optimum per acre application rate using information that relates yields to the amount of water applied over unit area. Recent work by Wigington and Short (1985) simulated the relationship between per-acre yields and irrigation rates in Okanagan orchards. This work could be used as the basis for an irrigation sub-model which could then be integrated into the optimization model for the Okanagan Basin. Further work by Wigington (1985) expanded the analysis to include other crop types for various soil and weather conditions. Using this information it would be possible to determine optimal irrigation rates for a number of different crops over a variety of geographical areas.

d. Water Quality Considerations

In its present form the model does not incorporate any values or requirements related to water quality. However, water quality will be affected by the activities in the model through such pathways as industrial and domestic discharge and leaching and erosion from agricultural lands. Furthermore, withdrawal of water from surface supplies may result in an increased concentration of effluents or nutrients in the unused water. Therefore, water use should not be considered in isolation from water quality. The

issue of optimal water quality is complex and incorporates such factors as ecosystem health, human health, aesthetic values, recreational uses and international obligations. For the purposes of water use modeling, a simpler approach would be to incorporate established water quality objectives into the model in the form of limits on activities which pollute the water system and constraints on activities which withdraw water needed for effluent dilution.

e. Further Testing of the Model

Further testing of the model could provide a more realistic evaluation of the applicability of linear programming to river Application to a another river basin with a basin planning. different set of economic activities would provide further testing of its capabilities to model a wide range of demands for water. additional test of the model would be to utilize it for planning purposes, rather than as a demonstration of the methodology. application would require a much more rigourous estimation of the structural relationships and demands for water than was undertaken in the demonstration model for the Okanagan River Basin. Extensive sensitivity analysis would have to be undertaken in order to determine the key variables in the model and to assess the reliability of the model solution. Such a project would require considerably more resources than were used in applying the model to the Okanagan Basin.

Initial work has begun in applying the model to the portion of the South Saskatchewan River Basin located in Saskatchewan. This should provide the opportunity to test the model in a situation where there are a variety of water users competing for relatively scarce supplies. A Canada Water Act study planned for this Basin will generate a significant amount of information on water supply and demand which could be used in the optimization model. This information would allow for a realistic application and assessment of the model. The model solution could also provide information on the allocation of water in the Basin which should be useful in the formulation of recommendations under the Canada Water Act study.

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APPENDIX

Complete Model Solution

This appendix shows the complete model solution reproduced from the MPSX printout. The first part of the printout summarizes the simplex search for the solution which maximizes the objective function. Phase one first searches for a starting basis or feasible solution. Once a starting solution has been found, phase two begins and further feasible solutions are examined, until the maximum value of the objective function is obtained. The printout then specifies the right hand side value of each constraint and the level of each variable (activity) in the optimal solution. Some conventions are used in the names of the variables and rows as outlined below.

1. Variable Names

All variables are referred to as "activities" in the optimal solution. These activities are given names, similar to the variable names used in chapter three. Each activity name is given a prefix to indicate the sub-basin to which it belongs. Immediately after the prefix there is a single digit from 1 to 7 which indicates the time period for the variable. The prefixes and the corresponding sub-basins are:

- A Trout Creek Sub-basin
- B Mission Creek Sub-basin
- C Peachland Creek Sub-basin
- D Powers Creek Sub-basin

E - Equesis Creek Sub-basin

F - Kelowna Creek Sub-basin

G - Penticton Creek Sub-basin

2. Row Names

All constraints are given a name which appears under the heading called "row" in the solution. All row names are appended with the letter "C" in order to distinguish them from names of activities. As with the activity names, row names are prefixed with a letter and number indicating the sub-basin and time period. The next three or four letters constitute a short code indicating the nature of the constraint. The basic codes used and the corresponding constraints are:

ACR - maximum acreage available for irrigation

ACRD - irrigation demands

RTR - return flows

STR - relationship between spawning trout and flows

KOK - relationship between kokanee and flows

DWN - downstream relationship between supply and demand

SPD - downstream supply

SPU - upstream supply

BAL - storage balance

REM - remaining flow to next reach

RET - return flows within the sub-basin

WAS - wastage factor

REC - relationship between recreation and lake levels

```
TITLE('MAINMOD')
MOVE(XDATA, 'BASIN')
MOVE(XPBNAME,'STGRAGE')
MOVE(XBOUND,'BND1')
MOVE(XGBJ,'OBJFUN')
MOVE(XRHS,'RHSA')
                                                                                                                          SAVE('NAME','BASE')
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SCRATCH2=-MPSX.S2
SYSPRINT=UNIT=SPRINT
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2- COLUMNS SECTION

O MAJOR ERROR(S). O MINOR ERROR(S) -

3- RHS'S SECTION.

RHSA

O MAJOR ERROR(S). O MINOR ERROR(S)

5- BOUNDS SECTION.

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C2KDK1C

PROBLEM STATISTICS

0.22 3427 LP ELEMENTS, DENSITY = 1869 VARIABLES, 830 LP ROWS,

THESE STATISTICS CONTAIN ONE SLACK VARIABLE FOR EACH ROW

O MAJOR ERRORS. 1 MINOR ERRORS,

0.02 TIME =

MAX SCALE BOUND

= BND1

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MAXIMUM PRICING NOT REQUIRED - MAXIMUM POSSIBLE

NO CYCLING

6664 46648 21240 57960 CORE 3576 SIZE 6664 4248 6440 NUMBER **⊳** ₽ 0 POOLS
H.REG-BITS MAP
BOUND VECTOR
WORK REGIONS
MATRIX BUFFERS
ETA BUFFERS

FIXED BOUNDED 430 O .FREE. NORMAL 399 667 TDTAL 830 1039 ROWS (LOG.VAR.) COLUMNS (STR.VAR.) 18 MATRIX RECORDS (WITHOUT RHS'S) ı 0.22 3427 ELEMENTS - DENSITY =

WRITE

0.03 TIME = OPTIMIZE SYSTEM MACRO CALLED

O.03 MINS CRASH

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SETUP

233 AFTER CHERCHE 52 AFTER PASS B COMPLETED

193 SELECTED STRUCTURALS

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RHS = RHSA OBJ = OBJFUN PRIMAL

TIME = 0.06 MINS. PRICING 5 SCALE = INVERT DEMANDED AFTER 573 MAJOR/ 573 MINOR ITERATIONS - FREQUENCY

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45897183.5727

FUNCTIONAL RESTRAINTS BOUNDS....

OBJFUN RHSA BND 1

SECTION 1 - ROWS

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	SLACK ACTIVITYLOWER	75.79082 136.42348	90.56197						•						9407.21962			• •			•				17952,33083	13419,54685	4243.25072	1694.96143	124.55592					 		٠
	ACTIVITY	75.79082- 136.42348-	90.56197-									•	•	. 25050	9407.21962	•					•			•	17952,33083-	54685	4243.25072- 4375 45804-	96143	124.55592-			 •		 		 ٠
MAINMOD	.ROW AT	STR1C BS		A1KOK1C UL						A2DWNC EQ								SPDC							A7BALC EQ A1SPUC BS		A3SPUC BS			A7SPUC UL			ASREMC EQ			
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⋖	816	A7RETC	E 0	•					
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4	818	AZWASA	EO	•		•			
	8 19	ASMEA	EO	•			٠		13.99810
	820	A4WASA	E C	•		•	٠	•	13.99810
	821	A S WAS A	C				•		13.99810
	822	AGWASA	C				•	•	13.99810
	823	AZWASA	EO	•			•	•	13.99810
4	824	ATWASB	0	•			•		
<	825	A2WASB	Е				•		
	826	A3WASB	EO	•			•		13.99810
	827	A4WASB	E			•	•	•	13.99810
	828	A5WASB	0					•	13.99810
	829	AGWASB	EO	•		•	•		13.99810
	08	ATWASB	E C			•	•		5.80728

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LOWER LIMIT.	· · · · · · · · · · · · · · · · · · ·	168400.00000	71720.00000 42500.00000 23500.00000 18010.00000 19930.00000 115650.00000	29900 . 00000 38700 . 00000 47000 . 00000 33800 . 00000 21600 . 00000 5759 . 00000 7109 . 00000 7108 . 00000 7108 . 00000 7108 . 00000
INPUT COST	00000			
ACTIVITY	110672.72832 68335.93965 8780.28988 7303.91551 8887.06931 7958.14284 35208.95712 2895.16500	687.41007 1144.31138 1144.31138 1144.31138 458.27338 168400.00000 340137.88639 439851.28328 442602.73654 42602.73654 426572.7850 425572.78720	71720.00000 23500.00000 18010.00000 19930.00000 115650.00000	29900.00000 38700.00000 47000.00000 33800.00000 21600.00000 46100.00000 5759.00000 7109.00000 8188.00000 7108.00000
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LOWER LIMIT.	17265.00000		7065.00000	7065.00000	14440,00000 11600,00000 2910,00000 1200,00000 860,00000 9750,00000
INPUT COST		. 15500 . 62600 . 62600 . 15500	800.0000	• • • • • • • • • • • • • • • • • • •	
ACTIVITY	17265.00000 5501.56815 6803.42310 7803.47475 6802.52310 4721.41980 3257.56485	336800.00000 336800.00000 336800.00000 336800.00000	1746.0000 682.03125 1133.76623 1133.76623 1133.76623 453.50649	7610.16025 8823.56401 8024.83049 7065.00000 6857.48200 13469.43200 10440.00000 4040.00000 1080.00000 774.00000	14440.00000 14600.00000 2910.00000 1200.00000 860.00000 9750.00000 17882.84675 153.73929
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. COLUMN.	01RETA 01RETA 02RETA 03RETA 04RETA 06RETA	01REC 02REC 03REC 04REC 05REC 07REC	SOACRIA SIACRIR SZACRIR SJACRIR SAACRIR SSACRIR SOSTO	\$2510 \$3510 \$4510 \$5510 \$6510 \$7510 \$7510 \$7800 \$38000 \$58000 \$58000	S1R0A S2R0A S2R0A S4R0A S6R0A S7R0A S1REL S2REL S3REL S5REL S6REL
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PAGE	REDUCED COST.			•				•			•													. 75000	3.00000	3.00000	00067									800.00000					•	•	٠						
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	LOWER LIMIT.	1740.00000	2200.00000	2450.00000	2440.00000	1850.00000	1180.00000	2700.00000	58.00000	65.00000	73.00000	80.00000	87.00000	58.00000	305.00000	•	•								•		•						•	•			•					20590.00000	17170.00000	4440.00000	6880 00000	2200.00000	9240.00000		٠.
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	ACTIVITY	1740.00000	2200.00000	2450.00000	2440.00000	1850.00000	1180.00000	2700.00000	58.00000	65.00000	73.00000	80.00000	87.00000	58.0000	305.00000	301.87800	376 27200	406.17000	389.77200	350.67600	256.48200	841.95000		7065.00000	7065.00000	7065.00000	7065.00000		H 10700 0000	1160.0000	496.00000	444.73929	120.00000	86.00000	975.00000	8512.00000	6213.13869 40255 42469	10255.42169	10255.42169	4092.30769	20684.16000	20590.00000	17170.00000	8440.00000	6880 00000	2200 00000	9240,00000	33880.46566	9222.86071
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	.REDUCED COST.	440.00014-	440.00014-	440.00014-	440.00014-	398,80038-					440.00004-		440.00004-	. 0000	440.00004=	440.00004-		435.30007~			•	•			•										440.00004		•								40.000.	
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, PAGE 40	REDUCED COST.		•						· •	•		• •		9,69597			•		•		9.96752				t.90000-	-00006	-00006.	1.90000-	-0000-								•				•		
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 	ACTIVITY	137.03704	137.03704	37	137.03704	•	482.60870	•	. 70	51.	837.59900	37.	80.51646		277				299.48718		35040.00000		869.56298	1186.89091	. •		1204 89091	49.00000	702.12354			1626.97531					447.344/3		(2)		184.0/134		
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MA	. COLUMN.	DIRTRER	DZKIKZK	D4RTR2R	DSRTR2R	DGRTR2R	D7RTR2R	DIBTERS	DZRTR3R	Dartrar	D4RTR3R	DSRTR3R	DERTROR	DOSTR1A	DIFISIR	D2F1S1R	D3FIS1R	D4FIS1R	DSFIS1R	D7FIS1R	DOKOK 1A	D1DOM1R	D 10WN 1R	D2DWN1R	D3DOM1R	D3DWN1R	D4DUM1R	DSDOM1R	DSDWN1R	Debuga ALMAGA ALMAGA	D7DOM1R	D7DWN1R	0.1808W	DZREL	D2ROBW	Darel	Dakoss	D4RCBW	DSREL	DSROBW	DGREL	D7REL	
	UMBER	1441	1442	1443	1445	1446	1447	1440	1450	1451	1452	1453	1454	1456	1457	1458	1459	1460	1461	1463	1464	1465	1466	1468	1469	1470	1471	1473	1474	14/5	1477	1478	1478	1481	1482	1483	1484	1486	1487	1488	1489	1491	

REDUCED COST.	•				. +			٠						1.90000=									•		•						1.017.1	1.71017	1.71017	1.71017			1.71017	1.71017	1.71017	1.71017		398.18208	•		•	
UPPER LIMIT.	NONE	1131.00000	NONE Part 4 00000	3/54.00000	3754 000000		3754.00000		3754.00000	NONE	3754.00000		3754.00000	NON PNON	NON	NONE	NONE	NONE	NONE	NONE	NONE	NONE	NON		NON	NONE	NONE	NONE	2406.00000	1334.00000	213.00000	86.0000	86.00000	625.00000	4334.00000	2156.00000	497.00000	179.00000	176.00000	176.00000		8/1/.00000	NONE	NON PINON	FINON	NONE
LOWER LIMIT.		1131.00000			•								•	. 131						•									2406.00000	1334.00000	215.00000	86.00000	86.00000	625.00000	4334.00000	2156.00000	497.00000	179.00000	176.00000	176.00000		8717.00000	•		•	
INPUT COST		•	•	•		-					•			•	•						•		•	•											•						-	800.00000				
ACTIVITY	306.		2165.61656	2983.20547	3254 00000	193 5 1935		85	2231.70693	77.40774	1765.40697	77.40774	•	562.55626	768	250	313,12070					608		1753.23463			325.67134	419	2406.00000		215.00000	98.00000	86.00000	625.00000	4334.00000	_	497.00000	179.00000	176.00000	176.	452	717	962.	6603.78788	מ	
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. COLUMN.	D7ROBW	DOSTO	DIROAW	DISTO	S C F C C	DISSID	D3STO	D4ROAW	D4ST0	DSROAW	DSSTO	DGROAW	DESTO	D7ROAW	01570	DIRETA	DZRETA	D3RETA	D4RETA	DSRETA	DGRETA	D7RETA	DIREMA	DZREMA	A BURNA	DSREMA	DGREMA	D7REMA	D 1ROA	DZROA	Daroa	DSROA	DEROA	D7ROA	D1ROB	D2ROB	Darob	04808	DSROB	DGROB	D7ROB	BOACR 1A	B 1ACR 1R	B2ACR1R	0.400 A C	BSACR 1R
NUMBER	6	A 1493	1494	1495	1400	1497	1499	1500	1501	1502	1503	1504	1505	1506	001	9051	1510	-	-	-	•	-	-	1517		1520	1521	1522	A 1523	T	1525	1527	1528	1529	A 1530	÷.	1532	1533	1534	_	A 1536	1537	1538	1539	4 4 4	1542

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PAGE	REDUCED COST.	8.08247															15, 13513-		•								•				•	7.72351			•	-08888 066		-08660.02		220.99990-		220.88890-	211,69993-	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		•		
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	LOWER LIMIT.			•		•		٠	٠	•		•				,	•		•			٠	٠	•	•	•		-	•	٠	• •		190.00000		248.00000	. 305		305.00000	٠	153.00000		133.00000))))		•		
	INPUT COST	10.00000	ē	•	٠	٠	٠	•	•	10.00000	•		•	•	•					•		٠		•	10.0000	-00010-	-00010-	-0000.	-0000		-00010-	10.00000	•	•	•	•			-	•	•		•					
	ACTIVITY	8		526.94118	941	526.94118	526.94118	263.47059		16.38933	5.02740	5.02740	5.02740	5.02740	5.02740	N	17.62293	•		2899,09384	4763.18025	3219.28215	823.75514		. 3536		28266.13929		60.52836	2402.28013	5882		190.			35117.92717							2535, 28013	11414,58824			531.96858	
MAINMOD	ΑT	٦,	BS	BS	82	BS	BS	BS	BS	BS	BS	BS	BS	82	BS	BS	S -	: =	i K	BS	BS	BS	BS	BS	BS	88	82	82	200	א מ מ	2 00	3 5	EQ	BS	0 0	20 0	9 V	0	BS	EQ	BS	Э П	82	ט נ ט נ	BS	BS	8 8 8) Ì
MAI	. COLUMN.	BORTR 1A	B 1RTR 1R	B2RTR1R	B3RTR1R	B4RTR1R	B5RTR1R	BGRTR1R	B7RTR1R	BORTR2A	B1RTR2R	B2RTR2R	B3RTR2R	84RTR2R	B5RTR2R	B6RTR2R	B7RTR2R	RIPIDAD	RORTROR	BARTRAR	B4RTR3R	B5RTR3R	BGRTR3R	B7RTR3R	BOSTR1A	BIFISIR	B2FIS1R	B3FIS1R	B4F1S1R	BSF151K	RZFISIR	BOKOK 1A	B 1DOM 1R	B 1DWN 1R	B2DOM1R	BZDWN1R	RADWIN IR	B4DOM1R	B4DWN1R	B5DOM 1R	B5DWN1R	B6DOM 1R	BGDWN1R	B 7 D W N T R	BIREL	BIROBW	B2REL R2RORW	:
	NUMBER	1543	1544	1545	4	1547	4	₹	1550	1551	1552	1553	1554	1555	1556	1557	1558	1560	•	1562	1563	1564	1565	1566	1567	1568	1569	1570	1571	15/2	1574	1575	A 1576	1577	A 1578	1579	1867	1582	1583	1584	1585	1586	1587	ב ס ס ס ס ס ס	1590	1591	1592)))

REDUCED COST.		198.91980 198.91980 198.91980 198.91980 198.91980 198.91980
UPPER LIMIT.	7	NDNE 13433.00000 8492.00000 717.00000 345.00000 310.00000 1993.00000 59253.00000 38425.00000 1860.00000
LOWER LIMIT.	5857.00000	13433 .00000 8492 .00000 717 .00000 345 .00000 310 .00000 1993 .00000 59253 .00000 38425 .00000 1860 .00000
INPUT COST	TNPUT COST.	
ACTIVITY	3431 .06242 3538.25383 5295.14482 1674.16742 1674.16742 3751.25073 1445.54455 1089.73558 1445.54455 1089.73558 1689.40422 1669.40422 1669.40422 1669.40422 1643.56436 17981.00000 15981.00000 17781.00000 17781.00000 17781.00000 17781.00000 17781.0000000000000000000000000000000000	.
AT	A A A A A A A A A A A A A A A A A A A	
.COLUMN.	B3REL B3REL B4ROBW B4ROBW B5RCBW B5ROBW B5ROBW B7ROBW B7ROBW B7ROBW B7ROAW B7ROAW B3ROAW B3ROAW B3ROAW B3ROAW B3ROAW B3ROAW B3ROAW B3ROAW B3ROAW B3ROAW B6STO B7ROAW B6STO B7ROAW B7ROAW B6STO B7ROAW	B6REMA B7REMA B1ROA B3ROA B3ROA B5ROA B5ROA B7ROA B1ROB B1ROB B1ROB B3ROB
NUMBER	NUMBER NUMBER 1594 1594 1596 1596 1597 1598 1600 1600 1600 1601 1601 1612 1613 1627 1627 1628 1629 1627 1627 1628 1629 1627 1627 1627 1627 1627	1632 1633 1634 1635 1636 1636 1640 1640 1642 1642 1643 1644 1642 1643 1644 1644 1644 1644 1644 1644 1644

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. PAGE	ACTIVITYINPUT COSTLOWER LIMITUPPER LIMITREDUCED COST.
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MAINMOD

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1606.00000 1606.00000 10610.00000 6150.00000 NDNE NDNE NDNE NDNE NDNE	3986 . 0000 NDNE NDNE NDNE NONE 1594 . 0000 NONE NONE NONE NONE	552.00000 NDNE NDNE NDNE NDNE NDNE 1000.00000 NDNE NDNE NDNE NDNE	2000 0000 35.0000 35.0000 35.0000 39.0000 0000 35.0000 35.0000 35.0000
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1606.00000 1606.00000 10610.00000 2795.45455 4659.09091 4659.09091	3986.00000 249.90596 249.90596 249.90596 249.90596 124.99216 80.62280 86.62280 86.62280 86.62280 86.62280 86.62280	43.31140 304.60766 	161, 25707 290, 26272 1161, 05087 1451, 31359 35, 00000 3023, 96302 39, 00000 4694, 09091 35, 00000 4694, 09091 35, 00000
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	AORTR1A A1RTR1R A2RTR1R A5RTR1R A6RTR1R AORTR2A A1RTR2R A2RTR2R A3RTR2R A4RTR2R		
			1682 1683 1683 1683 1686 1689 1690 1691 1693 1694

REDUCED COST.		15.55189~	6.45189~																			15.55189									15.55189-																
UPPER LIMIT.	NONE	35.00000 NONE	257.00000	NONE	NONE	NONE	NONE	NONE	NONE	NONE	NONE	NONE	NONE	NON	NONE	NONE	NONE	NONE COOO		9307.00000	NONE	9307.00000		9307.00000		9307.00000	NONE	9307.00000	AGN COOO	•	NONE	NONE	NONE	NONE	NONE	NON P	A PACK	INCN	NONE	NONE	NONE	NONE	NONE	NONE	NONE	25104.00000	13263.00000
LOWER LIMIT.		35.00000	257,00000							•		•	-					. 0003	00000.0000	•			-	•		•					2000,00000		•	•			-			•			•	•	00000	25104.00000	13283.00000
INPUT COST				•																•										-								•									•
ACTIVITY	2063.89343	325, 26272	257.00000	1418.05087	18288.85959	701.17012	13756.07561	349.23492	4579.77948	114.31143	4661.68767	32.40324	2031.49019	32.40324	292.85948	32.40324	1106.61973	311.43114	22505.00000	00000 7066	13756.07561	9307.00000	1648.06481	6375.28533	799.27993	2512.87759	679.56796	1160.95536	6/8.56/96 45/7 56383	4558 95590	5000.00000	9317.25000	1056.39750	1335.61500	1425.58750	1453 17000	870 05250	2560.61250	18747.80504	10974.28470	195.78857	92.39676	240.35383	369.35948	1702.11973	25104.00000	00000 . 58261
AT	BS	B S	EO	BS	BS	BS	88	BS	BS	BS	BS	BS	BS	BS	BS	82	200	2 2	ם נו	3 5	BS	1	BS	BS	BS	BS	BS	BS	200	2 0	ב	BS	BS	BS	8 0	מ מ	S W	ממ	BS	BS	BS	BS	BS	BS	BS	т С	3
. COLUMN.	A5DWN 1R	A6DWN 1R	A7DOM 1R	A7DWN1R	AIREL	A 1ROBW	A2REL	A2ROBW	A3REL	A3ROBW	A4REL	A4ROBW	ASREL	ASROBW	AGREL	AGROBW	A/REL	A / KUBW	A03-0	AISTO	AZROAW	A2STO	ABROAW	A3STO	A4ROAW	A4STO	ASRDAW	A5510	AGKUAW	AZBOAW	A7ST0	AOACRD	AIRETA	AZRETA	ABREIA	A47074	ASBETA	AZRETA	AIREMA	AZREMA	A3REMA	A4REMA	ASREMA	AGREMA	•	- (AZKUA
NUMBER	1696	1697	1699	1700	1701	1702	1103	1104	1705	1106	1707	1708	1709	1710	1711	1712	1713	- •	4746	A 1717	_	1719	1720	1721	1722	1723	1724	1725	1726	1728	1729	1730	1731	1732	1733	17.04	1736	1737	1738	1739	1740	1741	1742	1743	-	A 1745	

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PAGE	REDUCED COST.	13.99810 13.	
]]	UPPER LIMIT.	1831 . 00000 888 . 00000 755 . 00000 775 . 00000 779 . 00000 388 . 00000 388 . 00000 36 . 00000 379 . 00000 370 . 000000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 000000 370 . 00000 370 . 00000 370 . 00000 370 . 00000 370 . 000000 370 . 000000 370 . 000000 370 . 000000 370 . 0000000000000000000000000000000000	
]]	LOWER LIMIT.	1831.00000 888.00000 755.00000 779.00000 36.00000 36.00000 36.00000 36.00000 36.00000 36.00000 36.0000000000	
	INPUT COST	800.00000 10.00000 10.00000 10.00000 10.00000 10.00000 10.00000 10.00000 10.00000 10.00000 10.00000 10.00000	
 	ACTIVITY	1831.00000 888.00000 755.00000 755.00000 755.00000 306.00000 316.000000000000000000000000000000000000	
MAINMOD	AT		
MA	. COLUMN.	A3ROA A4ROA A4ROA A4ROA A4ROB A2ROB A3ROB A3ROB A3ROB A4ROB A3ROB A4ROB A5ROB A5ROB A5ROB A5ROB C04ACR1R C04ACR1R C05ACR1R	
	NUMBER	1747 1748 1750 1751 1752 1753 1755 1755 1755 1755 1755 1755 1755	

REDUCED COST.					•				•	•			•			•		•		•		•			•	•				•	•			٠		•			•		•	•	•			
UPPER LIMIT.	191.00000	NONE	206.00000	191.00000		191.00000	2 13 00000	NONE	191.00000		1192.00000	NONE	NON	NONE	NONE	NONE	NONE	NONE	NONE	NONE	NONE	NONE	NON		4255 00000		9656.00000	NONE	9656.00000	NONE	9656.00000	NONE 9656 DOOOD	NONE	9656.00000		9656.00000	NONE	NONE	NON			HINDIN	NON.	NONE	NONE	
LOWER LIMIT.	191.00000		206.00000	191,00000		191.00000	213,00000		191.00000		1192.00000		•												4255 00000													4255.00000					•	•	· · .	
INPUT COST						•				•			•	•			•						•		•				•						•	-							•	•		
ACTIVITY	191.00000	699 . 63636	206.00000	191.00000	688.72727	191.00000	213 00000	622.34091	191.00000	461.25000	1192.00000	2092.83333	•	362,64362	2602 . 16022	362.64362	495.94959	502.40864	186.31863	452.22390	170.11701	291.13299	170.11701	1259.77307	4255 00000	2174 61746	5348.11207	1294.32943	6279.79788	164.71647	6081.87073	80.10801 5659 57010	68.40684	5275.75305	68.40684	5053.02690	461.74617	4255.00000	0000.000	248.39235	262.13390	227.101.73	250.15390			
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. COLUMN.	C1DOM1R	CIDWNIR	C2DOM1R	CSDOM1R	C3DWN 1R	C4DOM1R	CSDOM1R	CSDWN1R	CEDOM1R	C6DWN1R	C7DOM 1R	CTDWN1R	CIREL	CORFL	CZROBW	COREL	C3R0BW	C4REL	C4ROBW	CSREL	CSROBW	CGREL	CGROBW	CIREL	CONTO	C 1804	C15T0	C2ROAW	C2ST0	C3ROAW	C3STO	CARDAW	CSROAW	CSSTO	CGROAW	CESTO	CZROAW	C7STO	COACRD	CIREIA	CZKEIA	C G K F I A	מביים מיי	- 1	CTRETA	
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LOWER LIMIT.	٠			•				2416.00000	1438.00000	183,00000	89.00000	76.00000	76.00000	513.00000	5302.00000	2891.00000	551.00000	207.00000	189.00000	189.00000	1536.00000
INPUT COST			-	•	•	•	-	•	•	•				-	-	-					٠
ACTIVITY	6225.46903	2843,71635	393.21635	179.58137	296.73299	296.73299	1655.07307	2416.00000	1438.00000	183.00000	89.00000	76.00000	76.00000	513.00000	5302.00000	2891.00000	551.00000	207.00000	189.00000	189.00000	1536.00000
AT	BS	BS	BS	BS	BS	BS	BS	EQ	EQ	EO	EQ	EO	EO	EO	EQ	9	ЕО	EO	EO	EQ.	EO
. COLUMN.	CIREMA	C2REMA	C3REMA	C4REMA	CSREMA	CGREMA	C7REMA	C1ROA	C2ROA	C3R0A	C4R0A	CSROA	CGROA	C7RDA	C1ROB	C2ROB	C3R0B	C4R0B	CSROB	CGROB	C7R0B
VUMBER	1849	1850	1851	1852	1853	1854	1855	1856	1857	1858	1859	1860	1861	1862	1863	1864	1865	1866	1867	1868	1869

EXIT - TIME = 0.16

Execution phase succeeded.

VSS CS: UNLOAD
Enter command:
Leaving the MPSX DRIVER.

Execution terminated 00:34:32 T=11.022 \$10.18
T=11.146 DR=0 \$10.25, \$14.53T

\$sig Logging on -LOG terminated.