EXTENT OF CARBONATION IN BUILDINGS

IN TORONTO

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EXTENT OF CARBONATION IN BUILDINGS IN TORONTO

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Canada Mortgage and Housing Corporation, the Federal Government's housing agency, is responsible for administering the National Housing Act.

This legislation is designed to aid in the improvement of housing and living conditions in Canada. As a result, the Corporation has interests in all aspects of housing and urban growth and development.

Under Part \mathcal{X} of this Act, the Government of Canada provides funds to CMHC to conduct research into the social, economic and technical aspects of housing and related fields, and to undertake the publishing and distribution of the results of this research. CMHC therefore has a statutory responsibility to make widely available, information which may be useful in the improvement of housing and living conditions.

This publication is one of the many items of information published by CMHC with the assistance of federal funds.

Caution

The methodologies, the interpretations and the recommendations are those of the consultants and do not necessarily reflect the view of Canada Mortgage and Housing Corporation, which assisted in the study and its publication. The results of the study are published for information purposes and the reader is cautioned that the scientific data generated are derived from a study of selected buildings in the Toronto area.

Extrapolation of this data to most structures in Canada may be feasible, provided that the vagaries of climates and the behaviour of various structural systems and building materials in those local areas are understood.

Extent of Carbonation in Buildings in Toronto

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Executive Summary

Introduction

Carbonation occurs when concrete reacts with carbon dioxide from air or water and reduces the pH of the concrete to below 9.0. At this low pH reinforcing steel is no longer passive and corrosion of steel may occur. Carbonation is not a problem in very dry concrete or in water-saturated concrete. More massive steel-reinforced concrete structures such as roads, bridges, dams, water and sewage-treatment plants are more likely to fail due to the effects of de-icing salts, freeze-thaw effects and sulphate or alkali-reactivity damage, than to the effects of carbonation.

Carbonation of concrete has attracted considerable and recent interest in Europe and Australia, where it is known to be the cause of current structural problems. As the Canadian building stock is younger than European carbonation troubled structures, it was considered prudent to initially investigate the literature to assess the likely occurrence of future carbonation-induced decay of reinforced concrete in Canada. To this end the Corporation had already funded a literature review and analysis. 1

As a potential problem was subsequently thought to exist in Canada, a research design has been developed and tested in Toronto and an assessment made of the impact of carbonation on concrete structures in that City.

Methodology

Major building owners such as Ontario Housing Corporation, Toronto Metropolitan Housing Authority, Public Works Canada, the Ontario Ministry of Government Services and members of the Canadian Institute of Public Real Estate Companies were interviewed and their willingness to have buildings investigated was determined. Twenty-eight buildings were eventually selected and tested with invasive but non-destructive procedures.

The classifications of building components examined were; cast-in-place balconies; vertical exposed components of cast-in-place structural elements and precast concrete facades.

¹ "Determine Potential of Carbonation of Concrete in Canada", CMHC, 1987.

A total of 348 concrete cores, each approximately $50 \, \text{mm}$ diameter x $75 \, \text{mm}$ long were removed, by diamond drilling, and examined by a series of testing techniques as follows: splitting tensile strength, absorption and voids, chloride ion content, carbonation depth and lime content.

A representative from IRC/NRC was retained to provide guidance in site selection, sampling procedures and assistance in analysis of the data.

Findings

The study shows that, in Toronto at least, a proportion of the building stock will experience carbonation corrosion damage within a desired service life.

Of all the building components tested, only the balconies in two already had carbonation to the depth of the specified cover to the steel reinforcement. However, one third of the balconies examined, had a rate of carbonation penetration higher than is considered desirable relative to design life expectancy.

None of the vertical cast-in-place components or pre-cast cladding units had carbonation to specified cover depths. However two structures in each category had excessively high rates of carbonation penetration relative to design life expectancy.

The lime content tests shows that ongoing carbonation may be to depths greater than those shown by the simple phenolphthalein indicator tests, which only shows when carbonation is complete. This finding will require consideration with regard to future recommendations and testing procedures.

The drilling and testing of cores by the procedures adopted was found to be technically sound and economic. Arranging and gaining access to structures presented some problems.

Conclusions

The splitting tensile and absorption and void data confirm the absence of anomalies in other data. It may not be essential to make these tests in future programmes of field study.

The chloride-ion content determinations eliminated the possibility of any anomalies in the carbonation depth test results. These tests could be made as a second and optional stage, in future field studies if shown to be necessary by the results of carbonation tests.

The lime content tests show that in some cases the carbonation depth as determined by phenolphthalein testing may be underestimated, on occasion, to a significant extent. It would therefore seem prudent to provide for this or other types of testing, in addition to the simple phenolphthalein indicator test, indicating only that the carbonation process is complete.

The study data suggests therefore, that in Toronto at least, a proportion of the building stock will experience carbonation corrosion damage within a desired service life.

Balconies appear to be the most vulnerable component with two in the survey sample with carbonation already having reached the specified reinforcement depth, and one-third of those tested likely to suffer damage within their service life.

With the vertical cast-in-place components and precast cladding, most of those examined would last their service life without damage, but two buildings had these components with carbonation depths that would result in a lower than desirable service life.

The cost of repairing corrosion damage will be very high if carbonation is allowed to proceed until it reaches the reinforcement. Accordingly it would be prudent to provide, to the industry, some guidance on the diagnostic and preventative measures which can be taken.

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ABSTRACT

This report is the second phase of a multi-phase study of the potential for carbonation in Canadian buildings. It describes a survey and testing of 28 buildings in Toronto to determine depths of carbonation. An assessment of this phenomena in relationship to other concrete properties is provided.

Based on the findings it is concluded that while no epidemic of damage is expected, in the Toronto area, some buildings there will experience corrosion damage within their service life with concurrent loss of structural properties.

Extent of Carbonation in Buildings in Toronto

CMHC File: 6710-12-1

Introduction

In 1986/87 a literature review was made, and a synthesis prepared, to "Determine Potential of Carbonation of Concrete in Canada". The study concluded that there was a significant potential for carbonation in some major urban areas of Canada and recommended a further study.

It was decided to concentrate the next phase of the work in one urban area. Toronto was chosen as a suitable and convenient location. Cores were taken from a number of buildings and information developed from the tests carried out on these cores.

Methodology

Through contact with the Federal and Provincial Governments and the Ontario Ministry of Housing, a total of 134 public and residential buildings were identified as candidates for testing. From these, a sample of 32 buildings were chosen and 28 were tested. The locations were in various parts of the Metropolitan Toronto area, see figure 1, and from all compass orientations.

The buildings are listed in Table 1 by a reference number. Table 1 also lists the samples obtained.

Samples were obtained from the top surfaces of exposed balconies, vertical cast-in-place exposed concrete columns and shear walls, and from pre-cast cladding panels.

Field Procedure

All test samples were 50mm diameter cores about 75-100mm long. These were drilled from the outside exposed surfaces remote from possible salt splash using a diamond drill bit cooled with water. Immediately after drilling the cores were marked for identification and then immersed in water until tested to prevent further carbonation. Cores were taken in sets of three and the location and orientation was noted for all samples.

Laboratory Procedures

To prevent any ongoing carbonation all samples remained immersed in water until tested.

Splitting Tensile Test

In order to make tests to determine carbonation depths, the cores were split in half along a major axis. To obtain an approximate strength value the failing load was noted and the splitting tensile strength calculated according to ASTM C496-86. These results are tabulated in Table 2. A total of 348 tests were made.

Carbonation Depth

Immediately after splitting, one half of the core was reimmersed in water to prevent any ongoing carbonation and the other half tested to determine the depth of carbonation. The test procedure used was based on that given in ISO Standard DOC N77E except that with the relatively small size specimen used four depth measurements were made and averaged. These carbonation depth results are summarized in Table 3. A total of 348 tests were made.

Absorption and Voids

In order to provide further data on concrete quality, absorption and voids were determined according to the procedure given in ASTM 642-82, however the specimens were inevitably smaller than the size specified in this standard. These results are summarized in Table 4. A total of 339 tests were made.

Chloride Ion Content

A number of horizons, with selected samples, were tested by the Ministry of Transportation of Ontario method to determine the water soluble chloride-ion content of the samples. The results are summarized in Table 5. A total of 279 tests were made.

Lime Content Tests

The lime content of 30 selected samples was determined by the National Research Council of Canada using a thermo gravimetric procedure and a copy of their report is attached as Appendix A.

Evaluation of Data: Part 1 Physical Tests

Tensile Strength

To ensure that any potential surface disfigurement was kept to a minimum, cores were made deliberately small and did not meet the dimensional requirements of ASTM-C496. The strengths obtained therefore may be approximate but are examined later with other data to see if any correlation or trends can be deduced relative to the carbonation data.

This tensile strength data has been summarized in Table 7 and the results of a statistical analysis of the data are given in Table 10.

It is seen that the variation in strength, from building to building, is quite high. Although in every case the average strengths of the public buildings are higher, the difference is marginal and not significant. This data did not make a significant contribution to the findings.

Absorption and Voids

The test specimens were much smaller than the minimum size specified in ASTM-C642 but again are used to see if any correlation or trends can be deduced relative to the carbonation data.

The data has been summarized in Table 8 and the results of a statistical analysis are given in Table 11.

Again it is seen that the results are highly variable and no clear difference between the quality of concrete in public and residential buildings is seen. This data also did not make a significant contribution to the findings.

Orientation

Orientation is not thought to be a factor in the depth of carbonation found. The orientation of each building is summarized in Table 12.

Carbonation & Carbonation Depth

In previous work¹ it has been found that the formula postulated by Pihlajavaara appeared to be suitable for concrete in Canada. This formula takes into account concrete quality and the environment and a tabulation of it is given below:

Estimation of carbonation depth in Portland cement concrete. Carbonation depth = $10B\sqrt{t}$. Values of constant B.

Quality of Concrete and Storage Condition

Carbonation	Low stre	ength	Middle	strength	High strength		
time							
(years)	Outdoors	Indoors	Outdoors	Indoors	Outdoors Indoors		
	(moist)		(moist)		(moist)		
			Thickne	ess of Car	bonation La	yer	
	B=0.6	B=1.0	B=0.2	B=.05	B=0.1	B=0.2	
1	6	10	2	5	1	2	
2	9	14	3	7	1.5	3	
5	13	22	4	11	2	4	
10	19	32	6	16	3	6	
25	30	50	10	25	5	10	

The constant B has been calculated for each component of each building and the results are summarized in Table 9 and their distribution is shown in Figure 2.

Results

If a value for B of 0.25 is taken as the upper limit for a quality of concrete reasonably resistant to carbonation in Canada most of the balcony and pre-cast concrete examined in this pilot study falls within this definition, while most of the concrete cast in columns and shear walls does not.

The data has been analysed statistically and the results summarized in Table 13.

This agreement shows clearly that the pre-cast concrete is, as would be expected, superior to the cast-in-place concrete from the point of view of carbonation. The balcony concrete is more susceptible to carbonation and the cast-in-place vertical components the least susceptible. It is this last category which property owners shall consider for further testing as it is a very critical structural component.

It is noteworthy that there is no relationship between the age of any of the structures and carbonation depths. These relationships have been plotted on Figures 13 to 15. Clearly carbonation depth must relate more to concrete quality and possibly its constituents, than to age.

The cover requirements required by the Building Codes for different building components are different and have changed over the years.

For the buildings in this sample most balconies would have had, according to code, 3/4" (19mm) cover and the vertical cast-in-place components 1-1/2" (38mm) or 2" (51mm) cover. For pre-cast facade panels the minimum cover specified would probably have been 3/4" (19mm).

By comparing averages and selecting individual results which emphasize poor resistance to carbonation the following rather random picture emerges from the Toronto area pilot study.

For balconies the average value of B determined and a cover of 19mm would produce a time to corrosion of the reinforcing steel of 82 years. However, in Building 62 and Building 88 corrosion depths in those balconies have already been reached. 6 out of the 19 balconies examined or almost 1/3rd of the buildings sampled would experience corrosion within 50 years.

For vertical cast-in-place concretes 38mm and 51mm carbonation depths would be reached in 50 years for buildings having a B value of 0.53 and 0.72 respectively. Only 2 buildings fall within this category.

For pre-cast facade panels only 2 of the buildings will experience corrosion within 50 years.

A summary of average carbonation depths for each building is given in Table 6.

Evaluation of Data: Part 2 Chemical Tests

Chloride Ion Content

The quantities of chloride-ion found are, in all cases, a small fraction of the amount that would be present if calcium chloride had been added to the freshly mixed concrete, as a component of a code-approved agent. It is therefore concluded that it was not more likely present. Average chloride contents found in all the cores examined are summarized in Table 14.

Where there was a gradient in chloride ion content from the exposed face of the concrete to the interior the differences were typical of a normal distribution of variation throughout a concrete building component. In some cases there may have been some minor leaching at the exposed face and in others some exterior deposition of chlorides but neither phenomenon, if they occurred, was of a magnitude to make any significant impact on the study data.

It is concluded therefore that the chlorides found are all or virtually all background chlorides present in the materials from which the concrete was made. They will therefore have had little if any effect on the carbonation results.

Lime Content

The study test data is given in Appendix A. A significant reduction in the original H_2O and $Ca(OH)_2$ contents indicates that carbonation is taking place.

Litvan & Meyer in their paper "Carbonation of Granulated Blast Furnace Slag Cement Concrete During Twenty Years of Field Exposure" remind that phenolphthalein is an acid-base indicator, and that the colour changes resulting from that test signifies not so much the presence of carbonate as the absence of lime. Accordingly the resulting red colour of the partial area must not be interpreted as evidence of no carbonation. Accordingly the thermo gravimetric approach used in the lime content tests improves the sensitivity of analysis suggesting ongoing carbonation to depths beyond that detected by the phenolphthalein indicator.

For the purpose of this evaluation the percentage of original Ca(OH)₂ at the three horizons tested by IRC/NRC, has been plotted in Figures 2 to 11 together with the carbonation depth as found by the phenolphthalein indicator test.

In Figure 4 it is seen that the carbonation front as determined by phenolphthalein is close to the horizon at which virtually all the original Ca(OH), was present. results might be adduced to the data in Figure 3. In all other tests however it will be seen that a significant reduction of Ca(OH), occurs deeper than the carbonation depth determined by phenolphthalein. This difference may be as little as 10 mm in Figure 8 but could be significantly greater depth of carbonation is in Figure 9. Here the significantly under estimated by the phenolphthalein tests.

This data was further examined by comparing it with other data from these samples. This data is summarized in Table 15 and the relationship between carbonation depth is compared to both absorption and splitting tensile strengths on Figures 16 and 17. No consistent relationship is shown between absorption and carbonation depth. There may be a rough correlation with splitting tensile strength similar to the carbonation depth compressive strength relationship found by Parrott.²

² L.J. Parrott, "A Review of Carbonation in Reinforced Concrete", C and CA, July 1987.

The data base is too small to make any conclusions with regard to carbonation depth and building orientation.

Conclusions

Methodology

The drilling and testing of cores by the procedures adopted was found to be technically sound, and economic. Arranging and gaining access to structures does on occasion present problems.

The splitting tensile and absorption and void data was interesting in that it helped confirm a lack of anomalies in the other data. It is not considered that it would be essential to make these tests in future programmes.

The chloride-ion content determinations also eliminated the possibility of any anomalies in the carbonation depth test results. In future these tests could be made as a second and optional stage, if shown to be necessary by the results of carbonation tests.

The lime content tests show that in some cases the carbonation depth as determined by phenolphthalein may be under estimated, on occasion to a significant extent. It would therefore seem prudent to provide for this or other types of testing in addition to the simple indicator test.

Test Results

Of all the building components tested, only the balconies in two of them had carbonation to the depth of the specified cover to reinforcement. On the other hand one third of all the balconies examined had B constants higher than is considered desirable, an indicator suggesting a lower than desirable service life.

None of the vertical cast-in-place components or pre-cast cladding units had carbonation to specified cover depths and in only two structures, in each category, were there excessively high B constants.

The lime content tests show however, that ongoing carbonation depths may be greater than those shown by simple indicator tests. This may mean that the actual time to corrosion may be less than that predicted from the results of indicator tests.

The study data suggests therefore, that in Toronto at least a proportion of the building stock will experience carbonation corrosion damage within a desired service life. Balconies appear to be the most vulnerable component with two in the survey sample with carbonation already having reached the specified reinforcement depth, and one-third of those tested likely to suffer damage within their service life. With the vertical cast-in-place components and precast cladding most likely would last their service life without corrosion induced structural damage, but two buildings had these components with carbonation depths which would result in a lower than desirable service life.

If carbonation is allowed to proceed until it reaches the reinforcement in those buildings examined, the cost of repair of the resulting corrosion damage would be high. It would therefore be prudent to provide, to the industry, some guidance on the diagnostic and preventative measures which could be undertaken.

Table 1
Building Sample

No			Balconies	Samples Tal Vertical Cast-in-Place Components	cen Pre-Cast Panels
<u>Publ</u>	<u>ic Buildings</u>			-	
4			_	3	3
6			9	6	_
. 7	•		9	9	-
8			_	. 3	3
11			_	=	9
128			_	-	_
130				9	9
131			_	3	9 3
132	•		_	. 9	9
133			_	9	g
134	•		_	=	9 9 3
201				•	•
Resi	<u>dential Buildi</u>	ngs			
28			9	9	_
29			9	3	-
38			9	_	_
44	•		9	9	-
61			6	6	_
62			6	6	· <u>-</u>
66			3	3	_
67		•	3	3	- .
69			3	3	- .
75			9	9	-
78		*	-	-	
79			9	9	_
81			6	_	-
86			_	_	18
88			9	9	_
91			9	– .	-
92				9	_
94			9 9	9 9	_
98			_	_	- ,
100	•		9	-	-
105			- .	-	_
				,	
		TOTALS	144	138	66 = 348

Table 2
Splitting Tensile Strengths

Building	Component	Splitting	Tongile.	Most /	MDa \
No.	Component	1	2	3	Average
1.0.		_	2	J	Average
4	V	4.90	4.31	4.95	4.72
	PC	5.85	5.12	5.70	5.56
6	В	5.54	7.09	6.11	6.25
	В	5.42	5.36	6.10	5.63
	B	5.70	5.54	5.58	5.61
	V	2.56	3.85	1.61	2.67
	V	2.25	2.64	3.21	2.70
7	${f B}$	4.31	4.95	5.18	4.81
	В	3.27	4.05	3.86	3.73
	В	4.29	3.37	4.00	3.89
	V	3.99	4.68	3.44	4.04
	V	4.88	3.24	3.73	3.95
•	V	4.32	3.31	5.03	4.22
8	V	3.43	2.96	2.08	2.82
	PC	5.49	4.90	3.31	4.57
11	PC	4.25	3.77	3.96	3.99
	PC	4.60	4.09	3.22	3.97
	PC ·	3.91	4.01	3.68	3.87
130	V	2.84	2.91	2.64	2.80
	V	3.18	2.94	2.63	2.92
•	V.	2.09	3.07	2.75	2.64
9	PC	4.49	2.76	3.53	3.59
. *	PC	3.33	2.59	3.26	3.06
	PC	3.00	2.93	5.27	3.73
131	V	4.35	5.87	5.62	5.28
	PC	4.68	4.08	3.50	4.09
132	V	3.82	3.95	3.64	3.80
	V	3.37	3.83	1.88	3.03
	V	3.21	3.52	3.08	3.27
	PC	3.84	5.14	4.21	4.46
	PC	3.76	4.50	4.08	4.11
•	PC	3.74	3.92	4.10	3.92
133	V	3.22	3.09	4.18	3.50
	V	3.99	3.97	4.33	4.10
	V	3.34	4.12	3.97	3.81
	PC	3.77	5.56	5.38	4.90
	PC	4.81	4.87	3.96	4.55
	PC	5.45	4.54	5.24	5.08
134	PC	4.36	3.99	5.43	4.59
28	В	2.91	3.13	3.67	3.24
·	В	2.99	3.12	3.21	3.11
	<u>B</u>	6.02	3.30	4.13	4.48
	<u>V</u>	3.24	3.97	3.25	3.49
	V	2.42	3.17	3.79	3.13
	V	3.34	4.81	3.25	3.80
•					

Building No.	Component	Splitting 1	Tensile 2	Test (MPa) Average
		•		•	•
29	В	3.37	3.67	3.70	3.58
	В	2.94	2.52	2.90	2.79
	В	2.89	3.01	4.23	3.38
	V	2.41	3.62	2.43	2.82
38	B.	4.07	4.09	3.16	3.77°
	B	3.24	3.12	2.84	3.07
	В	2.50	3.23	3.92	3.22
44	В	3.83	3.00	2.21	3.01
	• В	2.36	2.80	1.94	2.37
	В	2.21	3.81	2.81	2.94
	V	4.23	2.61	1.94	2.93
	V	3.34	2.55	2.34	2.74
	V	2.44	2.93	3.43	2.93
61	В	3.12	4.55	3.98	3.88
	В	3.60	3.25	3.11	3.32
	V	2.94	3.34	2.92	3.07
	V	2.96	3.80	3.30	3.05
62	В	4.43	4.52	4.62	4.52
	В	4.30	4.30	5.73	4.78
	V	3.06	3.64	3.41	3.37
	V	3.70	3.73	3.72	3.72
66	В	3.49	3.84	2.90	3.41
	V	3.72	2.53	2.48	2.91
67	В	3.02	2.91	2.23	2.72
	Λ.	2.55	2.05	3.16	2.59
69	В	4.22	2.27	2.76	3.08
	Λ	3. 65	3.63	6.57	4.62
75	В	3.91	4.53	3.58	4.01
	В	3.23	3.16	3.04	3.14
•	В	3.70	3.13	3.76	3.53
	V	2.66	2.29	2.66	2.54
	V	2.80	3.20	3.10	3.03
	\mathbf{V}	3.17	2.52	2.23	2.64
79	В	5.46	3.57	5.82	4.95
•	В	3.27	3.80	3.61	3.56
,	В	3.12	1.91	1.99	2.34
	V	2.38	3.31	2.27	2.65
	V	2.90	1.97	2.62	2.50
	V	2.17	2.62	2.12	2.30
81	В	4.30	3.11	2.87	3.43
	B	3.71	4.32	3.52	3.85
86	PC	3.80	3.42	3.85	3.69
	PC	2.44	3.75	3.96	3.38
	PC	2.28	2.64	2.14	2.35
	PC	4.58	4.88	3.90	4.45
•	PC	1.95	3.49	3.23	2.89
	PC	3.68	3.54	3.42	3.55

Building No.	Component	Splitting 1	Tensile 2	Test (M	Pa) Average
88	В	2.59	1.28	1.22	1.70
	В	2.25	2.45	2.52	2.41
	В	1.75	3.03	2.12	2.30
	V	2.64	2.74	2.90	2.76
	V	2.93	2.67	2.79	2.80
•	V	2.41	2.26	2.31	2.33
91	B	3.38	3.61	2.56	3.18
7	В	2.97	4.20	2.74	3.30
•	В	3.46	3.57	2.88	3.30
92	В	1.74	1.91	2.49	2.05
92	В	2.41	2.74	3.25	2.80
	В				
	A	2.35	2.46	2.95	2.59
		3.23	2.87	2.30	2.80
	V	2.77	3.11	3.00	2.96
0.4	V	2.82	2.69	2.65	2.72
94	В	2.39	2.69	1.63	2.24
	В	2.64	2.53	1.44	2.20
	В	2.88	3.18	3.43	3.16
	Λ.	3.39	3.26	2.33	2.99
	V	2.73	2.12	3.24	2.70
		3.46	2.92	3.10	3.16
100	· B	2.73	2.27	2.37	2.46
	В	3.61	3.33	2.81	3.25
	В	3.04	3.25	3.11	3.13

Table 3
Carbonation Depth

Building No.	Component	Orientation	Carbo	onatio	on De	epth (mm)
			1	2	3	Average
4	V	N	7	11	8	9
_	PC	N	N	N	N	N
6	В	N	4	5	3	4
	В	N	2	N	N	1
	В	S	3	1	3	2
	V	N	12	16	25	18
_	<u>v</u>	S	11	20	12	14
7 .	B .	N	5	N	1	2
	В	N	2	N	1	1
	B	W	2	1	5	3
	V	N	16	14	19	16
	V	N	14	21	12	16
•	V	<u>₩</u>	13	16	13	14
8	V	E	10	8	10	9
	PC	E	6	10	2	6
11	PC	W	5	5	4	5
	PC	W	6	2	5	4
120	PC	S	N	6	4	3
130	V	S	12	15	21	16
	V	S	13	13	15	14
	V	S	17	15	10	14
	PC	W	21	23	20	21
	PC	W	20	18	22	20
	PC V	W	21	20	20	20
131		W	4	45*	2	3
122	PC	W	2	2	1	2
132	V V	E E	21 23	26 21	19 24	22 23
	V	E	23 24	26	24	25
	PC	S	3	7	5	5
	PC	S	3 7	1	3	.4
	PC	S	í	4	4	3
133	V	S	Ŋ	5	7	4
133	v	S	3	10	2	5
	v	S S	1	7	10	5 6
	PC	_	N	Ň	N	N
	PC		N	N	N	N
	PC		N	N	N	N
134	PC	W	1	2	1	1
N - Nogli						21/072/05

N = Negligible

^{*} Excluded from the averages

Building No.	Component	Orientation	Car	Carbonation Depth (mm)			
			1	2	3	Average	
28	В	S S	21	21	22	21	
	B B	S E	18 6	18 7	17	18	
	V D	S	10	15	5 12	6 12	
	V	S	15	19	11	15	
	v	E	17	13	18	16	
29	В	W	8	6	9	8	
	В	${f E}$	2	2	2	2	
	В	E	3	3	2	3	
	V	S	23	22	20	22	
38	В	W	4	5	4	4	
	В	W	7	5	5	6	
44	B B	W S	1 . 5	10	3 8	· 5	
44	В	N S	N	4 5	D D	3	
	В	N	N	N	N	Ŋ	
	v	S	17	17	11	15	
	V	N	9	13	11	11	
	V	N	15	11	11	12	
61	В	S	20	25	25	23	
	В	N	6	13	10	10	
	V	S	27	15	15	19	
	<u>v</u> .	N	21	24	21	22	
62	В	E	25	24	22	24	
	B V	W THE	31 25	30	31	31	
	v V	E W	25 8	25 9	25 5	25 7	
66	v B	E	5	3	8	, 5	
	v	S	8	7	10	8	
67	В	S	7	7	9	8	
	V	S S	5	3	9	5	
69	В		4	3	5	4	
	V B	S E W	7	3 8 3	3	4 5 2	
75	В	W	N		2 5	2	
	В	W E	4	3	5	4	
	B V	W W	5 37	4	4	4	
	V V	W W	32	37 31	37 31	37 31	
	Ÿ	E	25	21	26	26	
79	В	W	6	10	6	7	
	В	W	7	10	. 7	7 8	
	B B	E	N	10	1	4	
	V	W	13	13	16	14	
	V	W	16	15	14	15	
	V	E	16	15	15	15	
81	В	N	2 3	2	4	3	
	В	N	3	4	4	4	

Building Component		Orientation	Car	bonation	De	pth (mm)
NO.			1	2	3	Average
86	PC(B)	N	2	3	N	2
	PC(B)	S	1	1	1	1
	PC(B)	N	3	3	7	4
,	PC	N	11	6	12	10
	PC	S	13	12	15	13
	PC	N	11	8	10	10
88	В	W	21	D :	21	21
	В	E	17	19 :	17	18
	В	E	20	16	15	17
•	V	W	14	14	14	14
	V	E	12	12	10	11
	V	E	11	15	12	13
91	В	N	9	7	11	9
	В	S	9	8	11	9
	В	S	5	14	7	9
92	В	W	11	18	16	15
	В	W	14	12	14	13
	В	E	10	N :	10	7
	V	W	26	28	28	27
	V	W	25	30	28	30
	V	E	29	33	28	30
94	В	S	23	16	16	18
	В	S	10	14	18	14
	В	N	20	19	20	20
	V	S	13	15	18	15
	V	S	15	18	18	17
	V	N	6	6	7	6
100	В	W	7	N	4	4
	В	E	N	N	N	N
	В	${f E}$	N	N	N	N

Table 4

Absorption and Voids

Building No.	Component	1	Abso 2	_	n (%) erage	Perm 1	meable 2	Voids 3 Ave	(%) rage
2.00			-	3 111	crage	-	2	J AVC	rage
4	V	4.6	4.7	4.1	4.5	11.8	11.7	11.0	11.5
	PC	2.6	2.6	2.7	2.6	6.8	6.9	8.0	7.2
6	В	5.2	5.2	5.8	5.4	_	12.0	13.3	12.7
	В	6.1	5.8	4.8	5.6	13.9	14.5	11.5	13.3
	В	6.2	5.3	5.5	5.7	14.1	11.0	12.6	12.6
	v	3.6	2.9	4.1	3.5	7.9	6.1	8.8	7.6
	V	3.6	3.3	3.5	3.5	7.2	7.1	7.5	7 . 3
7	В	4.5	4.5	4.6	4.5	11.6	11.3	11.4	11.4
-	B	4.3	4.4	4.6	4.4	11.1	11.2	11.5	11.3
	В	5.5	5.2	4.8	5.2	13.3	13.3	11.9	12.8
	V	4.3	4.3	4.6	4.4	11.0	11.3	11.0	11.1
	V	4.6	4.4	4.9	4.6	11.4	11.0	12.2	11.5
	v	5.2	4.8	5.1	5.0	12.5	12.3	13.2	12.7
8	v	5.1	6.0	5.9	5.7	14.5	14.4	15.6	14.8
O	PC	5.6	5.0	5.9	5.5	14.2	13.0	14.3	13.8
11	PC	4.7	5.0	5.6	5.1	11.3	12.9	13.5	12.6
<u> </u>	PC	4.9	4.8	4.8	4.8	12.5	12.6	12.3	12.5
	PC	4.7	4.9	4.7	4.8	11.5	11.7	12.3	11.8
130	V	6.8	6.2	5.3	6.1	16.8	14.6	13.8	15.1
130	v	5.6	5.3		5.5	14.0	12.7	15.4	14.0
	V.	5.4	6.1	5.7			15.6	15.4	14.8
	PC	4.9	5.0	6.5	6.0 4.9	13.7 11.9	13.8	11.9	
	PC			4.8			11.8		12.5
•	PC	5.1 6.1	5.3 4.9	5.3	5.2	13.7	12.1	13.2	12 . 9 12 . 7
131	V	6.0	6.5	5.0	5.3 6.3	14.2 15.4	16.1	11.7 16.2	15.9
TOT	PC			6.4	4.0	11.4	11.3	9.1	10.6
132	V	4.3	4.4	3.4			12.3		12.7
132	v V	4.9	4.9	5.3	5.0	12.7 13.8	13.5	13.1 13.9	13.8
	v V	5.5 5.6	5.3	5.3	5.4		13.5		12.9
	PC		4.9	4.4	5.0	14.1		11.6	
	PC	5.9 3.9	4.6	4.6	5.0	14.4	11.5 11.6	11.3	12.4
•	PC	5.8	4.7 6.3	4.7 4.7	4.4	9.6 13.8	14.9	11.4	10 . 9 12 . 2
133	V	4.7	5.1	5.0	5.6 4.9	13.5	11.9	11.5 12.2	12.5
133	V	4.9	4.8	4.2	4.6	11.8	11.5	10.5	11.3
	V	4.7	3.7	4.7	4.4	11.2	11.4	11.8	11.5
	PC	3.4	4.1	4.0	3.8	8.5	10.2	10.5	9.7
	PC	3.7	4.0	3.6	3.8	9.5	10.2	9.5	9.8
	PC	3.9	4.0	4.0	4.0	10.1	10.4	10.2	10.2
134						13.3			12.5
28	PC B	4.7 4.8	4.5 4.8	4.6 5.6	4.6 5.1	12.0	11.0 12.3	13.2 14.0	12.8
20	B	5.3	5.1		5.2	12.9	12.3	13.2	13.0
	В	4.6	4.5	5.1 5.4	4.8	11.9	11.9	12.6	12.1
	V	5.7	5.6	5.4 6.1	5.8	13.5	13.6	15.0	14.0
	v V			6.0	5.9	13.3	14.1	14.4	13.9
•		5.8	6.0				12.8	9.5	11.2
	V	4.5	5.1	3.8	4.5	11.3	12.0	9.0	11.0

Building No.	Component	1	Absor 2	_	n (%) Average	Permea 1	able Vo 2	ids(%) 3 Ave	rage
4	,								•
29	В	5.3	5.1	4.8	5.1	9.1	12.5	11.7	11.1
	В	4.8	4.5	4.8	4.7	12.8	11.6	12.4	12.3
	В	4.7	4.9	4.9	4.8	12.0	12.3	12.1	12.1
	V	5.1	5.0	5.4	5.2	12.8	12.4	12.8	12.7
38	В	5.3	4.6	4.6	4.8	12.7	11.4	11.2	11.8
	В	_	4.9	4.7	4.8	-	11.9	11.5	11.7
44	В	4.9	4.5	4.6	4.7	11.7	10.8	11.2	11.2
44	B B	4.5 5.0	4.4 4.0	4.6 4.8	4.5	11.6 12.1	10.8 11.7	10.8 11.9	11.1
	В	4.4	4.5	4.3	4.6 4.4	10.7	10.9	11.9	11.9 10.9
	·A	4.9	5.6	5.2	5.2	11.7	13.3	12.6	12.5
		4.2	4.3	4.4	4.3	10.6	10.7	11.6	11.0
	v	4.5	4.8	5.0	4.8	11.3	11.6	12.6	11.9
61	В	5.4	3.4	5.3	4.7	15.4	11.0	15.7	14.0
	B	5.7	5.5	5.7	5.6	14.8	13.7	13.9	14.1
	V	6.2	5.4	6.1	5.9	15.7	14.0	15.8	15.2
	V	5.5	5.7	5.5	5.6	13.9	14.2	14.0	14.0
62	В	5.8	5.1	4.9	5.3	17.3	17.3	16.4	17.0
	В	4.8	4.6	4.9	4.8	17.0	15.0	16.0	16.0
	V	6.1	5.4	7.2	6.2	15.1	14.0	16.5	15.2
	V	5.2	5.1	5.1	5.1	12.9	12.6	12.3	12.6
66	В	5.2	5.9	6.1	5.7	13.0	13.6	14.1	13.6
	Ā	2.7	2.4	2.4	2.5	6.5	5.7	6.2	6.1
. 67	В	5.2	5.1	5.7	5.3	12.4	. 12.0	13.9	12.8
60		2.5	2.1	2.3	2.3	6.2	5.1	5.9	5.7
69	B	2.8	1 2	2.7	2.8	7.0	2.0	7.1	7.1
75	V B	1.5 4.8	1.2 5.4	1.1 4.7	1.3 5.0	3.6 12.1	3.0 13.4	2.6 12.4	3.1 12.6
75	В	5.8	5.3	5.6	5.6	13.8	13.4	13.9	13.6
	В	6.0	6.0	5.1	5.7	15.1	14.5	13.2	14.3
	v	5.7	5.8	6.3	5.9	15.0	15.2	16.1	15.4
	Ā	6.5	5.8	6.0	6.1	15.7	14.1	14.6	14.8
	V	5.9	5.4	5.8	5.7	14.9	13.5	14.6	14.3
79 .	В	5.1	4.8	5.2	5.0	12.4	11.2	12.2	11.6
	В	4.4	5.0	4.5		11.3	12.4	11.5	11.7
	В	4.7	6.3	4.9	5.3	11.2	15.3	12.3	12.9
	V	1.6	2.8	1.7		4.7	9.3	4.5	6.2
	V	2.8	3.0	2.7		7.1	8.0	7.1	7.4
0.5	Ā	2.2	2.8	_	2.5	6.0	7.1	-	6.6
81	В	5.6	5.8	5.8	5.7	14.0	14.1	14.0	14.0
86	В	5.6	5.6	5.6		13.4 16.0	13.7	14.8 13.4	14.0
00	PC PC	5.6 4.9	5.6 5.6	5.7 5.4		12.1	13.3 13.6	13.4	14.2 13.0
	PC PC	6.9	5.6 6.6	6.5		18.4	15.4	15.5	16.4
	PC	4.0	4.3	5.8	4.7	9.9	13.9	14.6	12.8
	PC		4.8	5.6		12.7	11.5	13.2	12.5
	PC	5.5	5.8	5.8	5.7	13.4	13.2	13.6	13.4
						_			

Building	Component			Permeable Voids (%)				(%)		
No.		1	2	3	Average			2		rage
	•				-					
				•						
88	В	4.2	· _	5.8	5.0		17.4		15.7	16.6
	В	5.2	5.3	5.3			13.3	14.2	13.4	13.6
	В	5.4	5.4	3.1			14.3	13.7	8.5	12.2
	V	5.7	5.8	3.7			13.9	14.3	9.2	12.5
	V	5.9	5.5	6.2			14.9	17.5	15.8	16.1
	V	6.2	5.8	6.1	L 6.0		15.0	14.3	14.8	14.7
91	В	7.0	6.8	6.2	6.7		17.3	16.1	15.1	16.2
	В	5.5	5.7	5.3	L 5.4		14.2	14.8	13.1	14.0
	В	6.1	5.4	5.4	5.6		15.1	13.2	14.2	14.2
92	В	6.8	6.7	6.6	6.7		15.8	16.7	16.7	16.4
	В	6.4	5.8	6.5	6.2		15.6	14.2	16.0	15.3
	В	5.9	7.4	6.0	6.4		14.9	16.6	15.1	15.5
•	V	7.5	6.5	5.8	6.6		17.8	15.7	14.1	15.9
	V	6.0	5.8	6.2	6.0		13.3	14.5		14.2
	V	6.3	6.7	6.6			15.6		16.0	16.0
94	В	5.2	5.2	5.7		•	12.8	12.9	13.6	13.1
	В	4.9	5.4	5.4			12.2	13.3	13.7	13.1
•	В	4.7	5.6	6.0			12.6	14.1	13.9	13.5
	V	5.5	5.4	5.8			13.5	13.5	13.7	13.6
	V .	5.8	5.5	5.9			14.5	13.2	14.1	14. 0
	V	5.6	5.0	6.5	and the second s		13.7	12.4	15.4	13.8
100	В	6.7	5.6	1.8			16.0	14.1	8.8	13.0
	В	4.9	4.6	4.6			11.4	11.3	11.4	11.4
•	В	5.0	5.6	4.8	3 5 . 1		12.2	13.9	11.5	12.5

Table 5
Chloride Contents

BUILDING	BUILDING COMPONENT	CORE	HORIZON (M	M FROM OUTSI	DE FACE)
			0 - 10	30 - 40	60 - 70
6	V	1.	0.016	0.012	0.014
		2 3	0.016	0.015	0.011
		3	0.012	0.017	0.012
7	V	10	0.013	0.012	0.044
		11	0.010	0.017	0.025
		12	0.022	0.023	0.020
28	В	4	0.021	0.017	0.026
		5	0.026	0.035	0.027
		6	0.020	0.020	0.029
29	В	7	0.031	0.017	0.016
		. 8	0.037	0.016	0.021
		9	0.028	0.020	0.016
	V	10	0.019	0.031	0.052
-		11	0.014	0.023	0.010
		12	0.007	0.017	0.011
38	В	1	0.025	0.016	0.007
	•	2	0.018	0.007	0.013
		3	0.008	0.006	0.023
44	В	7 8	0.025	0.008	0.005
		8	0.011	0.012	0.012
	• ·	9	0.007	0.006	0.023
61	В	1	0.011	0.022	0.020
		1 2 3	0.011	0.035	0.045
		3	0.008	0.034	0.043
	V	10	0.010	0.019	0.028
•	•	11	0.021	0.022	0.022
		12	0.019	0.025	0.029
62	V	4	0.020	0.042	0.035
•		5 6	0.024	0.030	0.026
		6	0.023	0.027	0.024
	В	7	0.011	0.040	0.040
		7 8 9	0.012	0.034	0.030
•		9	0.005	0.042	0.037

BUILDING	BUILDING COMPONENT	CORE	HORIZON (M	HORIZON (MM FROM OUTSIDE FACE)		
			0 - 10	30 - 40	60 - 70	
66	В	1	0.012	0.012	0.010	
		2	0.010	0.012	0.012	
		2 3	0.012	0.012	0.012	
67	В	1	0.008	0.008	0.004	
		1 2 3	0.008	0.010	0.008	
		3	0.011	0.013	0.010	
69	В	1 2 3	0.029	0.028	0.019	
		2	0.034	0.029	0.026	
		3	0.033	0.021	0.022	
	v	4 5 6	0.030	0.034	0.030	
		5 .	0.044	0.051	0.054	
		6	0.054	0.070	0.059	
75	В	7	0.024	0.031	0.020	
		8 9	0.041	0.027	0.034	
		9	0.028	0.023	0.025	
	v	10	0.005	0.008	0.008	
		11	0.005	0.008	0.009	
		12	0.010	0.010	0.009	
81	• В	1	0.020	0.012	0.009	
	•	1 2 3	0.017	0.014	0.008	
		3	0.039	0.014	0.010	
86	B(PC)	1	0.017	0.011	0.012	
		1 2 3	0.015	0.012	0.010	
		3	0.021	0.010	0.012	
	V(PC)	4 5	0.011	0.009	0.010	
		5	0.011	0.009	0.013	
		6	0.005	0.008	0.007	
88	. В	7	0.017	0.045	0.025	
		8	0.017	0.024	0.036	
		9	0.015	0.038	0.017	
92	V	10	0.023	0.030	0.027	
		11	0.033	0.027	0.013	
•		12	0.026	0.036	0.035	
94	В	1	0.034	0.028	0.026	
		1 2 3	0.027	0.033	0.033	
		3	0.028	0.026	0.031	

BUILDING	BUILDING COMPONENT	CORE	HORIZON (MM FROM OUTSIDE FACE)		
			0-10	30 - 40	60 - 70
100	В	 4	0.011	0.009	0.009
		5	0.011	0.008	0.012
		6	0.011	0.009	0.012
130	V	7	0.008	0.014	0.013
		8	0.016	0.013	0.013
·		9	0.008	0.014	0.013
	PC	13	0.009	0.016	0.017
		14	0.012	0.012	0.012
		15	0.007	0.009	0.010
131	V	1	0.036	0.031	0.037
		1 2 3	0.038	0.027	0.029
		3	0.028	0.025	0.023
132	V	4	0.021	0.019	0.023
		5 6	0.017	0.022	0.018
		6	0.011	0.025	0.014
133	V	4	0.017	0.017	0.026
	·	5	0.012	0.009	0.013
		6	0.015	0.017	0.015
	PC	10	0.027	0.012	0.017
		11	0.012	0.009	0.013
		12	0.015	0.017	0.015

Table 6
Summary of Carbonation Depths

•				
Building No.	Age (Years)	Balconies	Average Dep of Carbonation Vertical cast- in-place Concrete	
4	27	_	9	N
6	12	2	16	_
7	20	2	15	_ _
8	20	<i>2</i>	9	- 6
11	16	<u>_</u>	- -	4
130	22	_	_ 15	20
131	16	_	3	20
132	25		23	4
133	22	·	5	N N
134	16	_		1 1
	10	_	_	1
		•		•
28	16	15	14	_
29	20	4	22	_
38	20	5		<u>_</u>
44	16	N	 13	
61	20	17	21	_
62	20	28	16	_
66	18	5	8	_
67	18	8	5	_
69	18	4	5	<u> </u>
7 5	17	3	31	_
75 79	16	5 6	15	_
81	23	4		_
86	16	4	_	2 (B)
	10	_	_	11(W)
88	18	_ 19	13	TT(M)
91	21	9	13	_
.92	15	12		_
94	18	17	13	_
100	17	1		-
100	1/	т	_	_
Ave	erage	8	14	5
Ran	nge	N-28	3-31	N-20

Note: 1. For the precast wall panels, if the results for buildings 130 and 86 are discarded, the results become: N-6

2. (B) Balconies, (w) Wall panels

Table 7
Summary of Splitting Tensile Tests

Average Tensile Strength (MPa)

Building No.	Balconies	Vertical Cast- in-place Concrete	Pre-cast
4	-	4.72	5.56
6	5.83	2.69	-
7	4.14	4.07	-
8	· –	2.82	4.57
11	_	- .	3.94
130	_	2.79	3.46
131	- ,	5.28	4.09
131	<u>-</u>	3.37	4.14
133	-	3.80	4.82
134	-	-	4.59
28	3.61	3.49	_
29	3.25	2.82	_
38	3.35	_	_
44	2.77	2.87	_
61	3.60	3.06	_
62	4.65	3.55	_
66	3.41	2.91	
67	2.72	2.59	_
69	3.08	4.62	
75	3.56	2.74	, -
79	3.62	2.48	_
81	3.64	-	-
86	-	-	3.14(B) 3.63(W)
88	2.14	2.63	3.03(W)
91	3.26	2.03	_
92		2.83	
92 94	2.48	2.83	
100	2.53 2.95	2.9 5	_ _

Table 8

Summary of Absorptions and Voids

Building No.

Absorption (%)

Voids (%)

	Balconies	Cast-in- place concrete	Pre-cast	Balconies	Cast-in- place concrete	Precast
4	· _ ·	4.5	2.6	<u>-</u>	11.5	7.2
6	5.6	3.5	_	12.9	7.5	_
7	4.7	4.7	-	11.8	11.8	_
8	<u>.</u> .	5.7	5.5	-	14.8	13.8
11		-	-	4.9	-	_
130						
_ 130	_	5.9	5.1	-	14.6	12.7
131	-	6.3	4.0	-	15.9	10.6
132	-	5.1	5.0	-	13.1	11.8
133	-	4.6	3.9	-	11.8	9.9
134	-	_	4.6	-	-	12.5
28	5.0	5.4	_	12.6	13.4	_
29	4.9	5.2	-	11.8	12.7	_
38	4.8	-	_	11.6	.=	-
44	4.5	4.8	-	11.3	11.8	-
61	5.2	5.8	-	14.1	14.6	· . -
62	5.1	5.7	_	16.5	13.9	_
66	5.7	2.5	-	13.6	6.1	_
67	5.3	2.3	_	12.8	5.7	_
69	2.8	1.3	_	7.1	2.6	_
75	5.4	5.9	-	13.5	14.8	_
7 9	5.0	2.4	-	12.1	6.7	-
81	5.7	-	_	14.0	_	-
86	-		5.9	-	-	14.5
	· -	-	5.2	-	-	12.9
88		5.0	5.3	-	14.1	14.4
L		-				
91	5.9	-	· -	14.8	-	_ '
92	6.4	6.4	-	15.7	15.4	-
94	5.3	5.7	-	13.2	13.8	-
100	4.8	_	_	12.3	_	_

Table 9
Summary of Constants B

Building	Age		•	
No.	(Years)	Balconies	Vertical cast- in-place Concrete	Pre-cast
4	27	_	0.17	Nil
. 6	12	0.06	0.35	_
7	20	0.05	0.34	-
8	20	-	0.20	0.13
11	16	_	_	0.10
130	22	-	0.32	0.43
131	16	-	0.08	0.05
132	2 5	-	0.46	0.08
133	22	- .	0.11	Nil
134	16		_	0.03
ŀ				
28	16	0.38	0.35	-
29	20	0.09	0.49	-
38	. 20	0.11	_	-
44	16	Nil	0.33	
61	20	0.38	0.47	-
62	20	0.63	0.36	-
66	: 18	0.12	0.19	-
67	18	0.19	0.12	-
69	18	0.09	0.12	-
75	17	0.07	0.75	-
79	16	0.15	0.38	-
81	23	0.08	-	-
86	16	-	-	0.05
-		-	-	0.28
88	18	0.45	0.31	-
91	21	0.20	-	-
92	15	0.31	0.72	-
94	18	0.40	0.31	-
100	17	0.02	-	-

Table 10
Splitting Tensile Tests: Statistical Summary

	Balconies	Vertical Cast- in-place Concre	te	Pre-cast
Without Public Building	N = 17 x = 3.21 s = 0.59	N = 13 x = 3.04 s = 0.57	Public Buildings	N = 8 x = 4.39 s = 0.64
All Buildings	N = 19 x = 3.39 s = 0.83	N = 21 x = 3.21 s = 0.79		N = 10 x = 4.19 x = 0.72

Table 11
Summary of Absorptions and Voids

Absorption (%)

	Balconies Cast-in- place concrete			Pre-cast
Excluding Public	N = 17	13	Public Buildings	8
Building	$\overline{x} = 5.1$	4.5	_	4.5
-	s = 0.76	1.72		0.92
All Buildings	N = 19	21		10
	$\bar{x} = 5.1$	4.6		4.7
•	s = 0.76	1.42		0.95
	5 0.70	* • 47		0.55

Voids (%)

	Balconies	Cast-in- place concre	ete	Pre-cast
Excluding Public Building	N = 17 x = 13.0 s = 2.08	13 11.2 4.32	Public Buildings	11.4 2.08
All Buildings	N= 19 $x = 12.9$ $s = 1.98$	21 11.8 3.76		10 11.8 2.11

Table 12
Orientation of Building Components

ľ												
Buildi No.	ng	Bal	conies	;		rtical ice Cor	Cast-I crete	[n-	Pre	cast C	Concret	:e
	N	S	E	W	N	s	E	W	N	S	E	W
4					9				N			
4 6	3 2	2			18	14						
7 8	. 2			3	16		_	14			_	
11						•	9			3	6	5
130			•			15				3	•	20
131		•				13		3				20 2
132							23	_		4		
133						5						
134												1
28		20	6			14	16					
29		20	6 3	8		22	10					
38			_	8 5								
44	2	.5			12	15						
61	10	23			22	19	.	_				
62 66		,	24 5	31		8	25	7				
67		8	5			5						*
69		4				J	5					
75			4	3			26	34				
79			4	8			15	15				
81	4								•	-		ъ
86 87									3 10	1 13		B W
88			18	21			14	14	10	ΤJ		VV
91	9	9										
92			7	14			30	29				
94	20	16	••	_	6	16						
100		<u> </u>	N	4	-				-			
Avgs.	7	11	8	11	14	13	18	17	7	5	. 6	7

Table 13

Values of B: Statistical Summary

	Balconies	Cast-In-Place Vertical Conc		Pre-Cast Concrete
Excluding Public Buildings	N = 17	13	Public Buildings	8
	$\bar{x} = 0.21$	0.38	241141190	0.10
,	s = 0.18	0.20		0.14
With	N = 19	21	All	10
Public Buildings	$\overline{x} = 0.20$	0.33	Buildings	0.11
	s = 0.17	0.18		0.14

Table 14

Average Chloride Contents

BUILDING COMPONENT: BALCONIES

BUILDING	PUBLIC(P) OR RESIDENTIAL(R)	HORIZON 0 - 10	(MM FROM OUTSI		
	RESIDENTIAL(K)	0 - 10	30 - 40	60 - 70	
28	R	0.022	0.024	0.027	
29	R	0.032	0.018	0.018	
38	R	0.017	0.010	0.010	
44	R	0.014	0.009	0.013	
61	R	0.010	0.030	0.029	
62	R	0.009	0.039	0.036	
66	R	0.011	0.012	0.011	
67	R	0.009	0.010	0.007	
69	·R	0.032	0.026	0.022	
75	R	0.031	0.027	0.026	
81	R	0.025	0.013	0.009	
88	R .	0.016	0.037	0.026	
94	R	0.030	0.029	0.030	
100	R	0.011	0.009	0.011	
BUILDING CO	OMPONENTS: VERTICAL C	AST-IN-PLACE O	CONCRETE		
6	P	0.013	0.015	0.012	
7	P	0.015	0.017	0.030	
29	R	0.013	0.024	0.024	
61	R	0.017	0.022	0.026	
62	R	0.022	0.033	0.028	
69	R	0.043	0.052	0.048	
75	R	0.007	0.009	0.009	
92	R	0.027	0.031	0.025	
130	P	0.011	0.014	0.013	
131	P	0.034	0.028	0.030	
132	P	0.016	0.022	0.018	
133	P	0.015	0.014	0.018	
BUILDING CO	OMPONENTS: PRE-CAST C	CONCRETE			
86	R	0.018	0.011	0.011	
,		0.009	0.009	0.010	
130	P	0.009	0.012	0.013	
133	P	0.018	0.013	0.015	

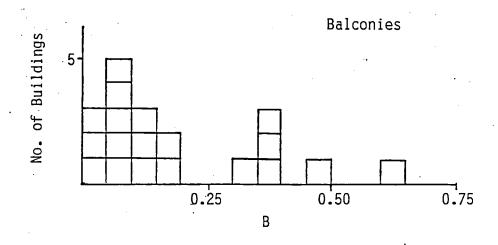
Table 15 Summary of Properties

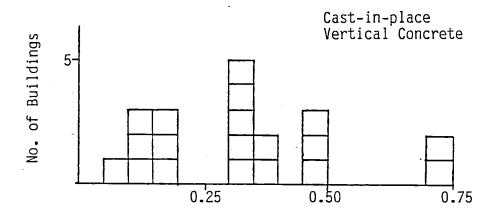
Building	Sample	Component	Orientation	Splitting Tensile Strength (MPa)	Absorption %	Carbonation Depth (mm)
6	6/5	V	S	2.64	3.3	2.0
61	61/5	V	S	3.34	5.4	15
91	91/1 91/8	B B	n s	3.38 3.57	7.0 5.4	9 14
92	92/3 92/9	B V	W W	2.49 2.30	6.6 5.8	16 28
133	133/9	V	S	3.97	4.7	10

NOTES: V - vertical cast-in-place
B - Balcony
S - South

N - North w - West







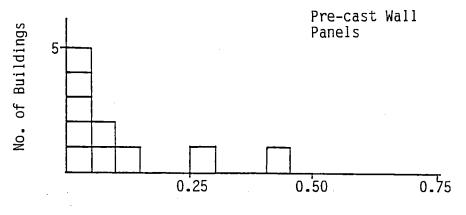


FIGURE 2: VALUES OF B

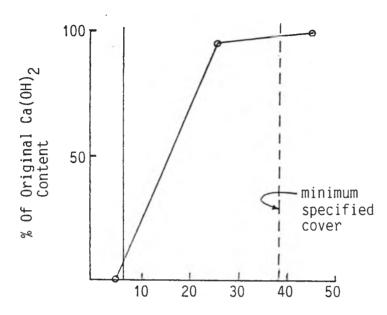


FIG 3 Sample 4/4

Distance from exposed face: mm

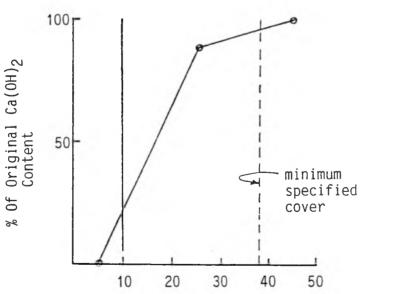


FIG 4 Sample 6/5

Distance from exposed face: mm

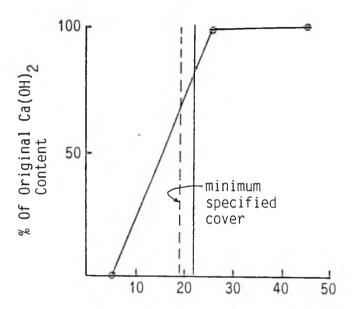


FIG 5 Sample 28/3

Distance from exposed face: mm

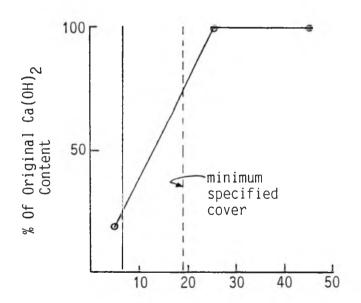


FIG 6 Sample 38/4

Distance from exposed face: $\ensuremath{\mathsf{mm}}$

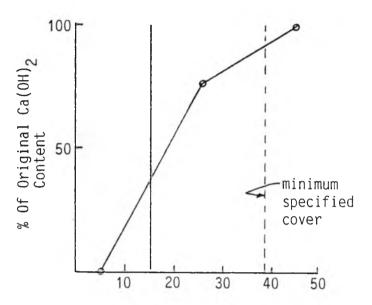


FIG 7 Sample 61/5

Distance from exposed face: mm

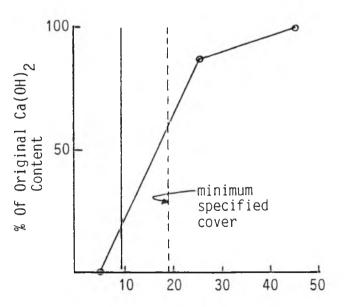


FIG 8 Sample 91/1

Distance from exposed face: mm

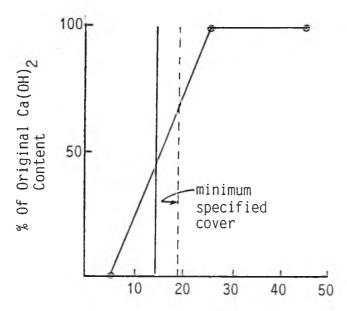


FIG 9 Sample 91/8



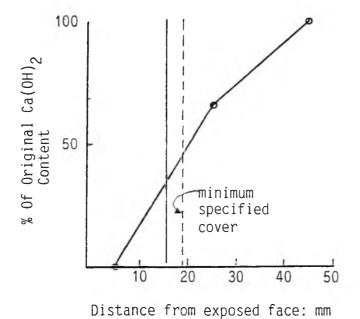


FIG 10 Sample 92/3

Vertical line and shaded area shows depth of carbonation as determined by phenolphthalein

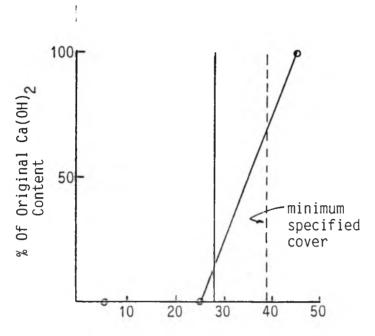
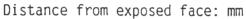


FIG 11 Sample 92/9



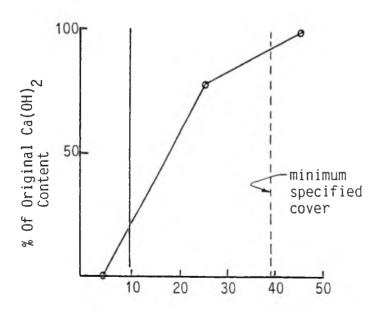
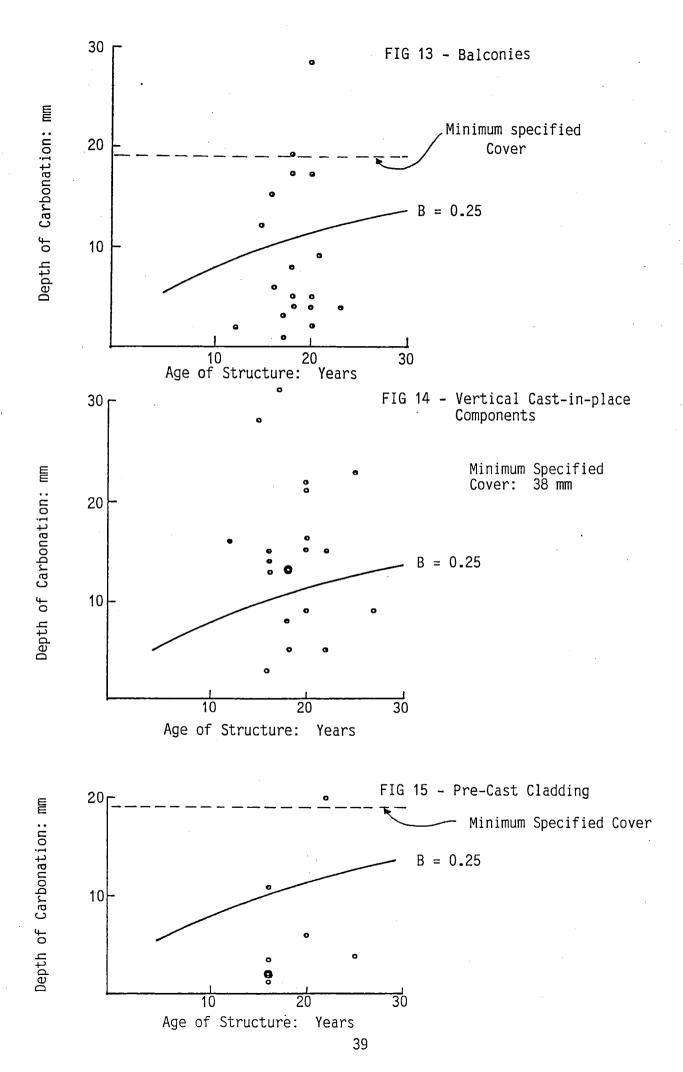
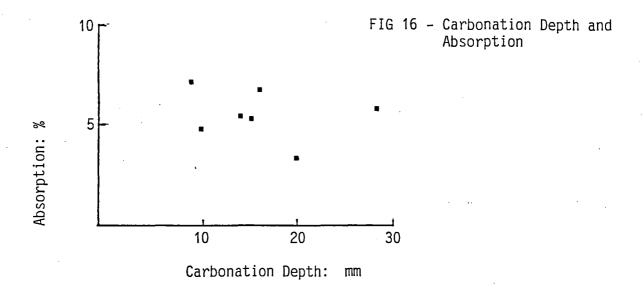
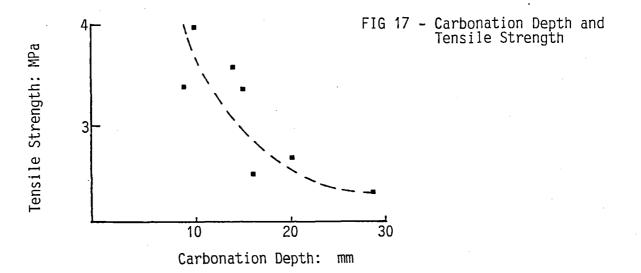


FIG 12 Sample 133/9

Distance from exposed face: $\ensuremath{\mathsf{mm}}$







APPENDIX "A"



National Research Council Canada

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Institute for Research in Construction Institut de recherche en construction

CLIENT REPORT

for

John A. Bickley Associates Limited Suite 214, 208 Evans Avenue Toronto, Ontario M8Z 1J7

Determination of the Lime and Carbonate Content of Concrete Core Specimens

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Report No. CR-5864.1

Report date: 8 December 1989

Contract No. CR-5864

Reference: Application for test dated 24 February 1989

Section: Materials

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DETERMINATION OF THE LIME AND CARBONATE CONTENT OF CONCRETE CORE SPECIMENS

Introduction

The lime and carbonate content of concrete core specimens, received from John A. Bickley Associates, were determined by Thermogravimetric Analysis.

Specimens

The cores received were registered in our laboratory as follows:

Original Code Number	IRC Code Number
4/4	D 135
6/5	D 136
28/3	D 137
38/4	D 138
61/5	D 139
91/1	D 140
91/8	D 141
92/3	D 142
92/9	D 143
133/9	D 144

With a water-cooled diamond saw 10 mm thick slices were cut off from the samples. The 0 to 10, the 20 to 30 and 40 to 50 mm sections (the reference point being the external surface) were selected for the analysis. These specimens were identified by adding to their code numbers the suffix 10, 30 and 50, respectively.

Without removing the coarse aggregates the specimens were dried and pulverized in a Buhler grinder.

The specimens to be analyzed were kept in a sealed vial until tested.

Thermogravimetry

Specimens, 50 mg in weight, were heated in a DuPont Model 9900 instrument at 20°C/min in a stream of nitrogen gas (100 mL/min).

Results

The results of the thermogravimetric analysis are shown in a graphical form in Figures 1 through 30.

The absolute weight loss percent (ordinate on the left hand side) as a function of temperature and the first derivative of the weight percent lost (ordinate on the right hand side) as a function of the temperature are given.

Due to a computer error the Run Date is incorrect.

The results are presented also in a tabular form (Table 1).

The peak of the derivative that occurs between 450 and 550°C is due to decomposition of Ca(OH)₂ in hydrated tricalcium silicate. The peak at approximately 780°C indicates the decomposition of CaCO₃.

The areas below the peaks were integrated and the obtained values are directly proportioned to the Ca(OH)₂ and CaCO₃ concentrations in the specimens.

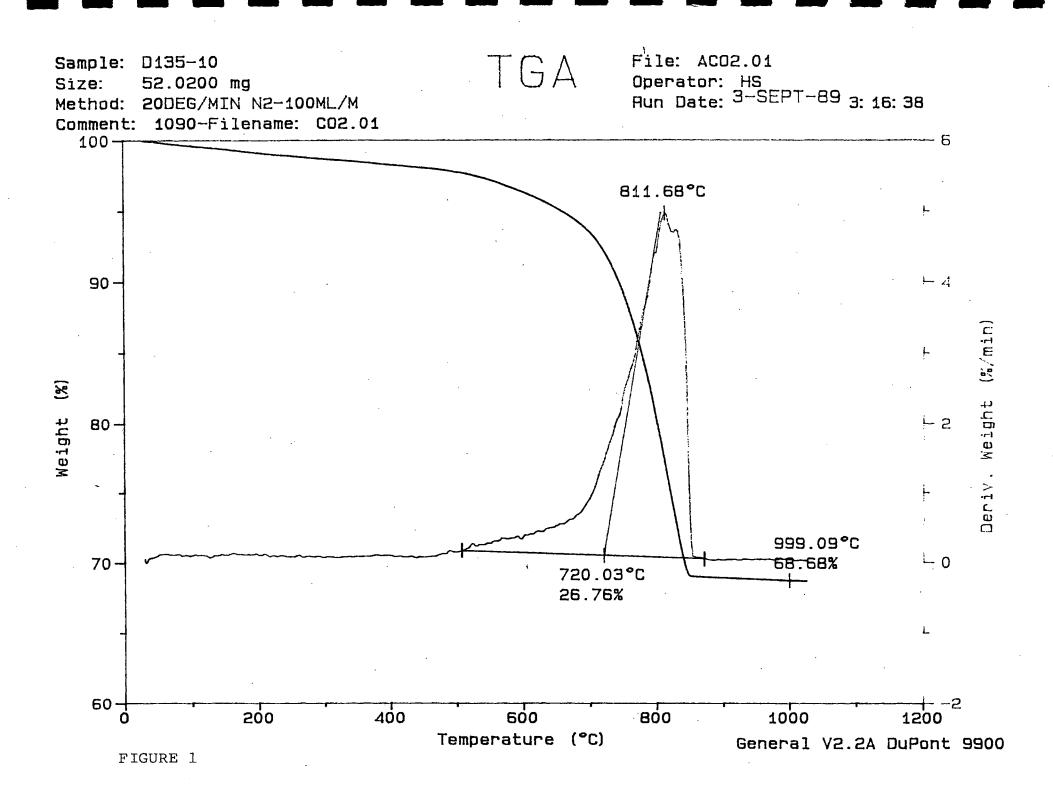
Comments

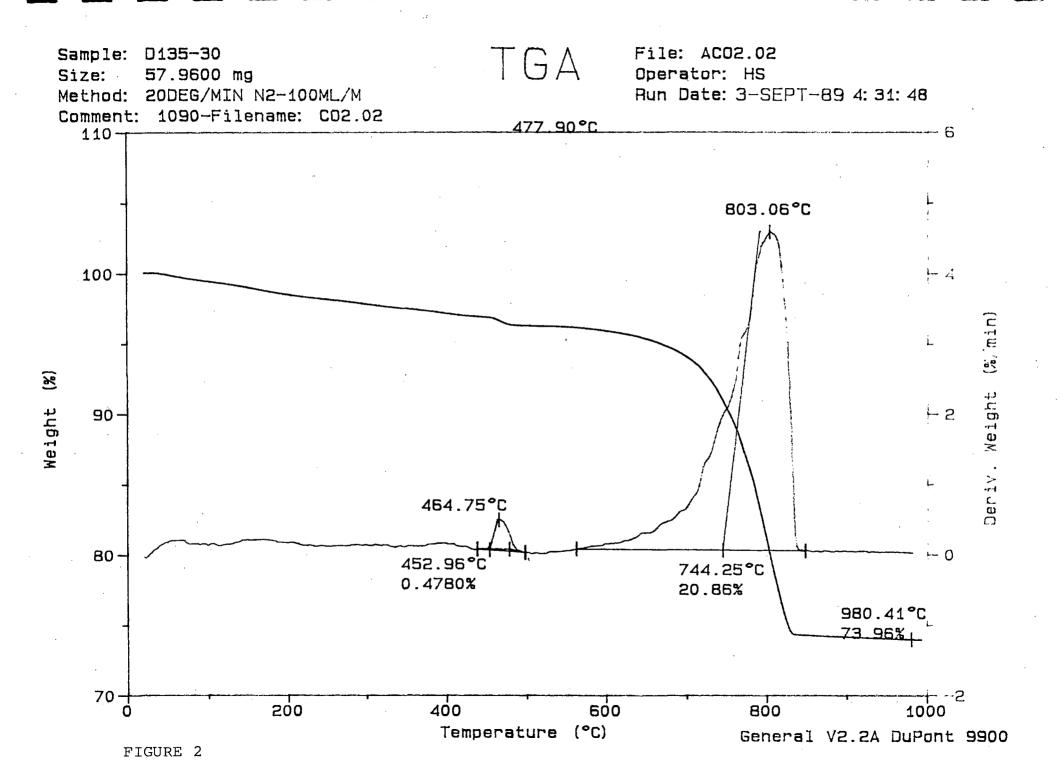
Because there is no method available to remove reliably from the concrete all aggregate, which may contain CaCO₃, the values given in Table 1 indicate the total CaCO₃ concentrations, not only that resulting from the carbonation of lime with atmospheric CO₂.

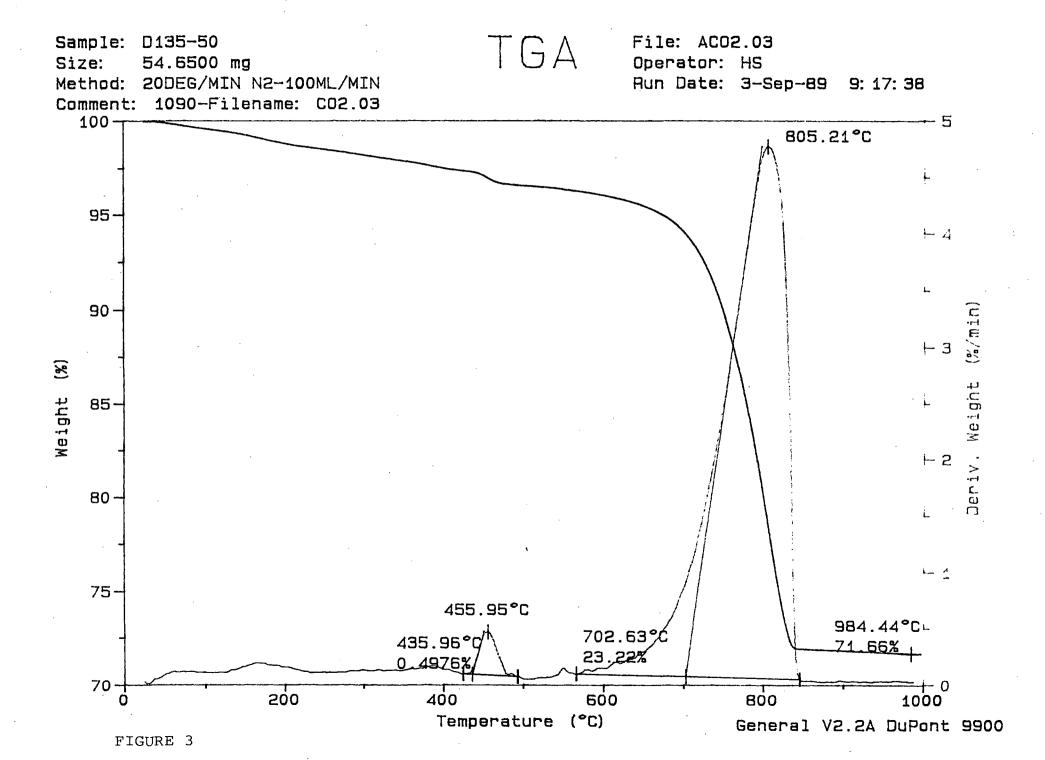
The extent of carbonation can be estimated from the weight losses at around 450°C. As a first approximation one may assume that at the peaks obtained by testing the code 50 specimens represent the lime content of, or close to, that originally present in the hydrated cement. Accepting the lime content of code 50 specimens as reference, the decrease at a given horizon is the measure of the extent of carbonation.

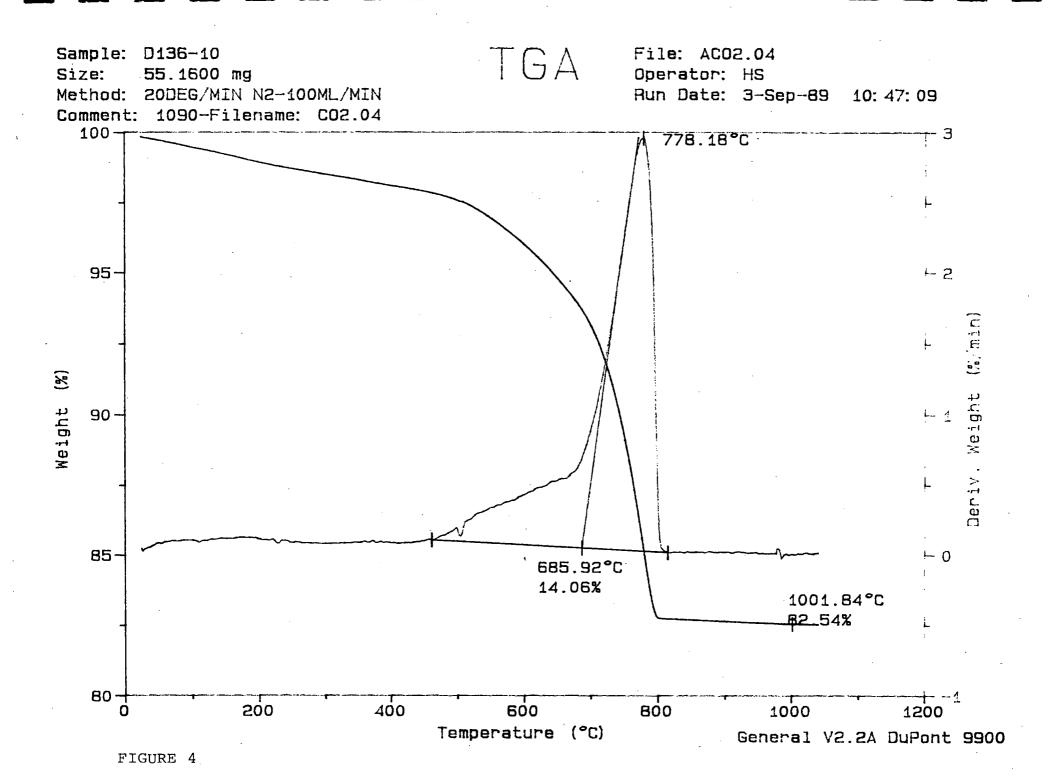
TABLE 1. RESULTS OF CARBONATION TESTS ON CONCRETE CYLINDERS

WEIGHT LOSS EXPRESSED AS % OF ORIGINAL SAMPLE WEIGHT							
SAMPLE	PEAK AT	450 DEG. C	PEAK AT 800 DEG. C				IGNITED WT AT 1000 DEG. C
NO.	AS H2O	AS Ca(OH)2		AS CO2	AS CaCO3		
D135-10	0.00	0.00		26.76	60.88		68.68
D135-30	0.48	1.97		20.86	47.46		73.96
D135-50	0.50	2.05		23.22	52.83		71.66
D136-10	0.00	0.00		14.06	31.99		82.54
D136-30	0.41	1.69		10.46	23.80		84.81
D136-50	0.46	1.90		9.66	21.98		84.95
D137-10	0.00	0.00		21.78	49.55		74.27
D137-30	0.20	0.80		27.72	63.06		67.37
D137-50	0.20	0.81		25.69	58.44	,	69.25
D138-10	0.08	0.34		21.11	48.03		73.36
D138-30	0.51	2.11		18.98	43.18		76.42
D138-50	0.43	1.77		21.84	49.69		73.49
D139-10	0.00	0.00		28.29	64.36		68.22
D139-30	0.23	0.93		29.85	67.91		66.03
D139-50	0.30	1.22		25.99	59.13		70.02
D140-10	0.00	0.00		29.19	66.41		66.94
D140-30	0.16	0.68		23.66	53.83		- 71.43
D140-50	0.19	0.77		27.68	62.97		68.52
D141-10	0.00	0.00		30.50	69.39		65.87
D141-30	0.14	0.56		27.42	62.38		68.55
D141-50	0.13	0.55		28.55	64.95		67.45
D142-10	0.00	0.00		23.67	53.85		73.00
D142-30	0.22	0.90		26.62	60.56		69.27
D142-50	0.33	1.36		21.75	49.48		73.95
D143-10	0.00	0.00		22.56	51.32		74.28
D143-30	0.00	0.00		25.35	57.67		70.82
D143-50	0.24	0.97		26.82	61.02		69.14
D144-10	0.00	0.00		25.03	56.94		71.66
D144-30	0.27	1.11		27.26	62.02		69.05
D144-50	0.34	1.40		23.46	53.37		72.50





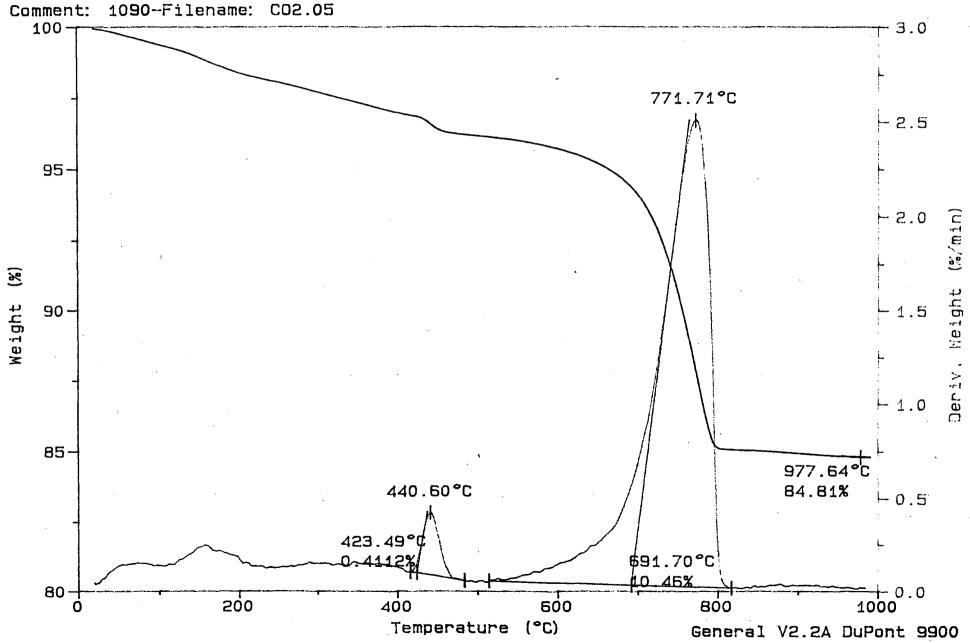


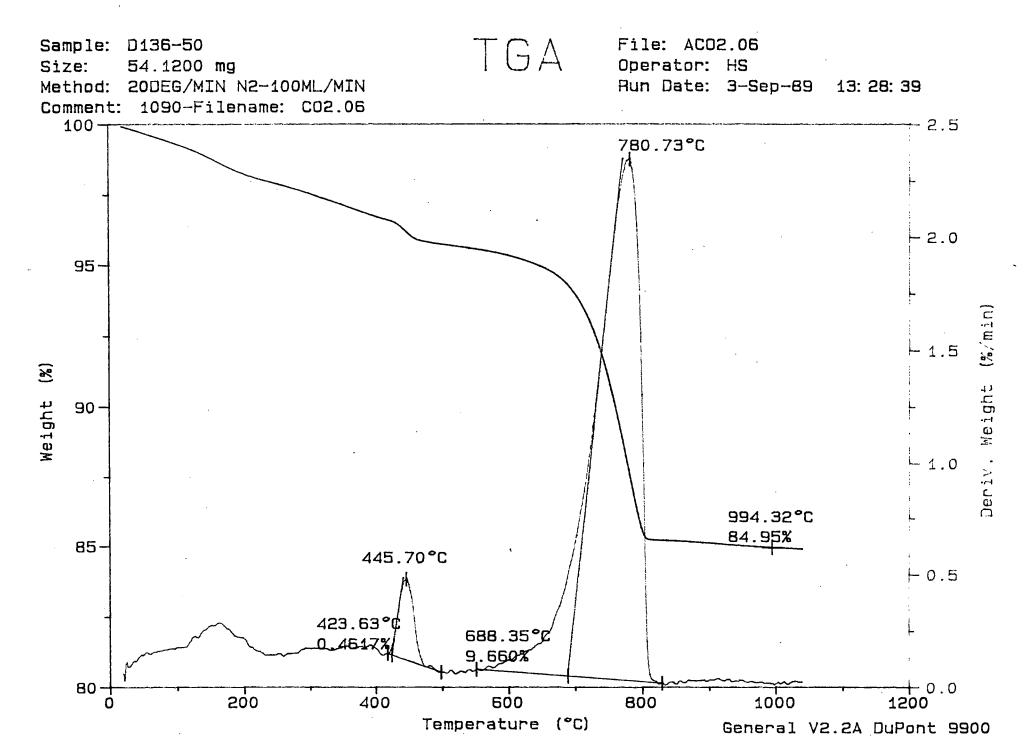


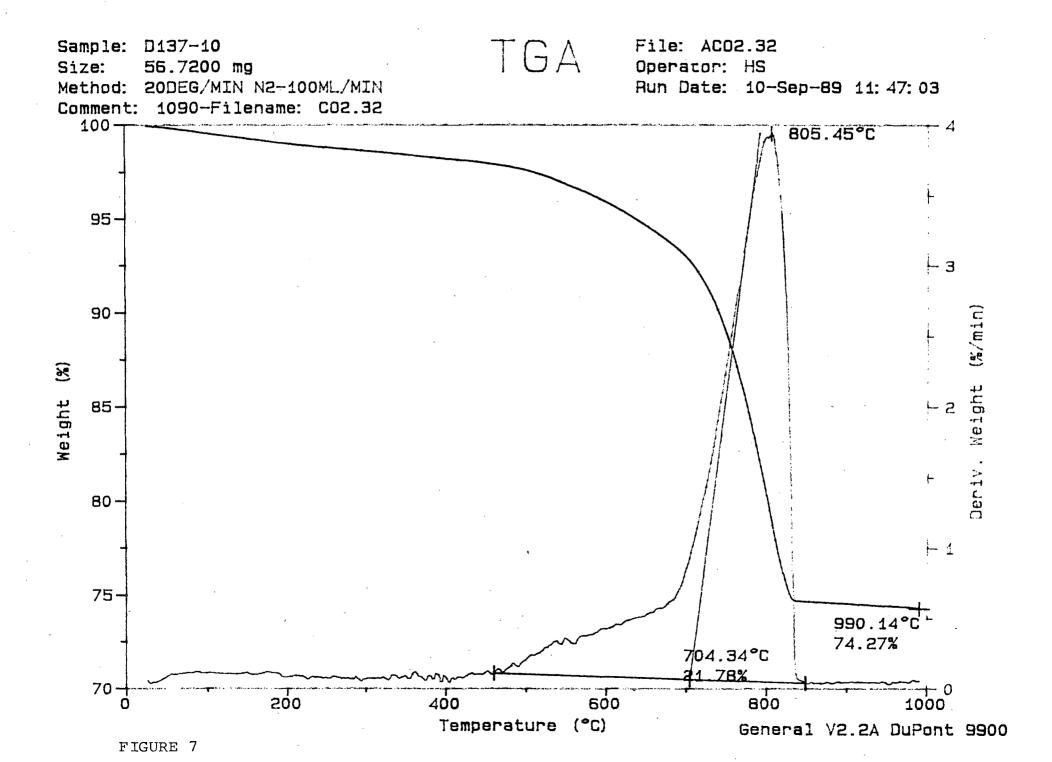
Sample: D136-30 Size: 54.8900 mg Method: 20DEG/MIN N2-100ML/MIN 100 -95-

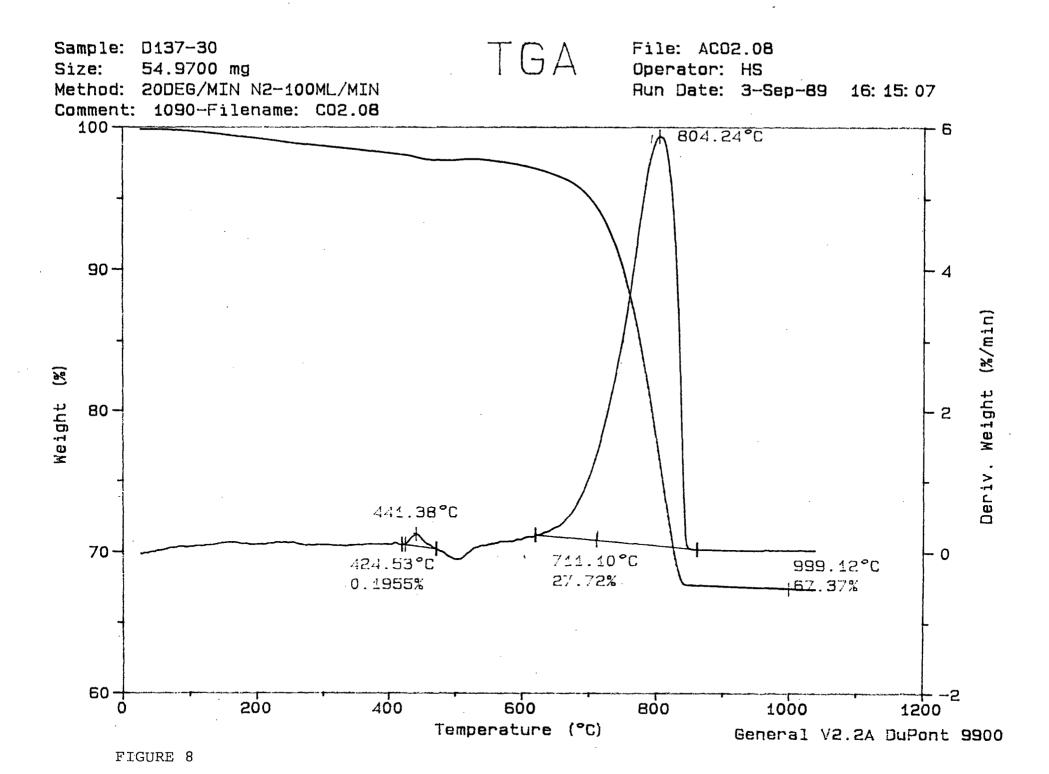
TGA File: ACO2.05 Operator: HS

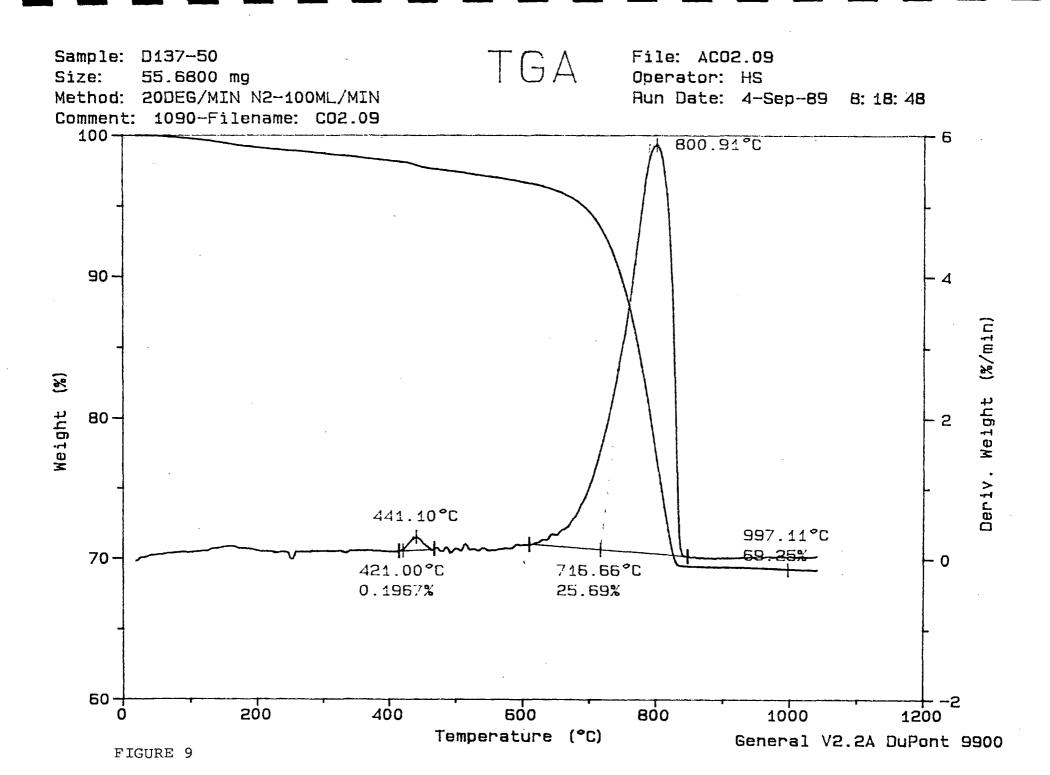
Run Date: 3-Sep-89 12: 10: 24

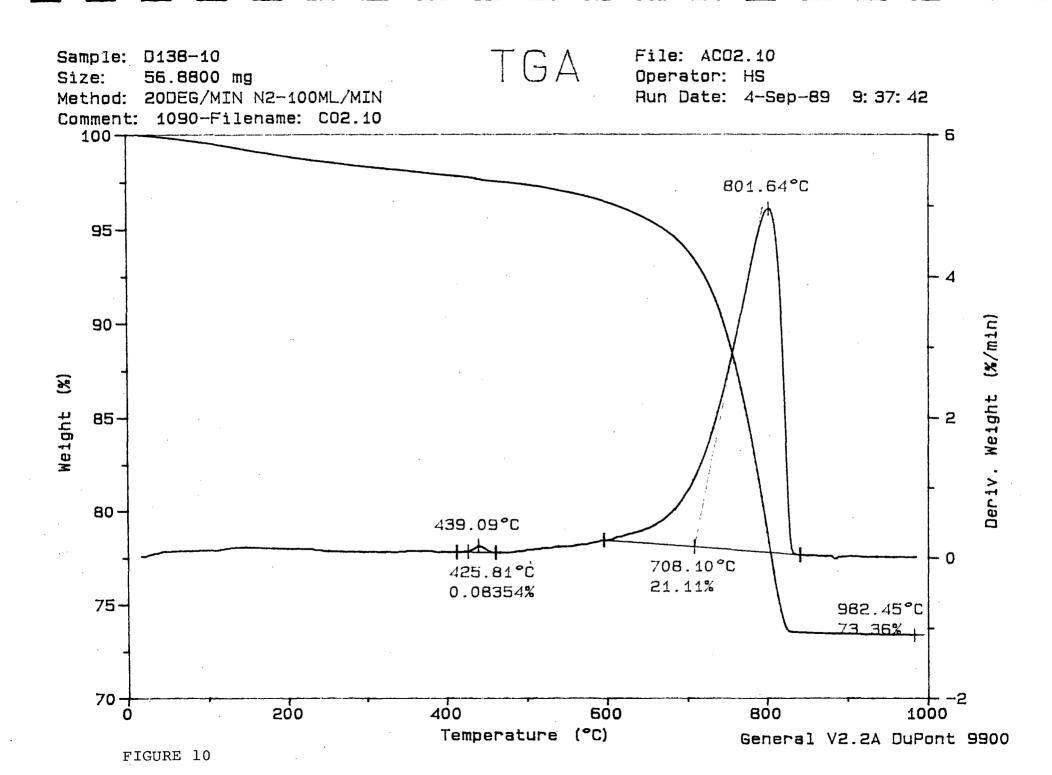


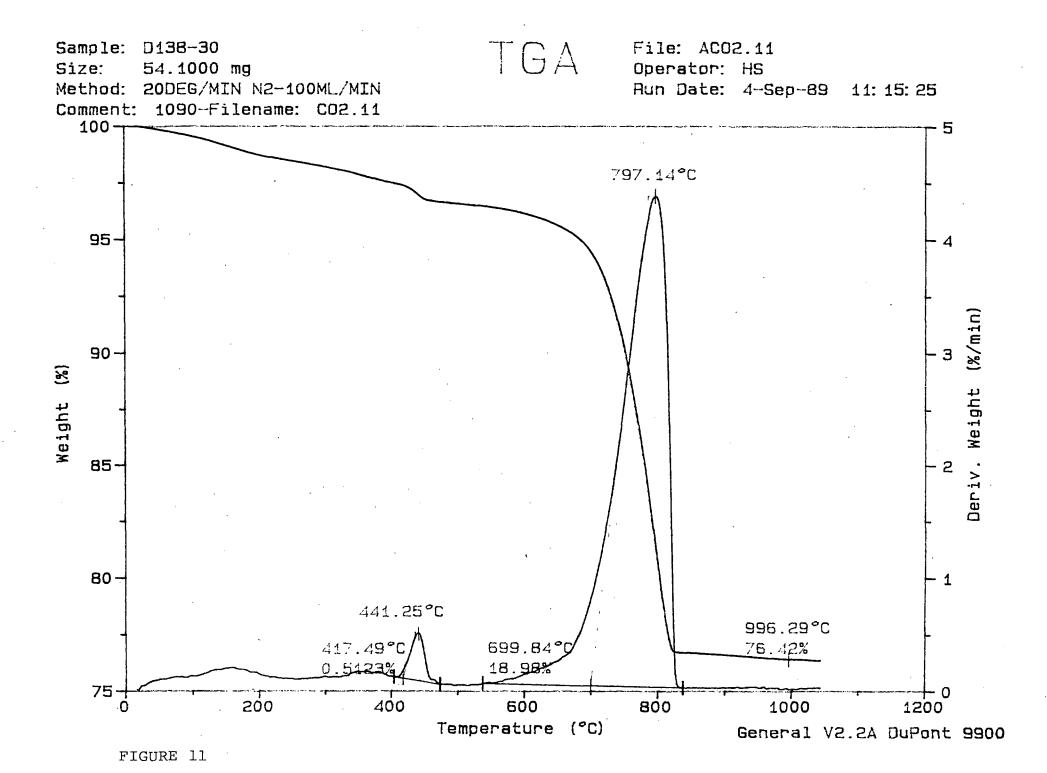












Sample: D138-50 Size: 56.7000 mg Method: 20DEG/MIN N2-100ML/MIN Comment: 1090-Filename: C02.12

TGA

File: ACO2.12 Operator: HS

Run Date: 4-Sep-89 12: 37: 21

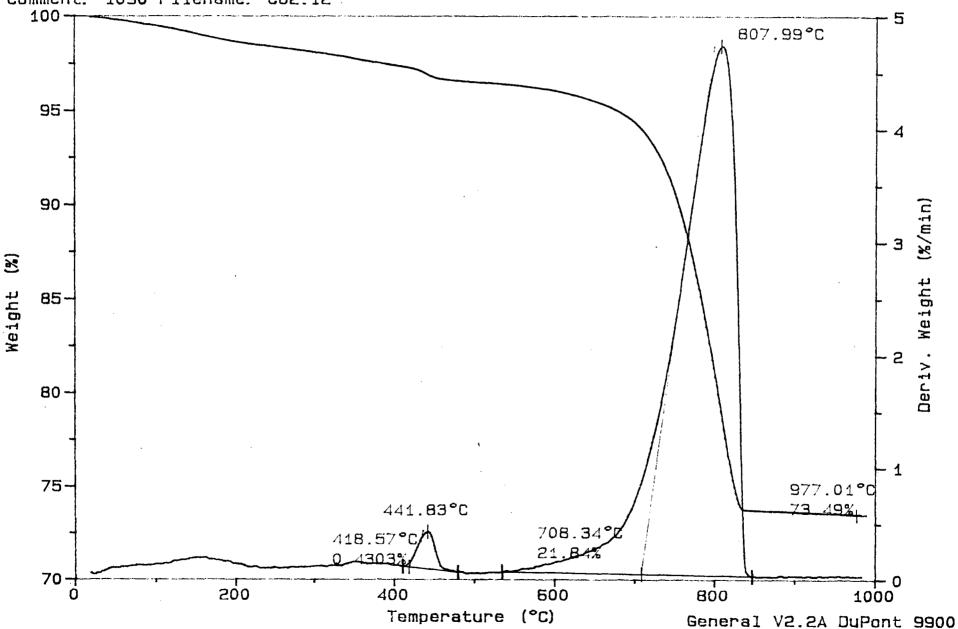
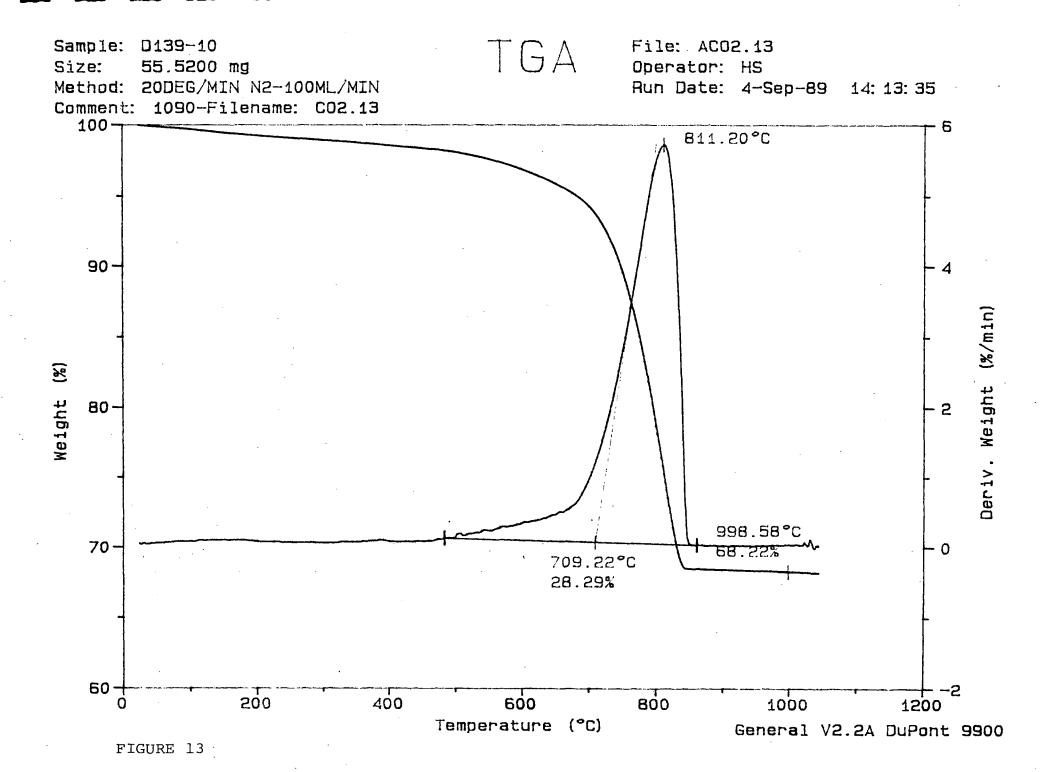
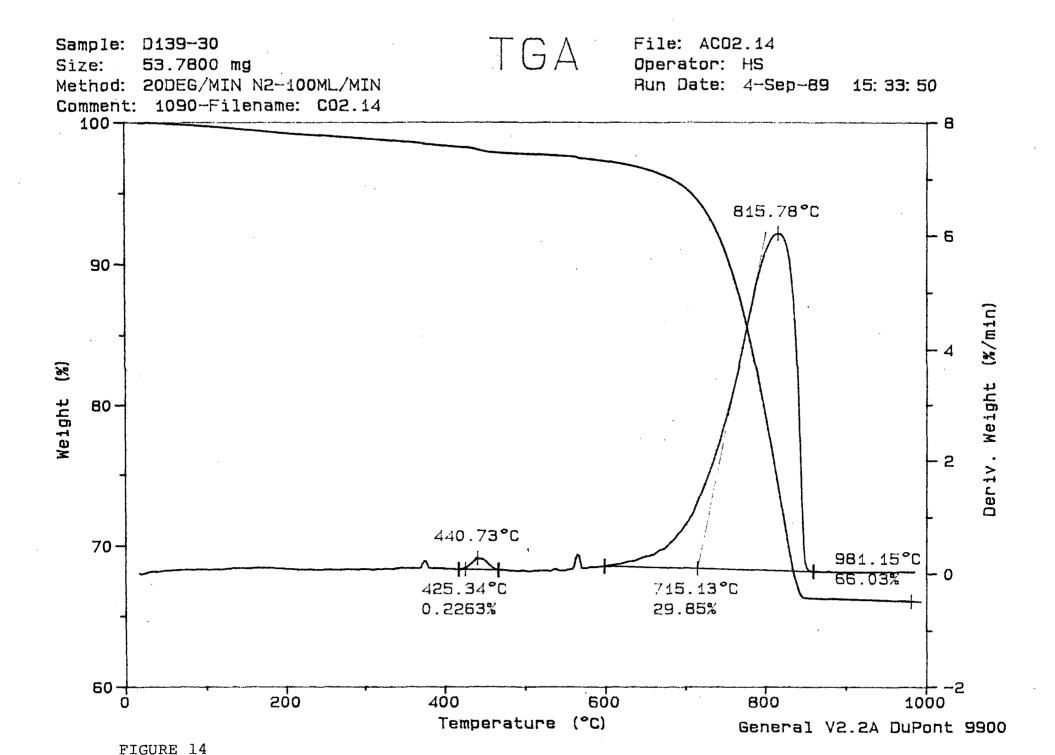
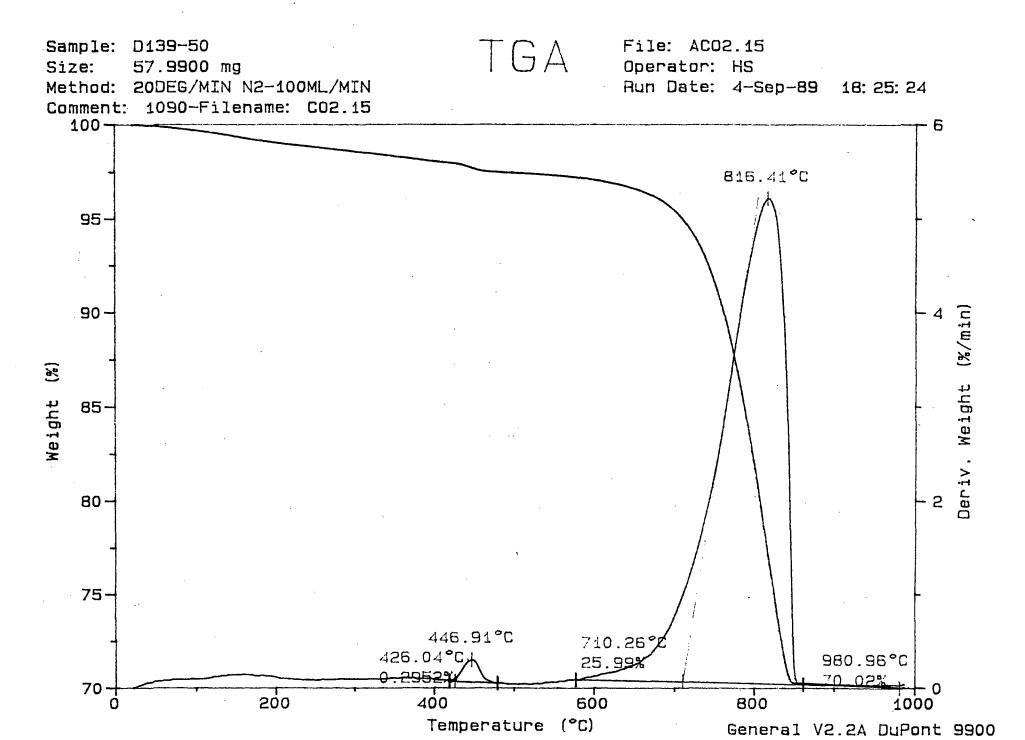
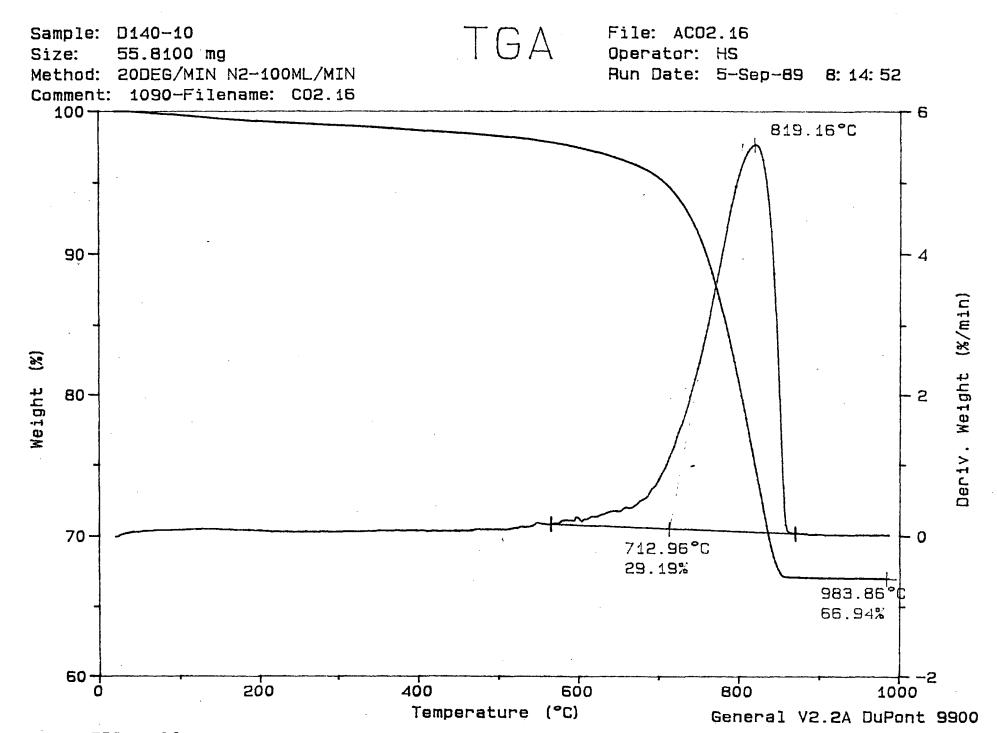


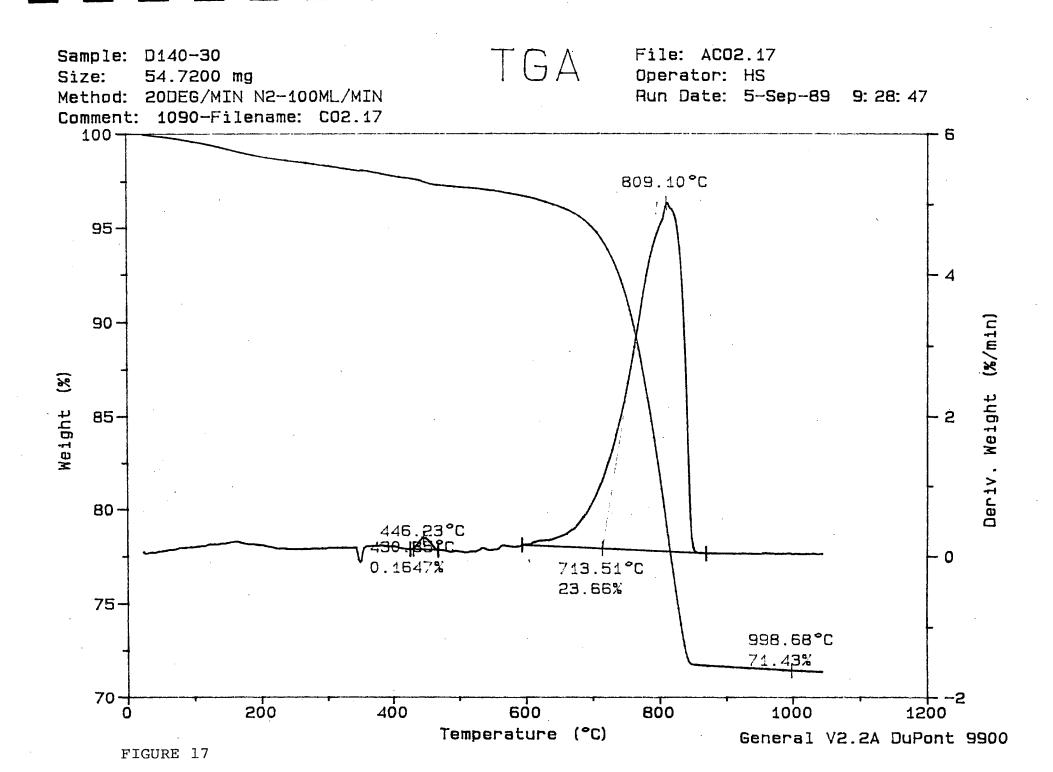
FIGURE 12











Sample: D140-50

56.3900 mg Size:

Method: 20DEG/MIN N2-100ML/MIN

Comment: 1090-Filename: CO2.18



File: ACO2.18 Operator: HS

Run Date: 5-Sep-89 10:41:19

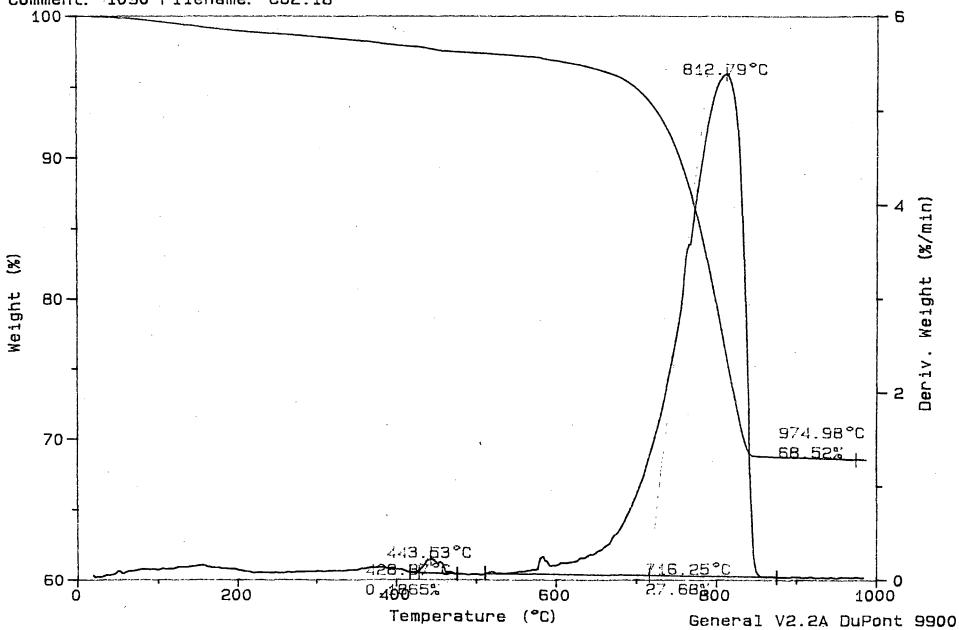


FIGURE 18

